

5 July 2021 Our Reference: 1664-1

The Planning Collective PO Box 591 Warkworth 0941

Attention: Burnette O'Connor

Dear Burnette,

RE: RESOURCE CONSENT ENGINEERING DESIGN REPORT FOR A PRIVATE PLAN CHANGE AND PROPOSED 16 LOT RESIDENTIAL SUBDIVISION AT 751 & 787 KAIPARA COAST HIGHWAY, KAUKAPAKAPA FOR RIVERVIEW PROPERTIES LTD

Aspire Consulting Engineers Limited have been engaged by Riverview Properties Ltd to prepare an engineering design report for the proposed 16 lot rural/residential subdivision at the above site to accompany an application for Private Plan Change and Resource Consent addressing the key infrastructure requirements with respect to the following:

- Earthworks
- Access
- Stormwater
- Water Supply and fire fighting

#### 1.0 Introduction and Proposal:

It is proposed to rezone 5.7 hectares from Rural - Countryside Living Zone to Residential-Rural and Coastal Settlement Zone. It is also proposed to subdivide the property into 16 lots. The lots range in size from 2,500 m<sup>2</sup> to 2,696 m<sup>2</sup>.

The site currently gains assess from Kaipara Coast Highway via MacLennan Farm Lane.

Auckland Councils GIS database (see fig. 1) outlines that there are no existing public wastewater and/or water supply available to the site.



Fig. 1 Auckland Council GIS Data

Existing wastewater and water supply are not available to the site.



Fig. 2 Proposed 16 lot residential subdivision scheme plan.





## 2.0 Earthworks:

It is proposed to undertake cut and fill earthworks to create suitable the subgrade formation for the JOAL's. Cut and fill earthworks modelling has been completed for the development with approximate cut and fill volumes of 200 m<sup>3</sup> and 75 m<sup>3</sup> respectively over an area of approximately 3,150 m<sup>2</sup>.

No earthworks are proposed on the lots.

Erosion and sediment controls are to be installed prior to the commencement of any earthworks on the site and maintained for the full duration of the works. Erosion and sediment control plans have been prepared refer appendix A for details. Typical silt control measures will be utilised including silt fences, topsoil bunding, clean water diversion bunds and decanting earthbunds in accordance with Auckland Council GD05. Final design details will be approved by Auckland Council at engineering design stage.

A geotechnical investigation report has been prepared by CMW Geosciences for the site. The earthworks will be undertaken under the observation of CMW Geosciences and certified upon completion.

Water carts and other dust suppression methods will be utilised to avoid dust nuisance.

The earthworks methodology is likely to be as follows:

- Install erosion and sediment controls
- Strip topsoil to stockpile or waste
- Undertake cut to fill
- Progressively metal or respread topsoil, grass and straw mulch as earthworks areas are completed
- Remove erosion and sediment controls when the site is stabilised.

#### 3.0 Access:

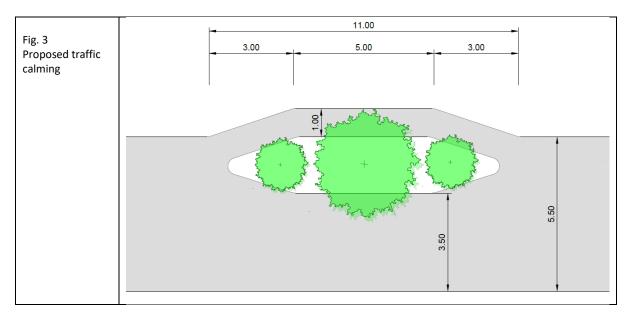
The site will gain access from Awatiro Drive and MacLennan Farm Lane and then Kaipara Coast Highway (SH16) via an existing intersection.

The existing intersection has been designed in accordance with Austroads and MOTSAM. It included dedicated right turn bay and left turn lane into MacLennan Farm Lane. It was designed for a speed of 100 km/hr although the speed limit is now 80 km/hr.

The proposed JOAL's will be formed in 150mm thick 20MPa exposed aggregate concrete with 8kg/m³ black oxide. The JOAL's will vary from 3.5m - 5.5m wide.

The JOAL will be a shared space. Planting and traffic calming is proposed to keep vehicle speed down. Where there is narrowing's and planting, pedestrians can safety walk behind the plants on a 1.0, wide concrete path.





For further information on access and traffic matters, refer to Stantec Transport Assessment.

#### 4.0 Stormwater:

The site currently discharges stormwater via an existing 450 mm dia public stormwater pipe in the southern corner of the site and to existing roadside swales. It is proposed to extend the stormwater network (which was installed and vested as part of stage 1) to provide stormwater connections to Lots 7-15. Lots 1-6 & 16 and the JOAL's will discharge to the existing roadside swales.

Although there is downstream flooding, the site is in the lower reaches of the catchment and if stormwater attenuation was implemented, flooding could worsen. Therefore, as with the stage 1, 36 lot residential development, no attenuation was required.

Stormwater treatment from the proposed JOAL will be via grassed swales. There is no treatment of stormwater required from individual lots. Calculations in accordance with TP 108 and TP 10 are attached in appendix B. The swales will discharge into a proposed stormwater network and/or existing roadside swales. An indicative pipe layout is included in the plans.

The proposed dwellings will have rainwater collection and storage in roof water tanks. Tank overflows will discharge to the stormwater connection.

Detailed engineering design details of the proposed stormwater drainage will be submitted to Auckland Council for approval at engineering design stage. All design detailing will be in accordance with Auckland Council Engineering Design specifications.



Below is the Auckland Council GIS Overland flowpaths.



We note that the overland flowpath circled in red is incorrect. The contours show the stormwater would be sheet flow and not concentrated in an overland flowpath. The overland flowpaths circled in magenta, were modified as part of stage 1 when MacLennan Farm Lane was constructed. Therefore, all actual overland flowpaths will generally remain unchanged and no dwellings will be proposed in overland flowpaths.

## 5.0 Water Supply:

Public mains water supply is not available to the site. The proposed development will utilise onsite storage tanks with collection from roof water for potable and non-potable supply.

The Country Side Living Tool Box for Stormwater Management Design has been adopted for the development. Refer Country Side Living Tool Box, Chapter 4.6, table C12 below:



The Rodney District Council has minimum water tank sizes for household water supply where the tank is the only water supply to the home. Table C12 provides the tank size needs for variable roof areas and number of bedrooms.

Table C12 Minimum Tank Size for RDC Homes Having Tanks as Sole Water Source										
Usable	Bedrooms									
Roof Area (m²)	1	2	3	4	5					
100	20	50								
120	15	35	75							
140	10	30	60							
160		20	50							
180			45	75						
200			35	65						
220			30	55	90					
240			30	50	80					
260				45	70					
280				40	65					
300				35	60					

Colours indicate units of 25 cubic metres (5,000 gallons):

1 x 25 m <sup>3</sup> 2 x 25 m <sup>3</sup> 3 x 25 m <sup>3</sup> 4 x 25 m
--

Detailed design of the water supply for each lot should be detailed, designed and approved with Auckland Council at building consent stage.

It is proposed to provide dedicated fire fighting water tanks as part of the development. They will have minimum storage of  $45 \text{ m}^3$  and the locations are shown on drawing 1664 - RC - WS501.

We believe this report covers the engineering requirements outlined for resource consent. Please feel free to contact the undersigned if you have any questions.

Report prepared by:

Phil Fairgray **DIRECTOR** 

# ATTACHMENTS:

APPENDIX A: RESOURCE CONSENT DESIGN DRAWINGS

APPENDIX B: SWALE CALCULATIONS



APPENDIX A: RESOURCE CONSENT DESIGN DRAWINGS



APPENDIX B: SWALE CALCULATIONS

#### GD01 Design Sheet - Swales 5/07/2021 For use with GD01 - Stormwater Guidelines for the Auckland Region Date: Address: McLennan Farm Lane Project: Stage 2 Reviewer: P Fairgray **INITIAL PARAMETERS** Input 646 Impervious area contributing to device $m^2$ Calculation Pervious area contributing to device 304 Results $m^2$ Total area contributing to device 950 $m^2$ **CONTROL DATA** 10% AEP peak rainfall 170.0 mm [1] 10% AEP peak flow rate 115.3 mm/hr 10% AEP runoff 0.02257 m<sup>3</sup>/s Water quality flow rate 10.0 mm/hr Runoff Coefficients 0.00170 m<sup>3</sup>/s 0.95 Water quality runoff [2] Impervious Pervious 0.3 SWALE SPECIFICATIONS Grassed Swale type Longitudinal slope (i) 1% Swale side slope (z) 3.0 1V:zH 0.30 Base width m Freeboard 110 Effective length

65.00	m
0.25	
0.03	[3]
19.73	min
29.65	m

Flow de		
WQF	70.0	mm
10%	mm	

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Swale capacity greater than runoff into swale?

Swale velocity below threshold?

WQF	10% AEP
OK	OK
OK	OK

[6]

# Notes:

Check

Manning's for Water Quality Flow

Manning's for 10% AEP

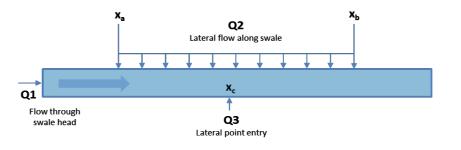
- [1] Peak 10-min flow from 10% AEP event is calculated as 6 x 11.3% of 24 hour rainfall depth (TP108)
- [2] Runoff from pervious surfaces is ignored for the water quality flow.
- [3] Manning's for grassed and vegetated swales based on GD01.
- [4a] In instances where 10% AEP flow depth is less than WQF depth, check for supercritical flow conditions.

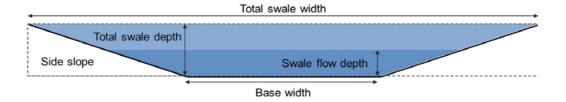
A faster flow speed will cause a shallower depth. In these instances, Manning's may not be suitable for calculation.

- [4b] If solver results in 0.00, manually enter estimate depths, then re-click solver button
- [5] Target time of 9 minutes must be achieved for adequate water quality treatment.

Variable	WQF	Unit	10% AEP
Water surface width	0.72	m	0.84
Cross-sectional area	0.036	m <sup>2</sup>	0.051
Hydraulic Radius	0.048	m	0.059
Swale Velocity	0.055	m/s	0.525
Swale flow	0.002	m <sup>3</sup> /s	0.027

	Effective Length Calculator								
		Flow Location							
Q1	0%	0.000000	Total Length	130					
Q2	100%	0.001705	Xa	0					
Q2	10076	0.001703	X <sub>b</sub>	130					
Q3	0%	0.000000	X <sub>c</sub>						
Total	100%	0.00170	Eff. Length	65.0					





# **RIVERVIEW PROPERTIES LIMITED** PROPOSED DEVELOPMENT MacLENNAN FARM LANE **KAUKAPAKAPA**

# **RESOURCE CONSENT DESIGN DRAWINGS:**

PRELIMINARY & GENERAL

PG101 **COVER AND CONTENTS PAGE** 

PG102 **EXISTING SITE PLAN** 

**EARTHWORKS** 

EW201 PROPOSED CONTOUR PLAN

PROPOSED CUT & FILL DEPTH CONTOUR **EW202** 

**PLAN** 

**EROSION & SEDIMENT CONTROL PLAN** EW203

**EW204-206 EROSION & SEDIMENT CONTROL DETAILS** 

**ROADING** 

**PROPOSED JOAL PLAN RD301** 

**RD302** TYPICAL JOAL X-SECTION

**RD303** TYPICAL JOAL NARROWING

**STORMWATER** 

SW401 PRELIMINARY STORMWATER LAYOUT

**SWALE CATCHMENT PLAN SW402** 

FIRE FIGHTING WATER SUPPLY

FIRE FIGHTING WATER SUPPLY WS501

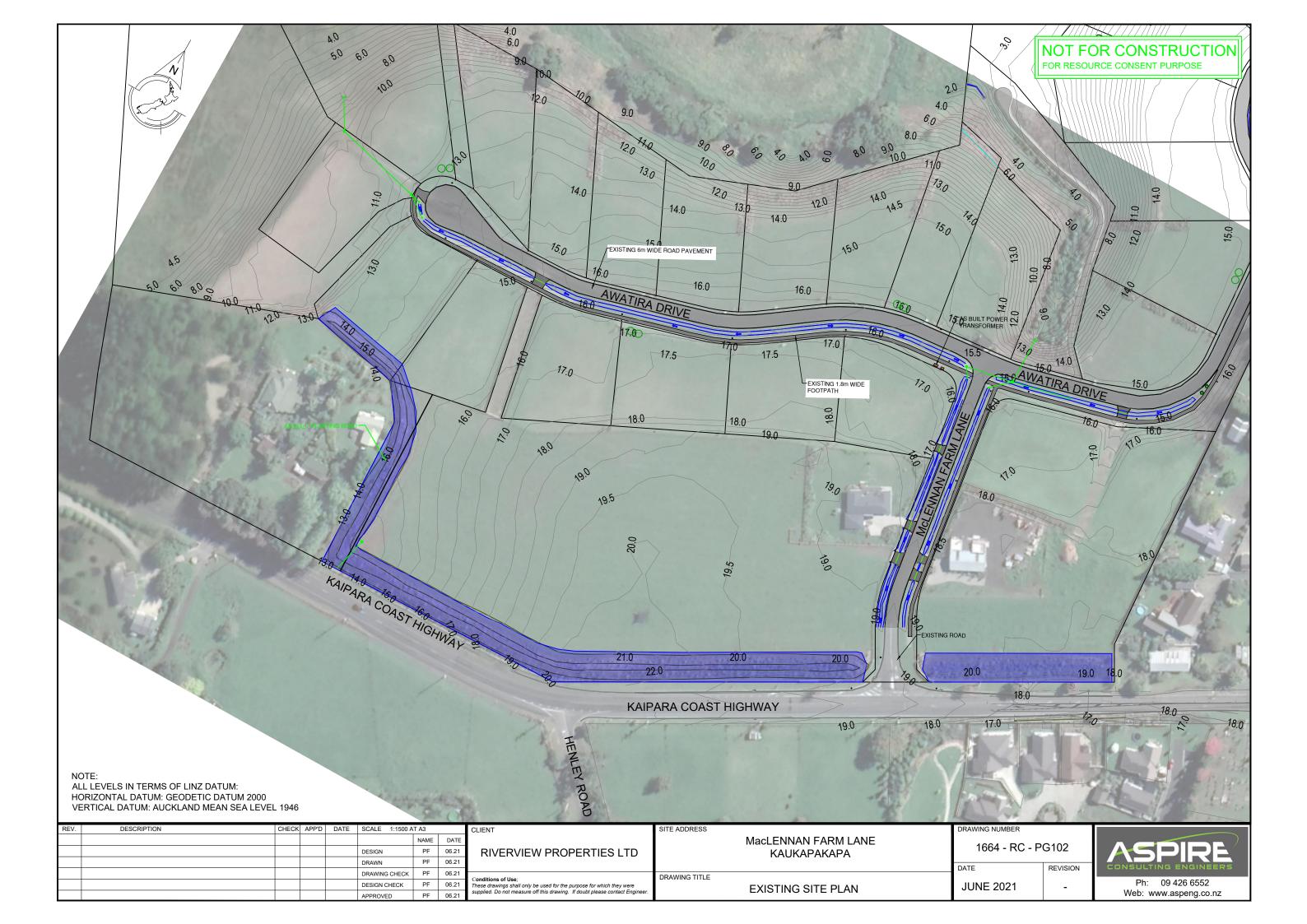


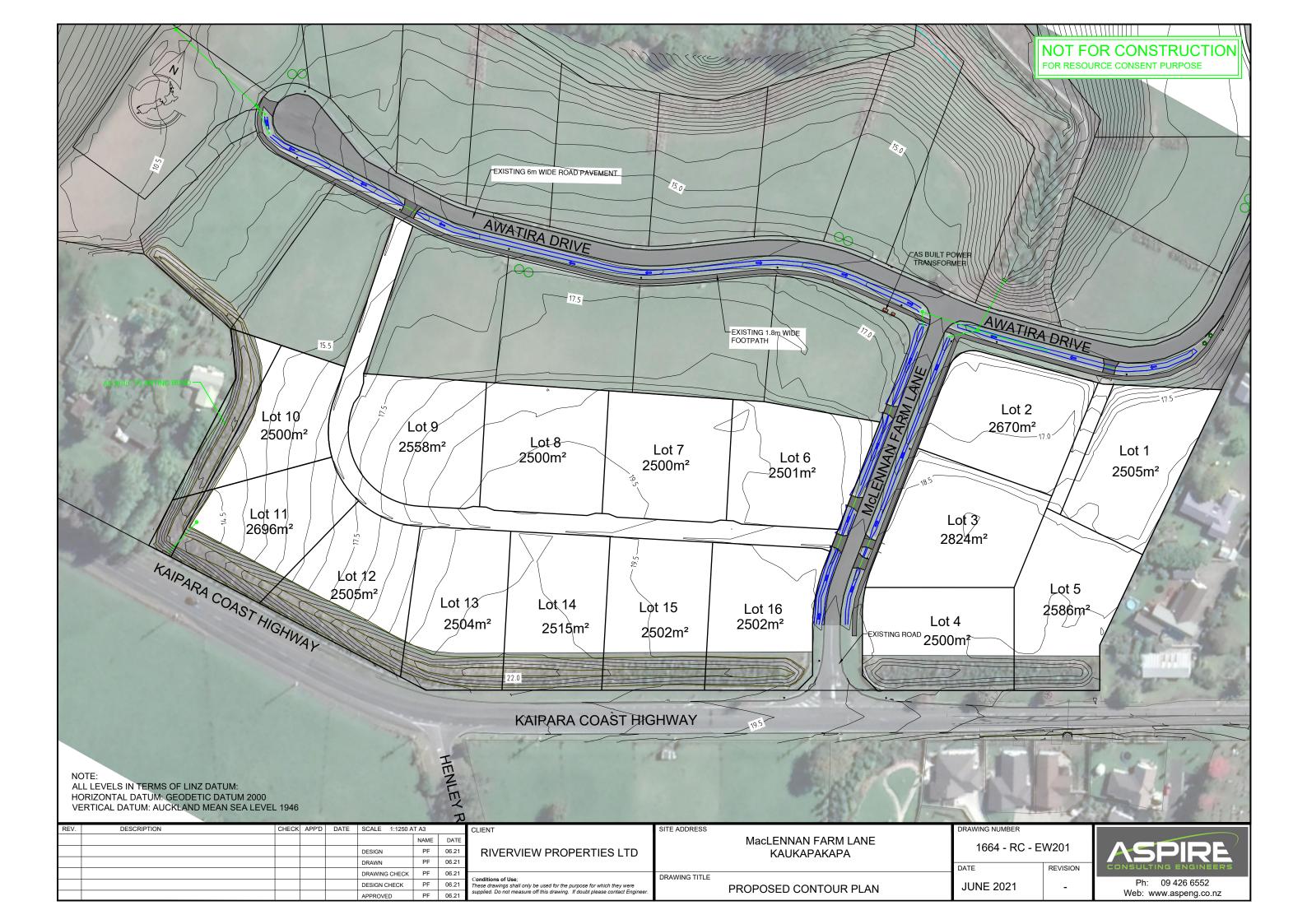
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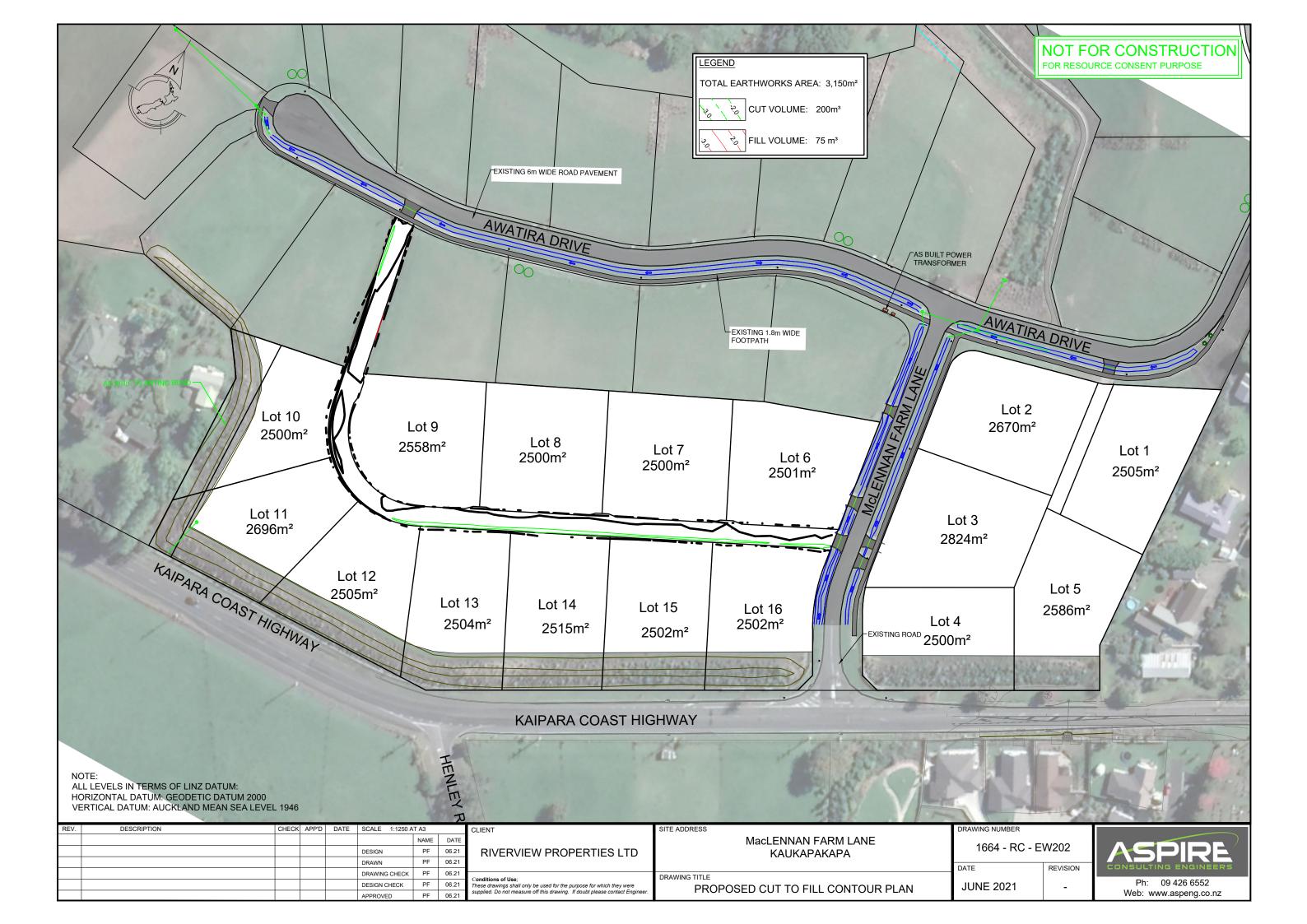
Ph: 09 426 6552

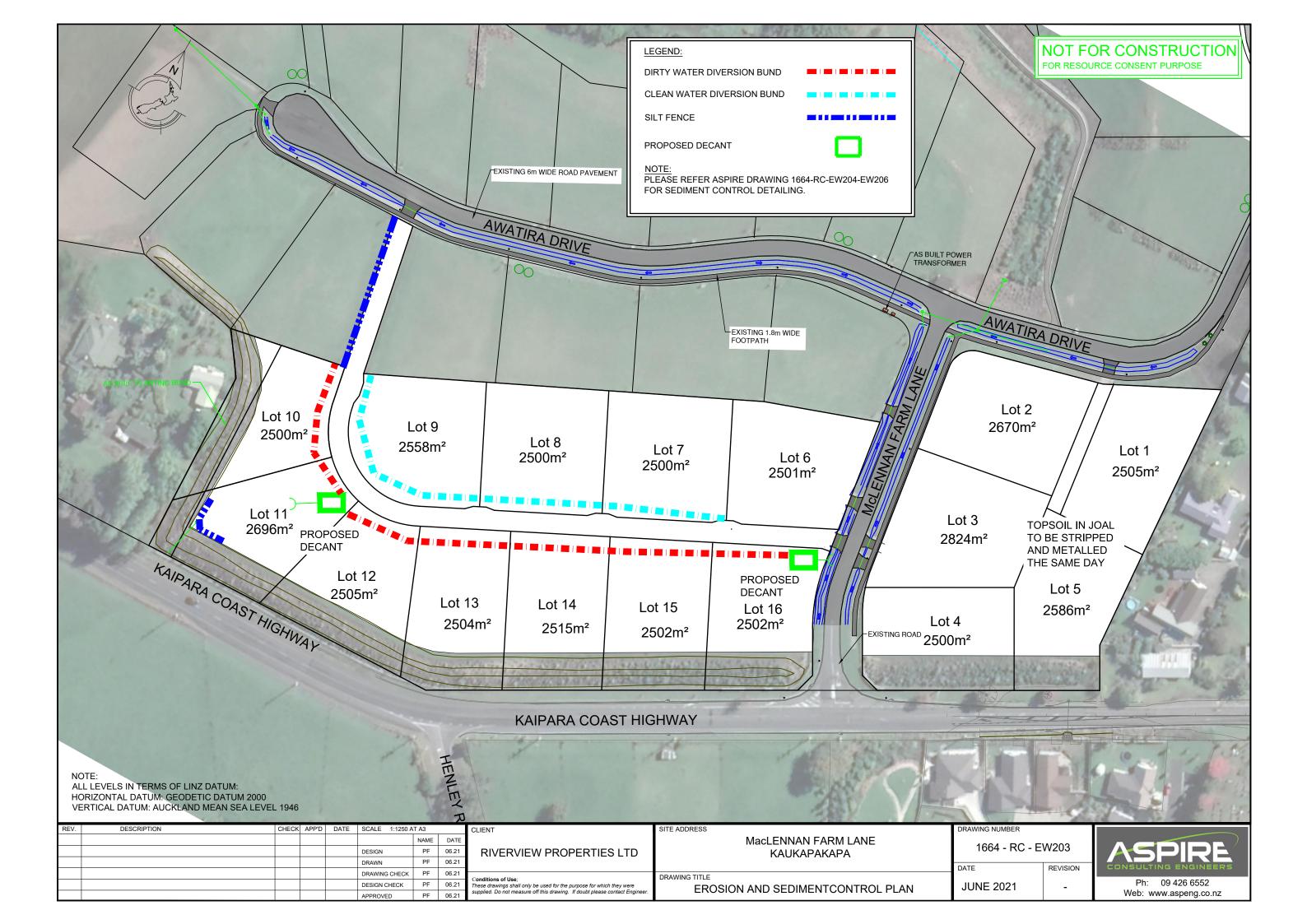
Web: www.aspeng.co.nz

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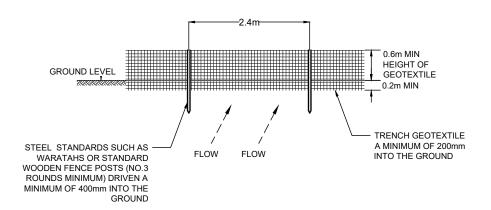




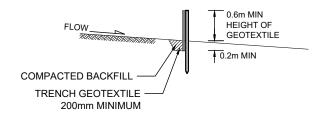


Slope steepness %	Slope length (m) (maximum)	Spacing of returns (m)	Silt fence length (m) (maximum)
Flatter than 2%	Unlimited	N/A	Unlimited
2 – 10%	40	60	300
10 – 20%	30	50	230
20 – 33%	20	40	150
33 – 50%	15	30	75
> 50%	6	20	40

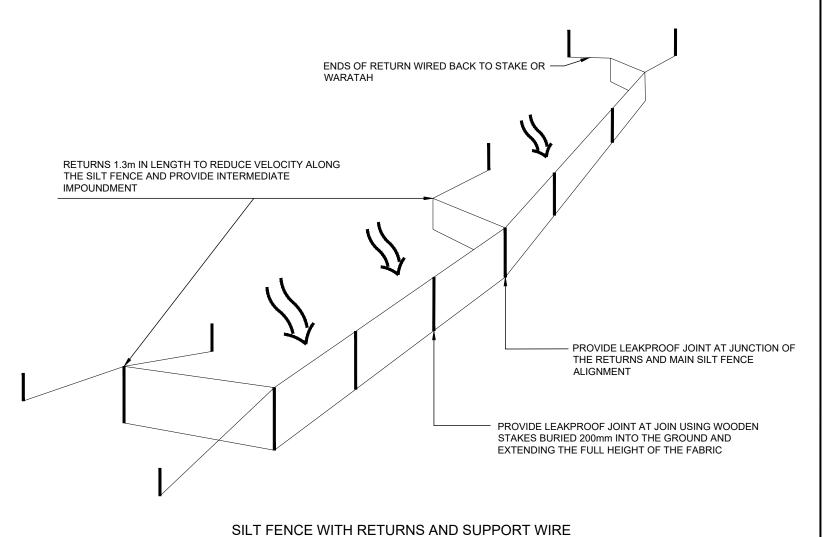
# SILT FENCE DESIGN CRITERIA TABLE



# SILT FENCE ELEVATION



# SILT FENCE CROSS-SECTION



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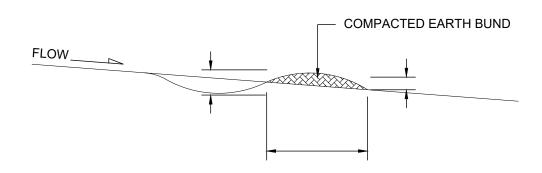
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CONTOUR DRAIN SPACING							
SLOPE OF SITE (%)	SPACING (m) OF CONTOUR DRAINS						
LESS THAN 5%	50						
5 - 10%	40						
10 - 15%	30						
15 - 30%	20						



# CONTOUR DRAIN CROSS-SECTION

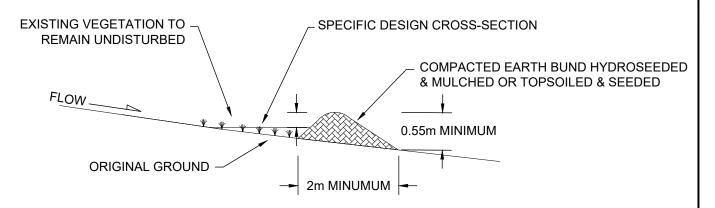
#### DIRTY WATER DIVERSION

FOR CONSTRUCTION AND OPERATION OF DIRTY WATER DIVERSION CHANNELS AND BUNDS:

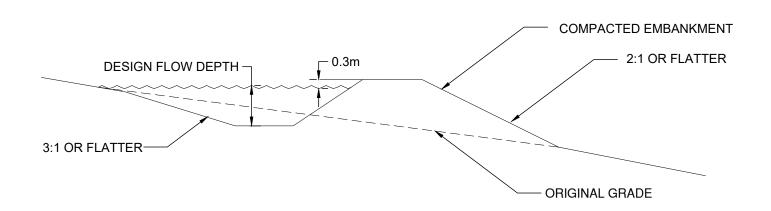
- PLAN AND CONSTRUCT ALL DIRTY WATER DIVERSION WORKS AS PART OF THE INITIAL SITE ESTABLISHMENT/ DEVELOPMENT ACTIVITIES
- DEFINE THE ROUTE AND SURVEY IT TO ACHIEVE THE CORRECT GRADIENT
- CONSTRUCT DRAINS WITH A UNIFORM GRADE ALONG THE INVERT, AS SUDDEN DECREASES MAY CAUSE SEDIMENT TO ACCUMULATE CAUSING THE BANK TO OVERTOP
- ENSURE THE BUNDS ASSOCIATED WITH THE DIVERSIONS ARE WELL COMPACTED AND STABILISED. IN SOME INSTANCES, THIS MAY REQUIRE SPECIFIC GEOTECHNICAL DESIGN TO ENSURE THE STABILITY AND INTEGRITY OF THE STRUCTURE
- MONITOR DIVERSIONS FOR EROSION. SUBJECT TO THE SOILS ON SITE IT IS LIKELY THAT EROSION CONTROL WILL BE NEEDED WHERE THE GRADIENTS ARE GREATER THAN 2% OR WHERE THE DESIGN VELOCITIES EXCEED 1m/SEC
- ENSURE THE FINISHED CROSS-SECTION MEETS ALL DESIGN REQUIREMENTS
- PROVIDE AN ADEQUATE OUTLET FOR EACH DIVERSION (i.e DIRTY WATER TO A SEDIMENT CONTROL DEVICE).

#### NOTE:

- THE DIVERSION CHANNELS SHOULD BE PARABOLIC OR TRAPEZOIDAL IN SHAPE
- ENSURE INTERNAL SIDES OF THE BUND ARE NO STEEPER THAN 3:1, AND EXTERNAL SIDES NO STEEPER THAN 2:1, AS OUTLINED BELOW



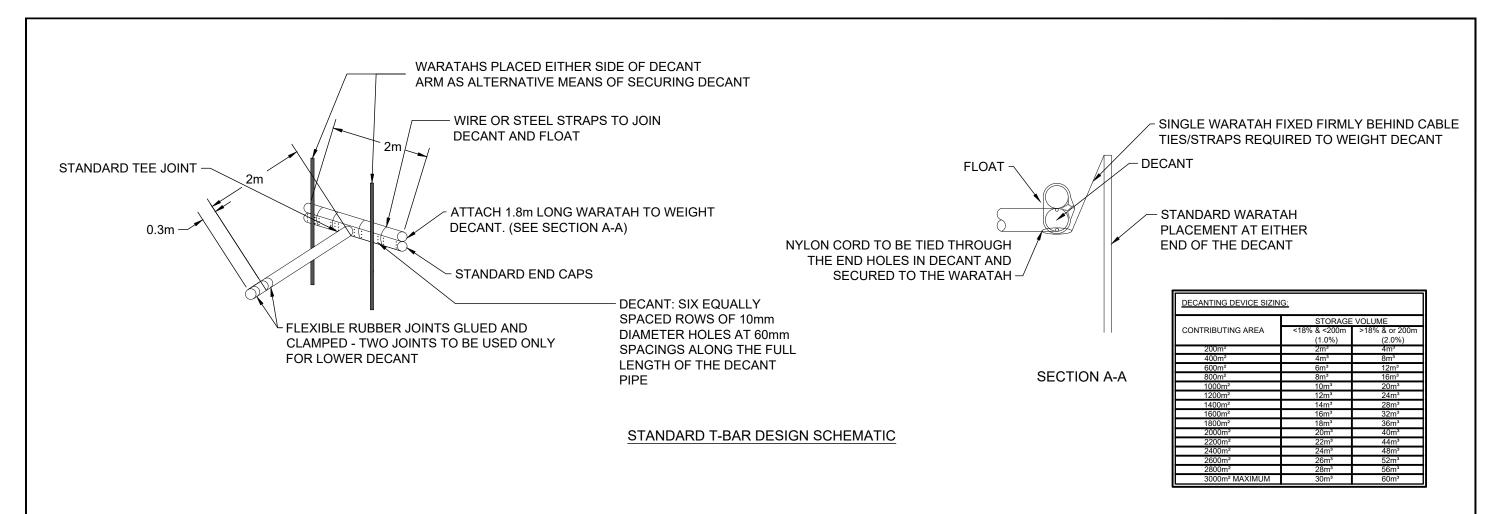
## **CLEAN WATER DIVERSION CROSS-SECTION**

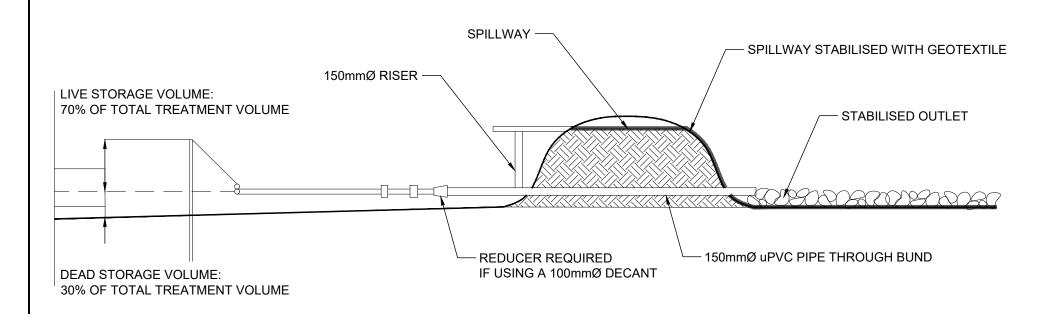


#### DIRTY WATER DIVERSION CROSS-SECTION

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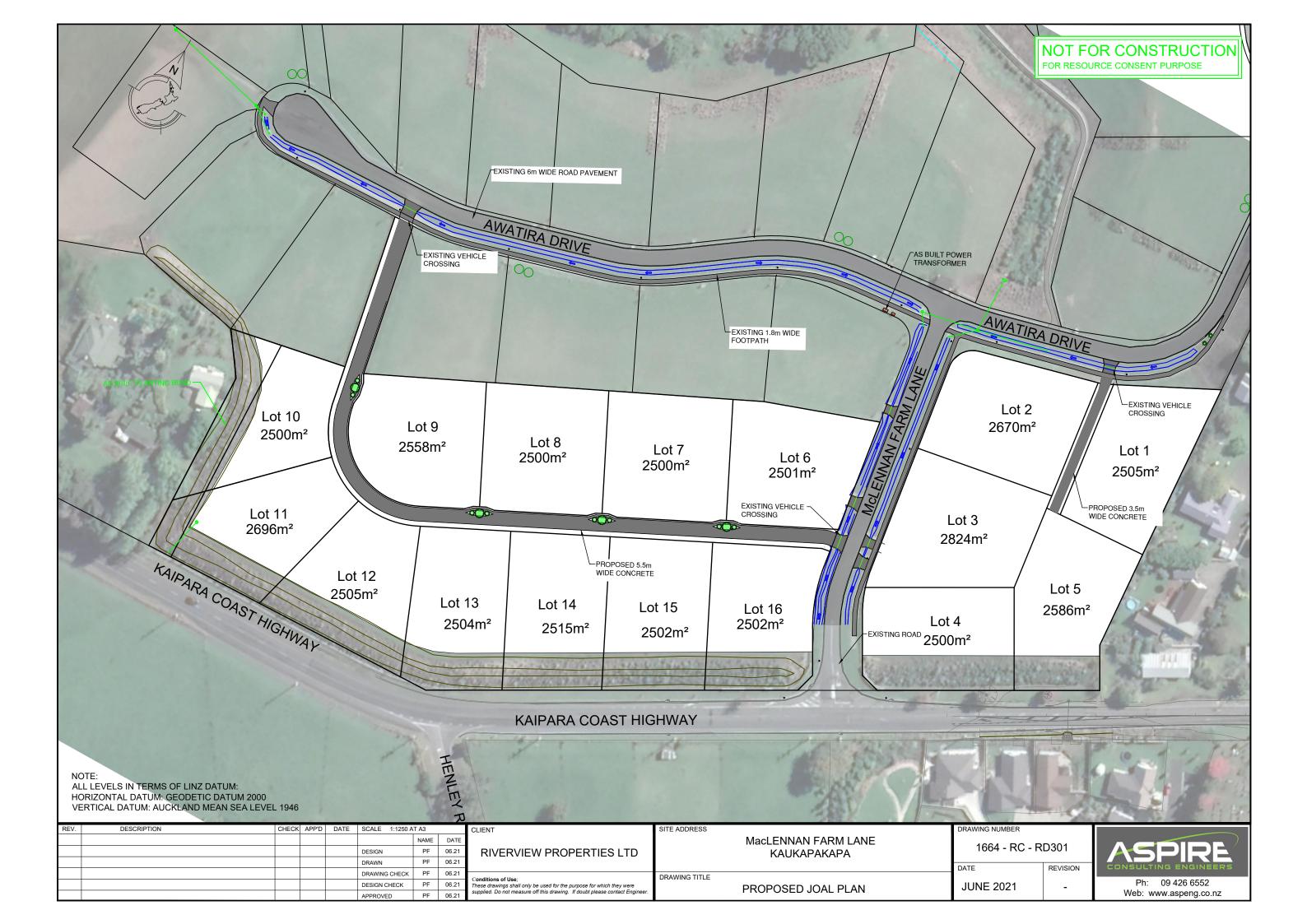
#### **DECANTING EARTH BUND CROSS-SECTION**

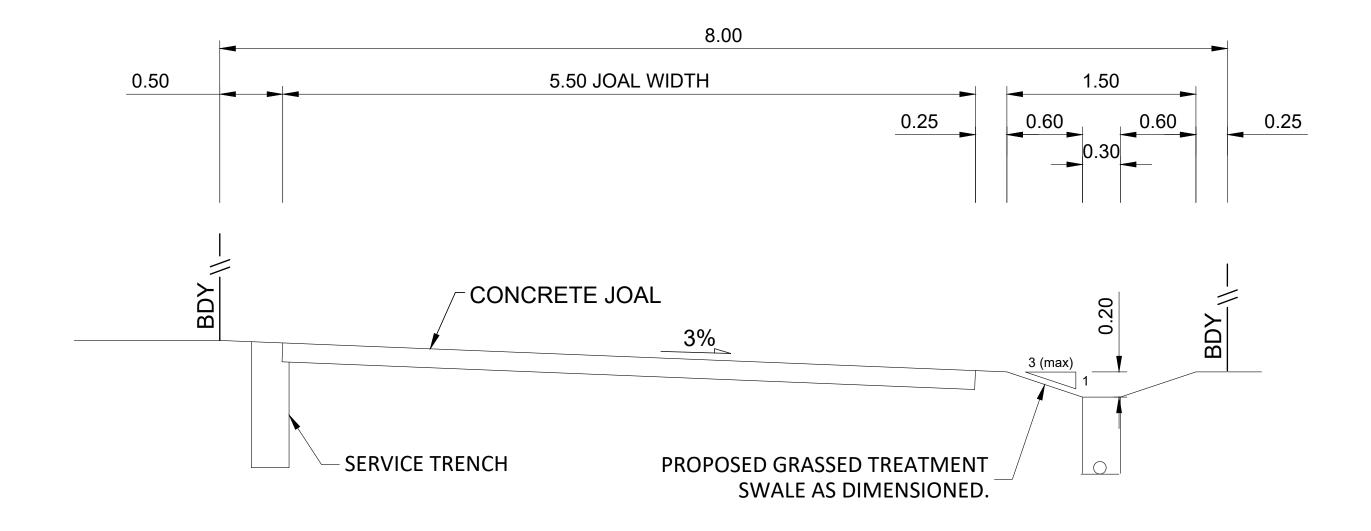
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#### **DEB SIZING:**

- DEB SIZING IS BASED ON CONTRIBUTING CATCHMENT AREA
- ON EARTHWORKS SITES WITH SLOPES LESS THAN 18% AND LESS THAN 200m IN LENGTH, CONSTRUCT DEB WITH A MINIMUM VOLUME OF 1% OF THE CONTRIBUTING CATCHMENT AREA (10m³ FOR EACH 1,000m²).
- ON EARTHWORK SITES WITH SLOPES GREATER THAN 18% OR GREATER THAN 200m IN LENGTH, CONSTRUCT DEB WITH A MINIMUM VOLUME OF 2% OF THE CONTRIBUTING CATCHMENT AREA (20m³ FOR EACH 2,000m²)
- THE SLOPE ANGLE IS DETERMINED BY THE SLOPE IMMEDIATELY ABOVE THE DEB, OR BY THE AVERAGE SLOPE OVER THE CONTRIBUTING CATCHMENT, WHICHEVER IS GREATER. THE SLOPE ANGLE SHOULD ALSO BE THE GREATER OF THE PRE- OR POST- CONSTRUCTION SLOPE.
- THE ABOVE CALCULATION DEFINES THE TOTAL STORAGE VOLUME WHICH IS MEASURED FROM THE BASE OF THE DEB TO THE TOP OF THE PRIMARY SPILLWAY.

Ph: 09 426 6552 Web: www.aspeng.co.nz





# TYPICAL 5.5m JOAL CROSS SECTION - WITH SWALE

NOTE: ALL LEVELS IN TERMS OF LINZ DATUM: HORIZONTAL DATUM: GEODETIC DATUM 2000 VERTICAL DATUM: AUCKLAND MEAN SEA LEVEL 1946

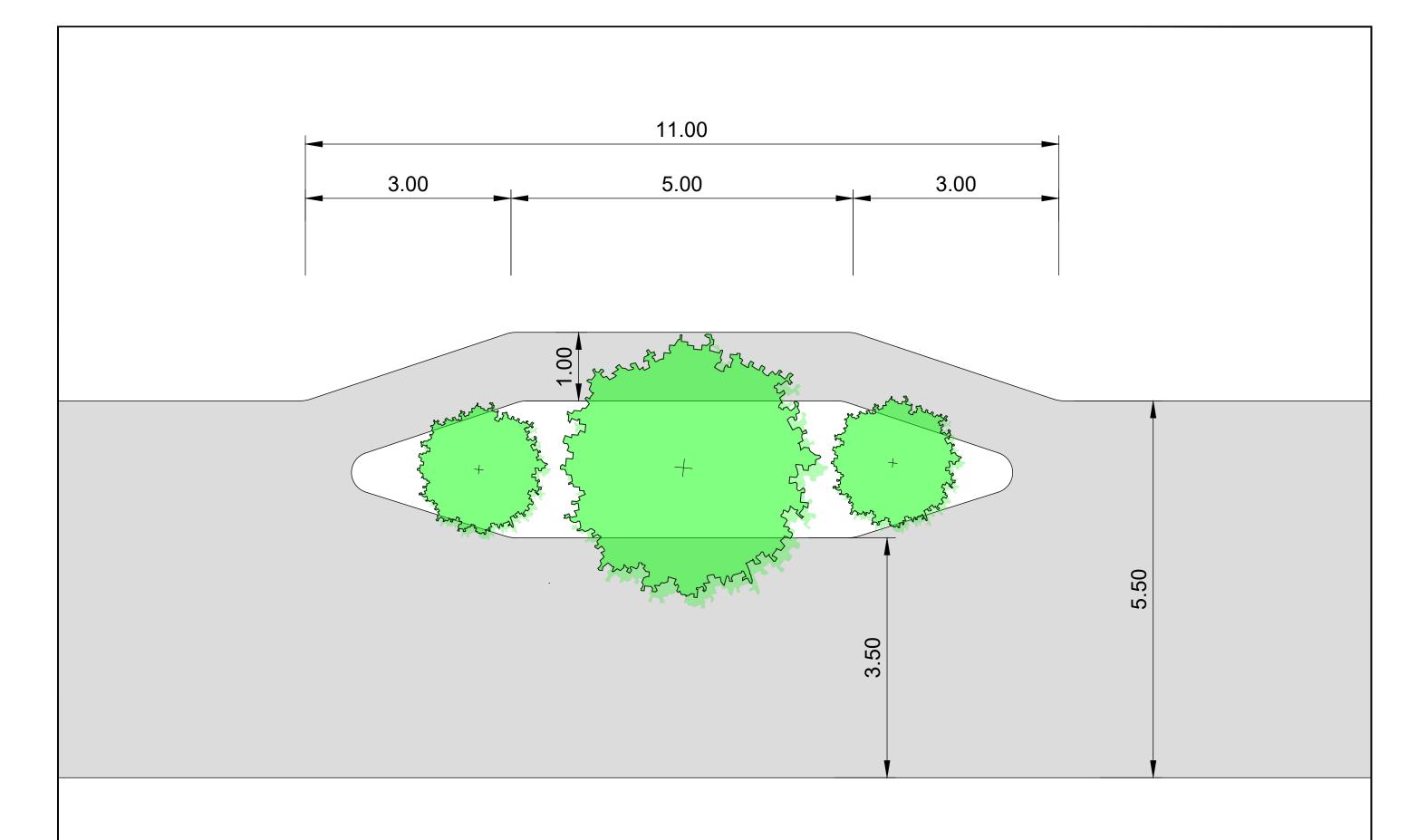
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1664 - RC - RD302

DATE

JUNE 2021



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MacLENNAN FARM LANE KAUKAPAKAPA
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TYPICAL JOAL NARROWING

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