


# STORMWATER MODELLING REPORT

FOR  
PROPOSED WARKWORTH SOUTH  
PLAN CHANGE AREA

 <b>Maven Associates</b>	<b>Job Number</b> 211001		<b>Rev</b> E
<b>Job Title</b> 1711 & 1723 State Highway 1, Warkworth Plan Change <b>Area</b> <b>Title</b> Stormwater Modelling Report	<b>Author</b> YW	<b>Date</b> 18.01.23	<b>Checked</b> AO

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D – TP 108 CALCULATIONS AND TIME OF CONCENTRATION CALCULATIONS

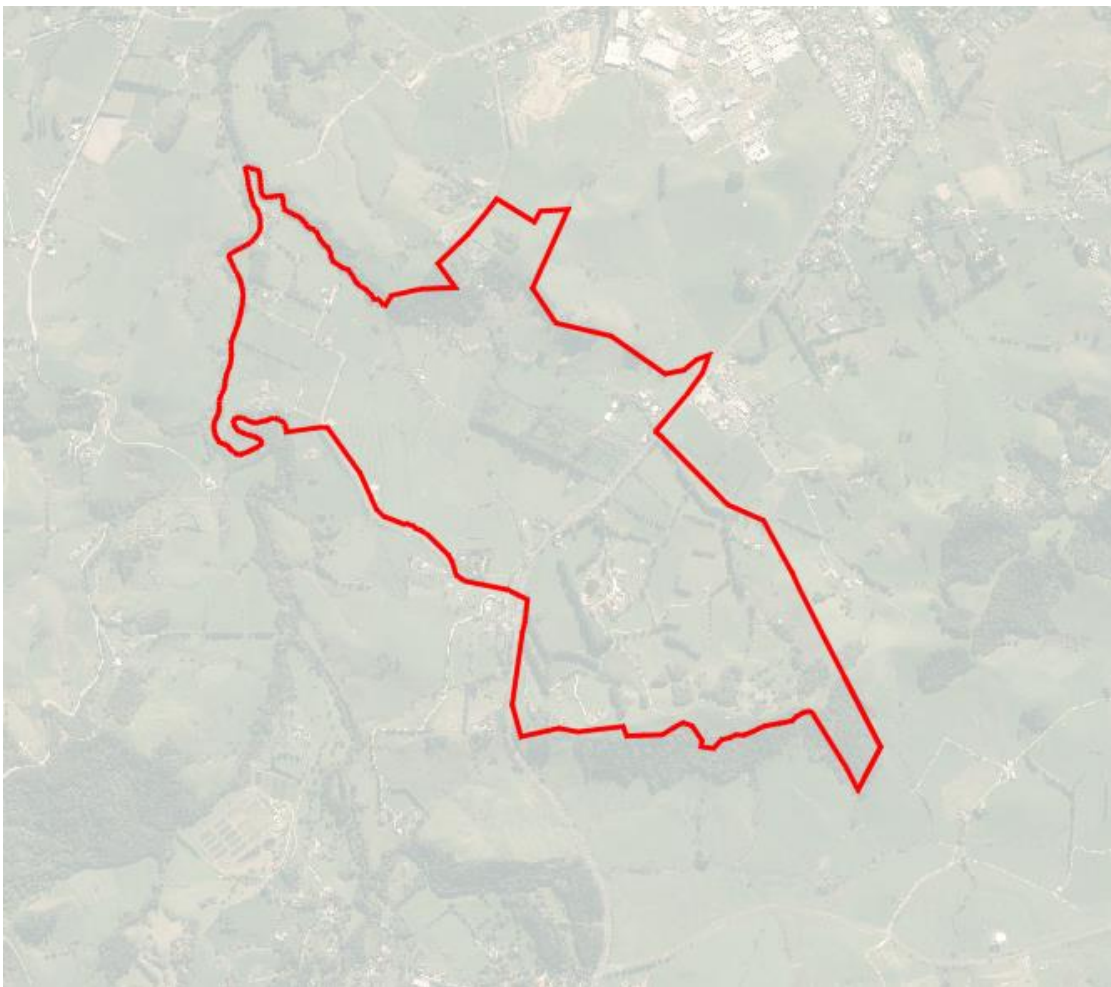
E – WOODCOCK BRIDGE SECTIONS

# 1 INTRODUCTION

## 1.1 PROJECT

Maven Associates have been engaged to assist in the development of a plan change application including determining setting baseline scenarios for predevelopment scenarios in various storm events and assessing the effects of development specific to the proposed plan change area (PCA) at 1711 & 1723 State Highway 1, Warkworth. Figure 1.1 shows the study area.

The objective of this report is to provide a preliminary analysis of the overland flowpaths in terms of peak flows and water level constraints. This will enable the assessment of mitigation measures required to ensure the proposal does not result in any adverse effect on the downstream properties. The analysis will be for a range of annual return period storms and include rainfall increases due to climate change.



**Figure 1.1 – Catchment Delineation**

## 1.2 PREVIOUS STUDY

A Rapid Flood Hazard Assessment was undertaken by DHI in 2009. This was done on a 10m grid. This assessment did not include either climate change or land development changes.

## 1.3 PROPOSED STRATEGY

A 2D model will be used in the area around the Scheme Plan boundary. This will enable the identification of all overland flowpaths. The upper catchment area will be modelled as individual catchments to provide boundary inflows. All analyses will be done using TP108, HEC-HMS and HEC-RAS in accordance with guidelines of the Auckland Council Stormwater Code of Practice.

## 1.4 SCENARIOS MODELLED

Table 1.1 shows the scenarios modelled. Scenarios will indicate the difference between today's existing flow environment and the future.

Scenario	Return period	Land-use	Rainfall
1	100-year	Existing	Existing - historical
2	100-year	Existing	Climate change
3	100-year	Developed	Climate change
4	50-year	Developed	Climate change
5	20-year	Developed	Climate change
7	10-year	Developed	Climate change

*Table 1.1 – Scenarios modelled*

## 1.5 SOURCES OF DATA

Attribute	Organisation
Catchment Plans	Auckland Council Geomaps
Contours	LINZ DEM 1m. The Terrain datum is New Zealand Vertical Datum. LiDAR/Site Survey by Parrallax Ltd. LiDAR/Site Survey by Maven Associates.
Flow & WL data	Healthywaters
Flood level evidence	None

*Table 1.2 – Source of Data*



## 1.6 REFERENCE TECHNICAL DOCUMENTS

- AUCKLAND COUNCIL CODE OF PRACTICE FOR LAND DEVELOPMENT AND SUBDIVISION. CHAPTER4 – STORMWATER, VERSION 3.00
- ACCEPTABLE SOLUTIONS AND VERIFIABLE METHODS, DOCUMENT E1 SURFACE WATER, MINISTRY OF BUSINESS, INNOVATION AND EMPLOYMENT,
- AUCKLAND COUNCIL TP108

## 2 HYDROLOGICAL MODELLING WITH HEC-HMS

### 2.1 METHODOLOGY

The analysis was done using the following steps:

1. Delineate the catchments,
2. Use Tp108 to calculate parameters,
3. Use HEC-HMS to create a rainfall hyetograph and flow hydrographs,

### 2.2 RAINFALL DATA

TP108 gives the following rainfall depths which are then adjusted for climate change as shown in Table 2.2. Climate change factor have been applied in accordance with Auckland Council code of practice (Version 3) assuming a 2.1°C increase in temperature as shown below;

Annual Exceedance probability exceedance	Percentage Increase in 24-hour design rainfall depth due to future climate change*
50%	9.0%
10%	13.2%
5%	15.1%
2%	16.8%
1%	16.8%

\* Assuming 2.1°C increase in temperature

*Table 2.1 - Climate change factors*

In accordance with TP108 section 2.3 an areal reduction factor (ARF) has been applied as the catchment has an area above 10 km<sup>2</sup>. ARF adjusted rainfalls are also shown in table 2.2. An ARF factor of 0.92 was used per TP108 table 2.2.

	TP108	Climate change	ARF adjusted
2-year	112	122	112
10-year	170	192	176
20-year	208	239	220
50-year	238	278	256
100-year	270	315	290

*Table 2.2 – Rain depths*

## 2.3 CATCHMENT SIZE

Figure 2.1 shows the catchment area modelled. Naming conventions of the subcatchments have been split between upstream and downstream of the PCA. The upper catchments are named upstream A to F and downstream catchments, downstream A to H. The yellow area (including the red boundary) is the 2D grid with the excess *Rain*. The catchment outflow of the Mahurangi River is at the northern edge. The Scheme Plan boundary is blue. The total area is 49km<sup>2</sup>.

## 2.4 LAND-USE AND SOILS

The soil is assumed to be Group C with a curve number of 74. The land cover for the existing scenario has been obtained via delineation of impervious areas shown on the Auckland Council GIS aerial. The land-use is predominantly Rural-production and Rural-coastal with a small area of conservation, according to the AUP, see Figure 2.2. For the proposed scenario, the MPD (maximum probable development) of the proposed zoning has been used as well as MPD for the yellow designated FutureUrban. The FutureUrban zoning included in the developed scenarios assumes an average impervious area of 60%. The combined curve numbers and initial abstractions have been calculated according to TP108 and may be found in appendix D based on existing and developed land-use. Only catchments *Rain* and upstream catchment *F* will have a change in impervious area.

The full TP108 details to calculate the peak flows and times of concentration may be found in Appendix D. The total catchment area and the time of concentration suggests an area reduction factor of 0.92. This has been applied to the rainfall as per Table 2.2. This data can now be inserted into a HEC-HMS model.

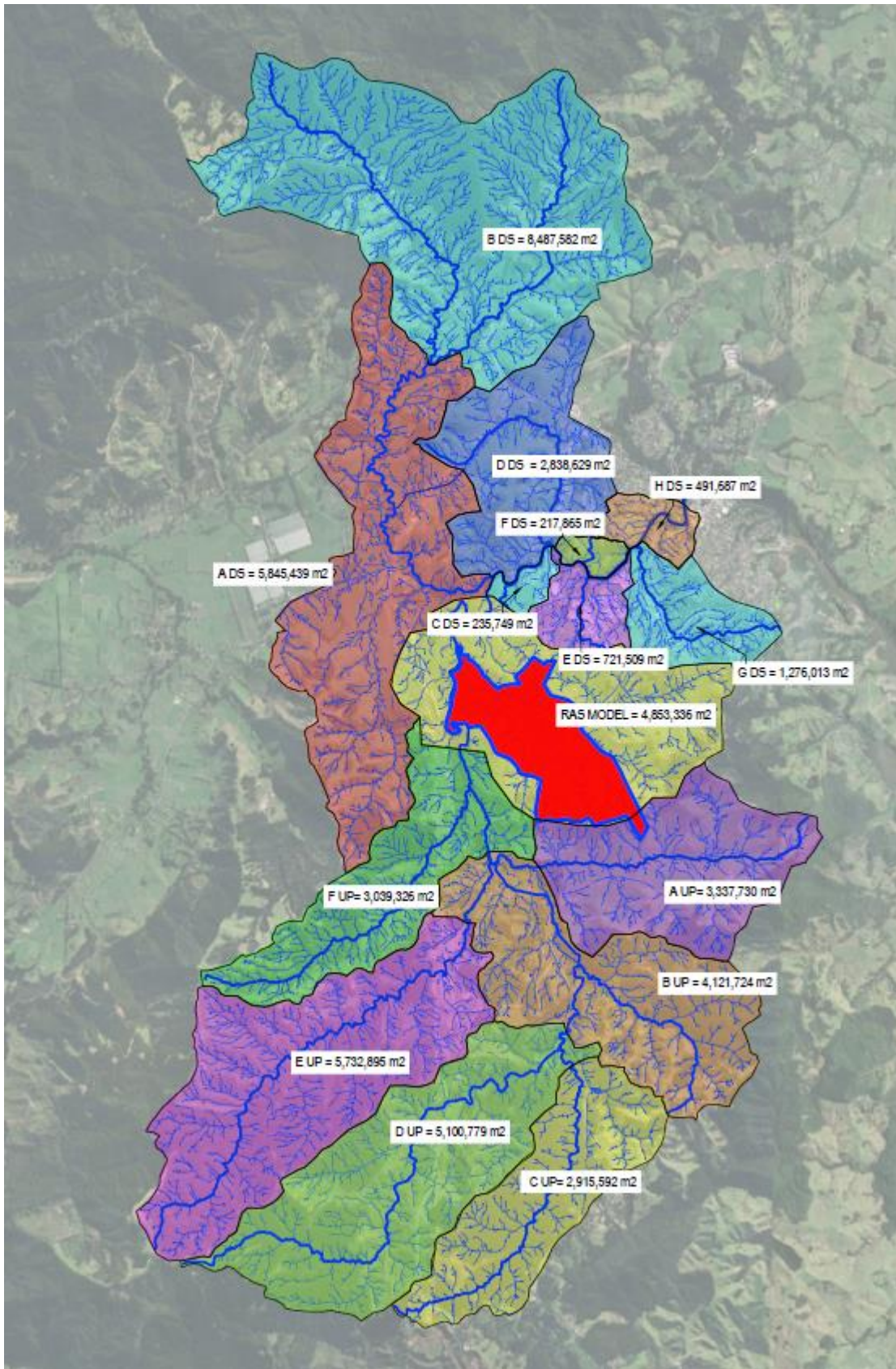


Figure 2.1 – Catchment Boundary



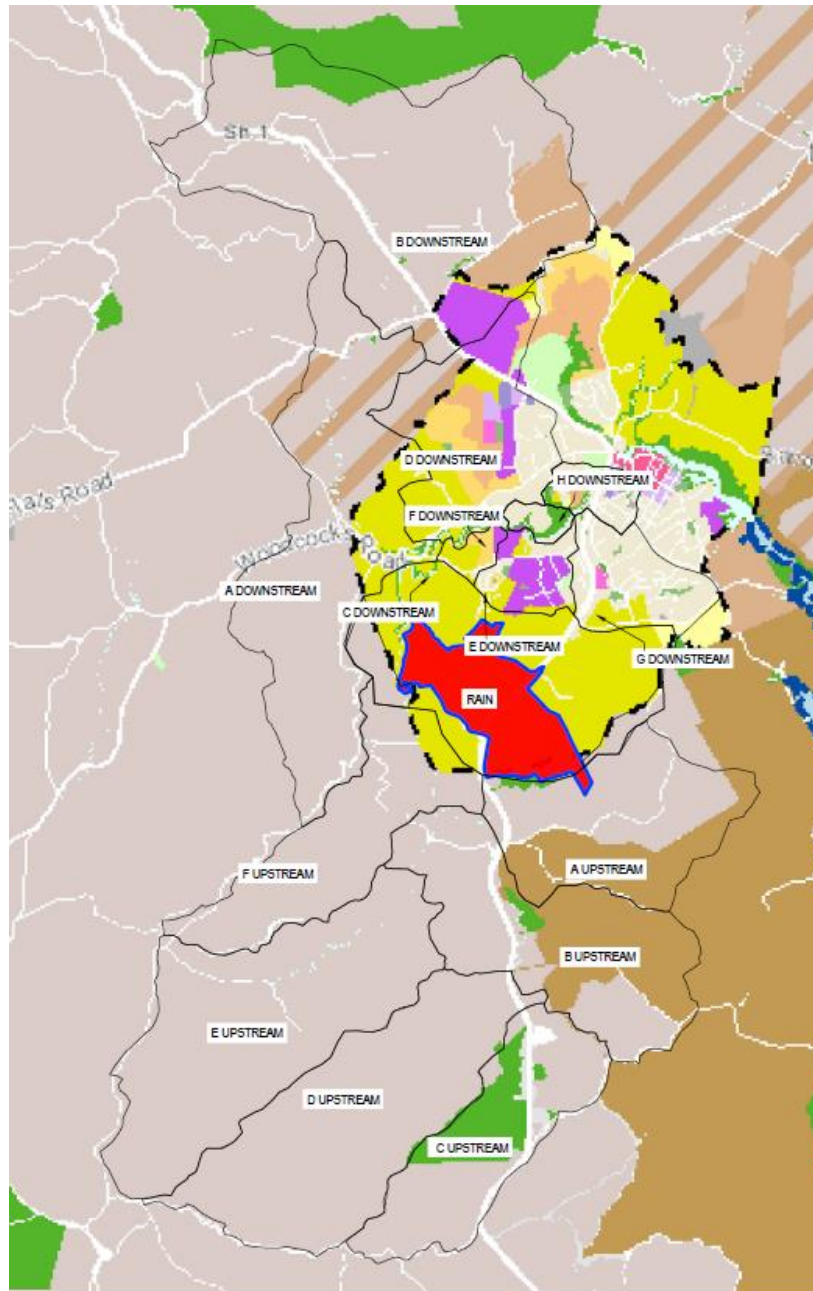


Figure 2.2 – Land-use zones

	Upstream of PCA								Downstream of PCA							
	Impervious %	Rain	A	B	C	D	E	F	A	B	C	D	E	F	G	H
Total		485	334	412	292	510	573	304	585	849	24	284	72	22	128	49
Rural / Vegetated area	1%	471	331	405	291	481	572	302	581	845	12	276	27	22	15	10
Urban MPD	60%	0	0	0	0	0	0	0	0	0	12	5	0	0	113	39
Open Space - Conservation	1%	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Transport Corridor	90%	13	3	7	1	0	0	2	4	4	0	3	0	0	0	0
Residential - Large Lot	35%	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Residential - Single House	60%	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Residential - Mixed Housing Urban	60%	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Residential - Terrace & Apartment	70%	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Business - Local Centre Zone	100%	0	0	0	0	0	0	0	0	0	0	0	45	0	0	0
<b>Impervious</b>		16.42	6.01	10.35	3.81	4.81	5.72	4.82	9.41	12.05	7.32	8.46	45.27	0.22	67.71	23.60
<b>Pervious</b>		468.91	327.76	401.82	287.75	505.27	567.57	299.11	575.13	836.71	16.25	275.40	26.88	21.57	59.89	25.57

Figure 2.3 – Existing land use calculations

	Upstream of PCA								Downstream of PCA							
	Impervious %	Rain	A	B	C	D	E	F	A	B	C	D	E	F	G	H
Total		485	334	412	292	510	573	304	585	849	24	284	72	22	128	49
Rural / Vegetated area	1%	51	331	405	291	481	572	285	581	845	12	276	27	22	15	10
Urban MPD to	60%	276	0	0	0	0	0	17	0	0	12	5	0	0	113	39
Open Space - Conservation	1%	5.7	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Transport Corridor	90%	13.0	3	7	1	0	0	2	4	4	0	3	0	0	0	0
Residential - Large Lot	35%	13.9	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Residential - Single House	60%	22.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Residential - Mixed Housing Urban	60%	74.2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Residential - Terrace & Apartment	70%	25.4	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Business - Local Centre Zone	100%	3.4	0	0	0	0	0	0	0	0	0	0	45	0	0	0
Impervious		261.76	6.01	10.35	3.81	4.81	5.72	15.10	9.41	12.05	7.32	8.46	45.27	0.22	67.71	23.60
Pervious		223.57	327.76	401.82	287.75	505.27	567.57	288.83	575.13	836.71	16.25	275.40	26.88	21.57	59.89	25.57

Figure 2.4 – Developed land use calculations

## 2.5 HEC-HMS MODEL

The data was then transferred to HEC-HMS. Figure 2.5 shows the model set-up. Calculations for the time of concentration of the reaches may be found in Appendix D. The reaches between junctions have been incorporated respectively to the time it would take to arrive at the downstream connection.

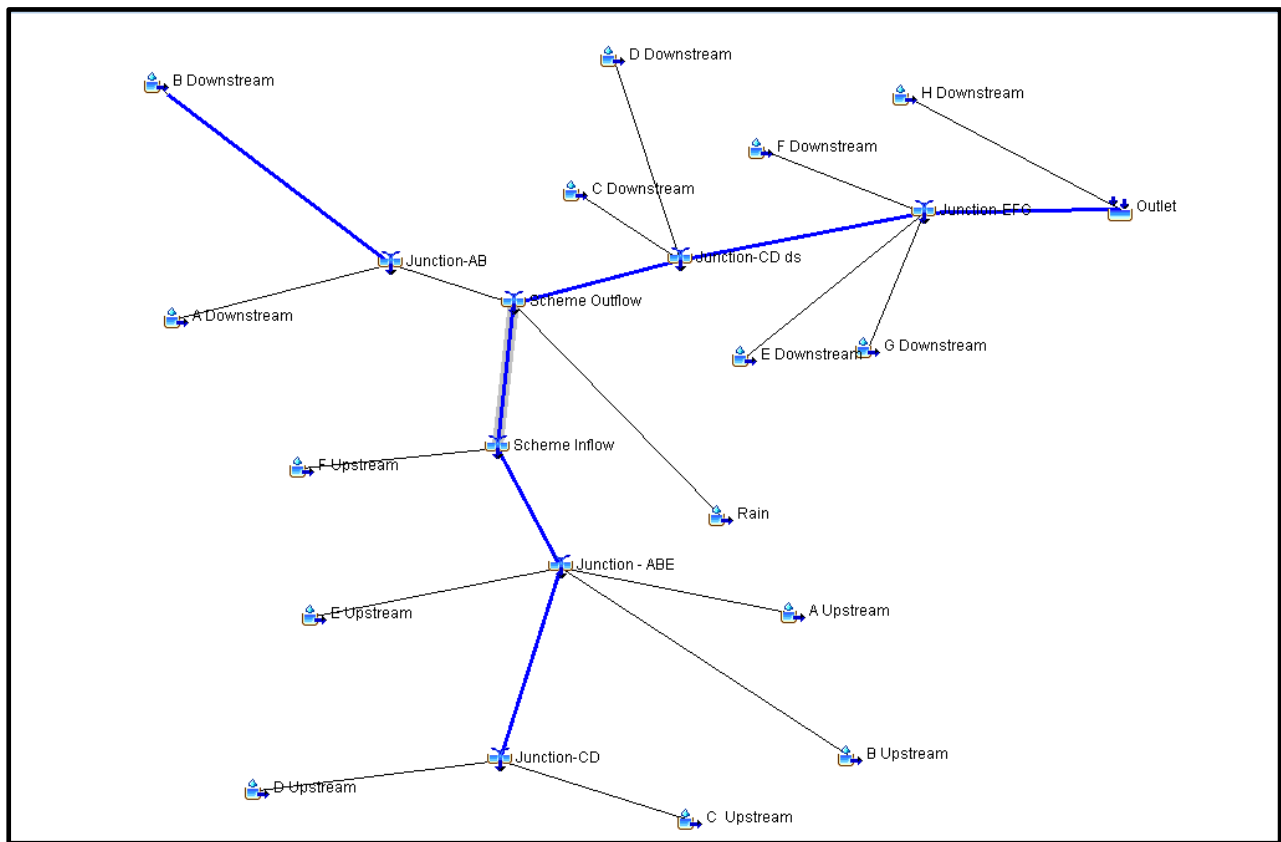


Figure 2.5 – HEC-HMS model set-up

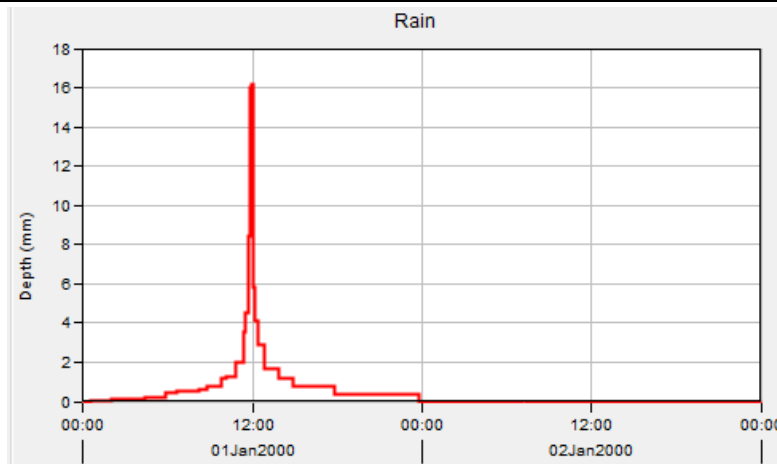


Figure 2.6 shows the 100-year developed land-use rainfall hyetograph for the grid.

**Figure 2.6 – Rainfall excess, 100-year, climate change, developed**

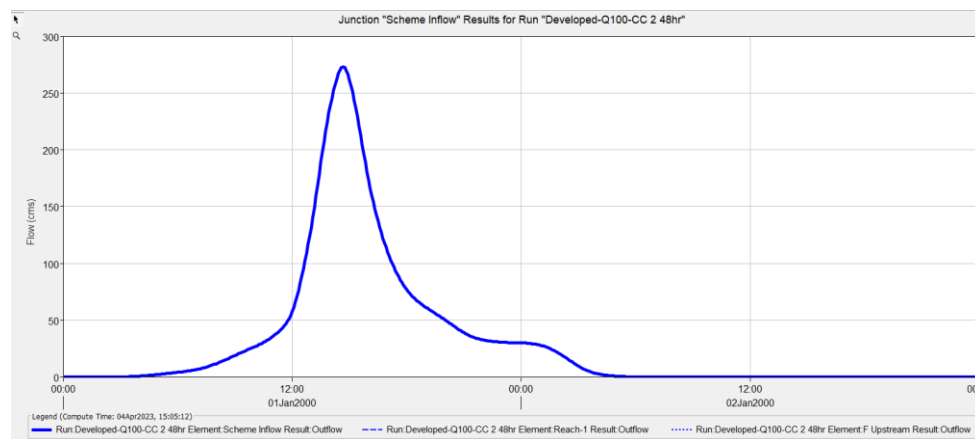


Figure 2.7 shows the hydrograph for scheme inflow for the 100-year storm with climate change rain and developed.

**Figure 2.7 – Flow hydrograph, 100-year, climate change, developed**

### 2.5.1 Effects of climate change

Figure 2.8 shows the global summary of the existing catchment flows against those that are expected to occur due to climate change and development. The scheme inflow has increased from 224m<sup>3</sup>/s to 273m<sup>3</sup>/s. Most of the 49m<sup>3</sup>/s increase is due to climate change. The volume increase is almost 0.91 million m<sup>3</sup>.

At the scheme outflow the changes are 347m<sup>3</sup>/s to 421m<sup>3</sup>/s. Thus, the catchment is expected to yield 74m<sup>3</sup>/s, (this entire increase is due to climate change as explained in section 2.5.2). This increase is 17%. The volume increase is 1.6 million m<sup>3</sup>.



Figure 2.8 – Global summary of flows and volumes for the 100-year storm (historical rain, existing land-use vs climate change rain and existing land-use vs. historical rain, proposed land-use vs climate changed rain and developed)

Global Summary Results for Run "Existing-Q100-existing 3"

Project: Warkworth South Simulation Run: Existing-Q100-existing 3

Start of Run: 01Jan2000, 00:00 Basin Model: Existing Land-Use  
 End of Run: 02Jan2000, 00:00 Meteorologic Model: 100yr-existing-existing-land  
 Compute Time: 04Apr2023, 11:34:28 Control Specifications: 24hr (Maven)

Show Elements: All Elements Volume Units:  MM  1000 M3 Sorting:

Hydrologic Element	Drainage Area (KM2)	Peak Discharge (M3/S)	Time of Peak	Volume (1000 M3)
A Downstream	5.8454	55.71354	01Jan2000, 13:50	999.18175
A Upstream	3.3380	36.86217	01Jan2000, 13:20	576.96580
B Downstream	8.4876	91.34174	01Jan2000, 13:25	1462.51227
B Upstream	4.1220	46.67449	01Jan2000, 13:20	715.49697
C Downstream	0.2357	5.01350	01Jan2000, 12:25	46.24902
C Upstream	2.9156	35.38431	01Jan2000, 13:10	506.20497
D Downstream	2.8386	36.46248	01Jan2000, 13:00	496.99865
D Upstream	5.1008	56.15127	01Jan2000, 13:20	878.69326
E Downstream	0.7215	16.17126	01Jan2000, 12:25	156.27172
E Upstream	5.7329	58.75500	01Jan2000, 13:35	982.55892
F Downstream	0.2179	4.58160	01Jan2000, 12:20	38.57723
F Upstream	3.0393	38.64703	01Jan2000, 13:00	529.75106
G Downstream	1.2760	25.09295	01Jan2000, 12:30	267.38809
H Downstream	0.4917	11.32758	01Jan2000, 12:20	101.86108
Junction - ABE	21.2093	208.34781	01Jan2000, 13:55	3628.33498
Junction-AB	14.3330	132.15756	01Jan2000, 14:50	2411.57944
Junction-CD	8.0164	90.79421	01Jan2000, 13:15	1384.89822
Junction-CD ds	46.5092	354.71174	01Jan2000, 16:05	7668.43491
Junction-EFG	48.7246	359.38568	01Jan2000, 16:45	8008.70568
Outlet	49.2163	360.41859	01Jan2000, 17:25	7969.16268
Rain	4.8533	54.41783	01Jan2000, 13:20	844.55754
Reach-1	21.2093	208.07416	01Jan2000, 14:45	3564.45241
Reach-2	8.0164	90.70163	01Jan2000, 14:20	1353.31328
Reach-3	8.4876	91.30646	01Jan2000, 15:00	1412.39769
Reach-4	24.2486	224.27160	01Jan2000, 15:55	3978.68919
Reach-5	43.4349	346.41930	01Jan2000, 16:10	7125.18723
Reach-6	46.5092	354.68892	01Jan2000, 16:45	7546.46865
Reach-7	48.7246	359.38568	01Jan2000, 17:25	7867.30160
Scheme Inflow	24.2486	224.27160	01Jan2000, 14:40	4094.20347
Scheme Outflow	43.4349	346.50079	01Jan2000, 15:30	7234.82617

Global Summary Results for Run "Existing-Q100-existing-CC 3"

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Project: Warkworth South Simulation Run: Existing-Q100-existing-CC 3

Start of Run: 01Jan2000, 00:00 Basin Model: Existing Land-Use  
End of Run: 02Jan2000, 00:00 Meteorologic Model: 100yr-CC-existing-land-use  
Compute Time: 04Apr2023, 11:25:23 Control Specifications: 24hr (Maven)

Show Elements: All Elements ▾

Volume Units:  MM  1000 M3

Sorting:  ▾

Hydrologic Element	Drainage Area (KM2)	Peak Discharge (M3/S)	Time of Peak	Volume (1000 M3)
A Downstream	5.8454	67.89074	01Jan2000, 13:50	1220.76543
A Upstream	3.3380	44.93661	01Jan2000, 13:20	704.70863
B Downstream	8.4876	111.35746	01Jan2000, 13:25	1786.68282
B Upstream	4.1220	56.84513	01Jan2000, 13:20	873.60158
C Downstream	0.2357	6.01131	01Jan2000, 12:25	55.75929
C Upstream	2.9156	43.11220	01Jan2000, 13:10	618.28270
D Downstream	2.8386	44.41328	01Jan2000, 13:00	606.60934
D Upstream	5.1008	68.48363	01Jan2000, 13:20	1073.67741
E Downstream	0.7215	19.12037	01Jan2000, 12:25	186.00455
E Upstream	5.7329	71.63279	01Jan2000, 13:35	1200.75451
F Downstream	0.2179	5.57805	01Jan2000, 12:20	47.09980
F Upstream	3.0393	47.10883	01Jan2000, 13:00	646.89674
G Downstream	1.2760	29.80892	01Jan2000, 12:30	319.54365
H Downstream	0.4917	13.48195	01Jan2000, 12:20	121.97268
Junction - ABE	21.2093	253.81225	01Jan2000, 13:55	4433.46367
Junction-AB	14.3330	160.93311	01Jan2000, 14:45	2947.84554
Junction-CD	8.0164	110.71134	01Jan2000, 13:15	1691.96012
Junction-CD ds	46.5092	431.66357	01Jan2000, 16:05	9377.50515
Junction-EFG	48.7246	437.12556	01Jan2000, 16:45	9785.00179
Outlet	49.2163	438.34159	01Jan2000, 17:25	9738.70730
Rain	4.8533	66.26538	01Jan2000, 13:20	1030.78437
Reach-1	21.2093	253.46225	01Jan2000, 14:40	4357.48934
Reach-2	8.0164	110.56449	01Jan2000, 14:20	1654.39895
Reach-3	8.4876	111.29836	01Jan2000, 15:00	1727.08011
Reach-4	24.2486	273.06652	01Jan2000, 15:55	4866.95901
Reach-5	43.4349	421.57056	01Jan2000, 16:05	8715.13652
Reach-6	46.5092	431.59631	01Jan2000, 16:45	9232.35379
Reach-7	48.7246	437.12556	01Jan2000, 17:25	9616.73461
Scheme Inflow	24.2486	273.06652	01Jan2000, 14:40	5004.38608
Scheme Outflow	43.4349	421.69390	01Jan2000, 15:30	8845.58892

Project: Warkworth South Simulation Run: Developed-Q100

Start of Run: 01Jan2000, 00:00 Basin Model: Developed Land-Use  
 End of Run: 02Jan2000, 00:00 Meteorologic Model: 100yr-existing-existing-land  
 Compute Time: 05Apr2023, 15:26:21 Control Specifications: 24hr (Maven)

Show Elements: All Elements ▾

Volume Units:  MM  1000 M3

Sorting: Alphabetic ▾

Hydrologic Element	Drainage Area (KM2)	Peak Discharge (M3/S)	Time of Peak	Volume (1000 M3)
A Downstream	5.8454	55.71354	01Jan2000, 13:50	999.18175
A Upstream	3.3380	36.86217	01Jan2000, 13:20	576.96580
B Downstream	8.4876	91.34174	01Jan2000, 13:25	1462.51227
B Upstream	4.1220	46.67449	01Jan2000, 13:20	715.49697
C Downstream	0.2357	5.01350	01Jan2000, 12:25	46.24902
C Upstream	2.9156	35.38431	01Jan2000, 13:10	506.20497
D Downstream	2.8386	36.46248	01Jan2000, 13:00	496.99865
D Upstream	5.1008	56.15127	01Jan2000, 13:20	878.69326
E Downstream	0.7215	16.17126	01Jan2000, 12:25	156.27172
E Upstream	5.7329	58.75500	01Jan2000, 13:35	982.55892
F Downstream	0.2179	4.58160	01Jan2000, 12:20	38.57723
F Upstream	3.0393	39.32813	01Jan2000, 13:00	536.15710
G Downstream	1.2760	25.09295	01Jan2000, 12:30	267.38809
H Downstream	0.4917	11.32758	01Jan2000, 12:20	101.86108
Junction - ABE	21.2093	208.34781	01Jan2000, 13:55	3628.33498
Junction-AB	14.3330	132.15756	01Jan2000, 14:50	2411.57944
Junction-CD	8.0164	90.79421	01Jan2000, 13:15	1384.89822
Junction-CD ds	46.5092	354.24160	01Jan2000, 16:05	7835.33315
Junction-EFG ds	48.7246	358.90511	01Jan2000, 16:45	8174.93964
Outlet	49.2163	359.93801	01Jan2000, 17:25	8134.74845
Rain	4.8533	68.19996	01Jan2000, 13:10	1005.79512
Reach-1	21.2093	208.07416	01Jan2000, 14:45	3564.45241
Reach-2	8.0164	90.70163	01Jan2000, 14:20	1353.31328
Reach-3	8.4876	91.30646	01Jan2000, 15:00	1412.39769
Reach-4	24.2486	224.28327	01Jan2000, 15:55	3985.02675
Reach-5	43.4349	345.89698	01Jan2000, 16:10	7292.08547
Reach-6	46.5092	354.20834	01Jan2000, 16:45	7712.70260
Reach-7	48.7246	358.90511	01Jan2000, 17:25	8032.88737
Scheme Inflow	24.2486	224.28327	01Jan2000, 14:40	4100.60950
Scheme Outflow	43.4349	345.98777	01Jan2000, 15:30	7402.40131

Project: Warkworth South Simulation Run: Developed-Q100-CC 2

Start of Run: 01Jan2000, 00:00 Basin Model: Developed Land-Use  
End of Run: 02Jan2000, 00:00 Meteorologic Model: 100yr-CC-developed-land-use  
Compute Time:04Apr2023, 11:34:07 Control Specifications:24hr (Maven)

Show Elements: All Elements ▾

Volume Units:  MM  1000 M3

Sorting: Alphabetic ▾

Hydrologic Element	Drainage Area (KM2)	Peak Discharge (M3/S)	Time of Peak	Volume (1000 M3)
A Downstream	5.8454	67.89074	01Jan2000, 13:50	1220.76543
A Upstream	3.3380	44.93661	01Jan2000, 13:20	704.70863
B Downstream	8.4876	111.35746	01Jan2000, 13:25	1786.68282
B Upstream	4.1220	56.84513	01Jan2000, 13:20	873.60158
C Downstream	0.2357	6.01131	01Jan2000, 12:25	55.75929
C Upstream	2.9156	43.11220	01Jan2000, 13:10	618.28270
D Downstream	2.8386	44.41328	01Jan2000, 13:00	606.60934
D Upstream	5.1008	68.48363	01Jan2000, 13:20	1073.67741
E Downstream	0.7215	19.12037	01Jan2000, 12:25	186.00455
E Upstream	5.7329	71.63279	01Jan2000, 13:35	1200.75451
F Downstream	0.2179	5.57805	01Jan2000, 12:20	47.09980
F Upstream	3.0393	47.85160	01Jan2000, 13:00	653.82898
G Downstream	1.2760	29.80892	01Jan2000, 12:30	319.54365
H Downstream	0.4917	13.48195	01Jan2000, 12:20	121.97268
Junction - ABE	21.2093	253.81225	01Jan2000, 13:55	4433.46367
Junction-AB	14.3330	160.93311	01Jan2000, 14:45	2947.84554
Junction-CD	8.0164	110.71134	01Jan2000, 13:15	1691.96012
Junction-CD ds	46.5092	430.57852	01Jan2000, 16:05	9554.84443
Junction-EFG	48.7246	436.03322	01Jan2000, 16:45	9961.73207
Outlet	49.2163	437.24925	01Jan2000, 17:25	9914.85577
Rain	4.8533	80.98906	01Jan2000, 13:10	1201.88354
Reach-1	21.2093	253.46225	01Jan2000, 14:40	4357.48934
Reach-2	8.0164	110.56449	01Jan2000, 14:20	1654.39895
Reach-3	8.4876	111.29836	01Jan2000, 15:00	1727.08011
Reach-4	24.2486	273.05174	01Jan2000, 15:55	4873.82581
Reach-5	43.4349	420.48551	01Jan2000, 16:05	8892.47580
Reach-6	46.5092	430.50396	01Jan2000, 16:45	9409.08407
Reach-7	48.7246	436.03322	01Jan2000, 17:25	9792.88309
Scheme Inflow	24.2486	273.05174	01Jan2000, 14:40	5011.31833
Scheme Outflow	43.4349	420.57851	01Jan2000, 15:30	9023.55489

### 2.5.3 Effects of the proposed development

A graph of the flows at the scheme outflow of the existing catchment flow with climate change against flow of the developed catchment with climate change can be found in the appendix A, the table below summaries the findings.

Rain event	Land-use	Climate change	Catchments A-F (m3/s)	Rain (m3/s)	Downstream Catchment A & B (m3/s) outflow	Scheme Outflow (m3/s)
100yr	Existing	No	224.3	54.4	132.2	346.5
100yr	Existing	Yes	273.1	66.3	160.9	421.7
100yr	Developed	No	224.6	68.2	132.2	344.2
100yr	Developed	Yes	273.1	81.0	160.9	420.6

Table 2.3 – Peak flow comparison, 100-year, climate change, existing vs developed

Table 2.3 shows that the peak flow for 100year storm events exiting the scheme area (Scheme outflow) decreases by 0.9m<sup>3</sup>/s , even though there is an increase in impervious area of the development. This is explained by the decrease in time of concentration of the developed Rain catchment, which results in the runoff from the catchment reaching the Scheme outflow before the runoff from the upstream catchments (A-F). As shown in figure 2.8, for the developed catchment, the 100year time of peak flow of the *Rain* catchment is 13:10 and for the upstream catchments, *A to F* (Reach 4) is 15:55. This demonstrates the peak flow from the *Rain* catchment exits the catchment boundary 2 hour 45 min prior to the arrival of upper catchment peak flow.

#### Downstream effects

Table 2.4 below shows the peak 100year stormwater events at the catchment junctions downstream of the site. Similarly to the effects described above the it is noted that the peak flows decrease as a result of the development. This is explained by the decrease in time of concentration of the developed Rain catchment, which results in the runoff from the catchment reaching the Scheme outflow before the runoff from the upstream catchments (A-F).

Rain event	Land-use	Climate change	Downstream Junction CD (m3/s)	Downstream Junction EFG (m3/s)
100yr	Existing	No	354.7	359.4
100yr	Existing	Yes	431.7	437.1
100yr	Developed	No	352.3	358.9
100yr	Developed	Yes	430.6	436.0

Table 2.4 Peak 100yr flows at junctions downstream of the proposed development

### 2.5.3 Localised event scenario

A localised event scenario has been modelled which includes an Upstream PCA 2yr event and a 100yr event within the PCA (rain catchment) and downstream catchments of the PCA 100yr ARI for before and post development, no climate change. These runs are considered necessary to understand the effects of the development on the existing scenarios.

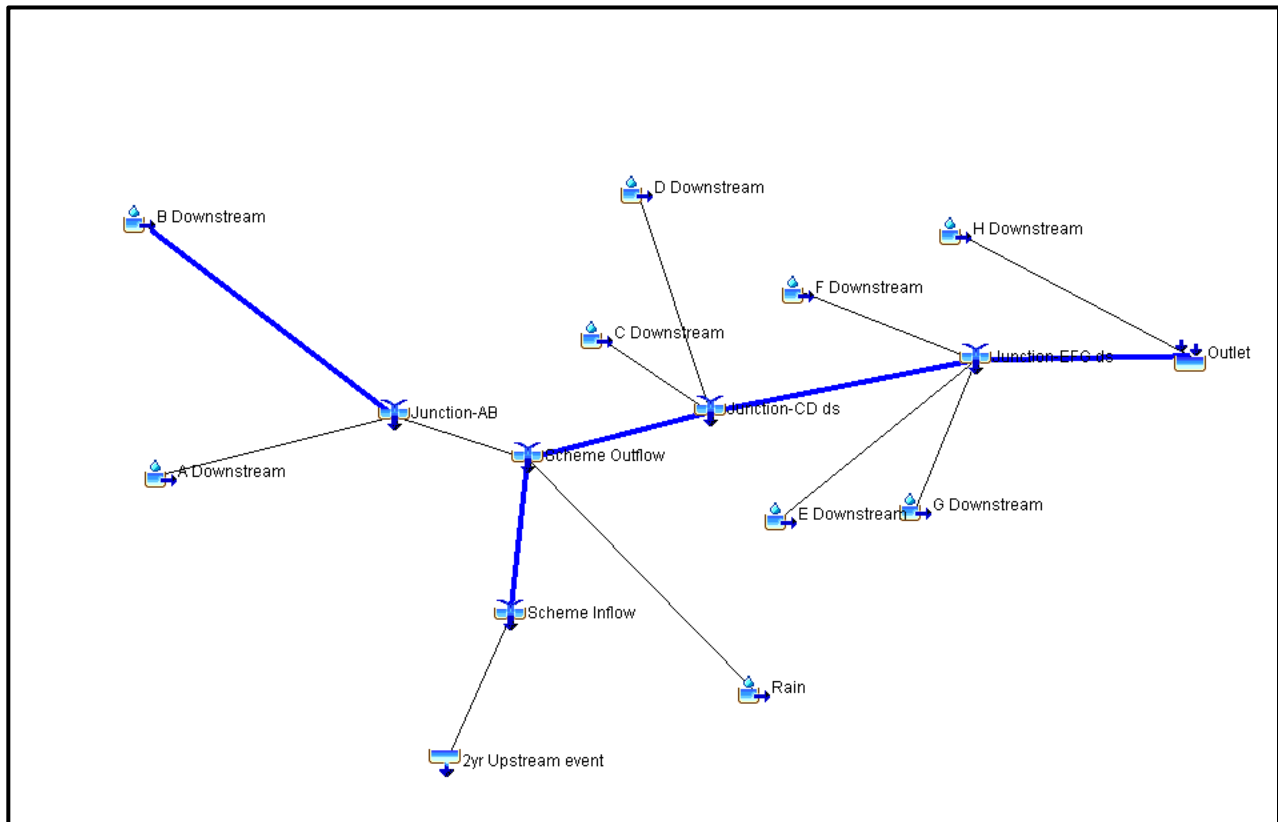


Figure 2.-9 Localised Event Scenario

Rain event	Land-use	Climate change	Scheme Outflow (m3 /s)	Downstream Junction CD (m3/s)	Downstream Junction EFG (m3/s)
100yr	Existing	No	212.6	222.8	227.6
100yr	Developed	No	212.4	222.6	227.4

Table 2.5 Peak 100yr flows at junctions downstream of the proposed development for a localised 100yr event scenario without climate change

Table 2.5 above shows the 100yr peak stormwater event for a localised 100yr event for the PCA area and downstream PCA area. Similarly to the effects described above the it is noted that the peak flows slightly decrease as a result of the development. This is explained by the decrease in time of concentration of the developed Rain catchment, which results in the runoff from the catchment reaching the Scheme outflow before the runoff from the upstream catchments (A-F).



### 3 HYDRAULIC MODELLING WITH HEC-RAS

#### 3.1 METHODOLOGY

The analysis was done using the following steps:

1. Delineate the perimeter for the grid,
2. Create a grid and sub-grid areas,
3. Input flow hydrographs and other boundaries
4. Input structures,
5. Run scenarios.

#### 3.2 HEC-RAS MODEL LAYOUT

HEC-RAS software was used to generate water levels throughout the catchment. A 2D model was developed using a combination of LINZ Terrain data and site-specific LiDAR and topographical survey. A Manning's n of 0.1 was used in the grid. A 5m x 5m grid was used. Figure 3.1 shows the grid and its boundary conditions. Appendix B shows culvert details used in the model.

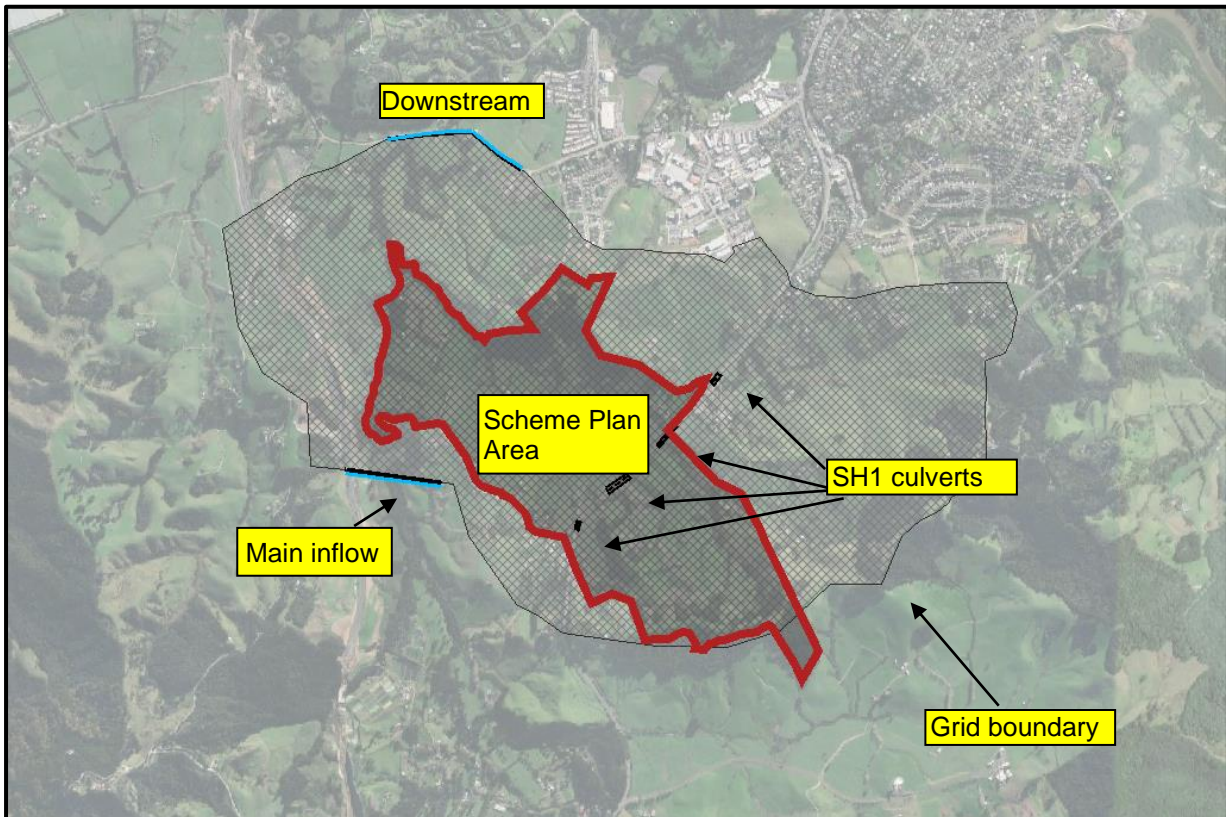


Figure 3.1 – HEC-RAS model set-up



### 3.3 BOUNDARIES

There are three boundaries. These are:

- Rain on grid – as per figure 3.1.
- Main inflow for mahurangi river
- Downstream boundary using a normal depth method with a gradient of 0.002.

### 3.4 FLOODPLAIN COMPARISON

Figure 3.2 compares the Geomaps floodplain against the 100-year storm for developed land and climate change rainfall. The patterns are similar. The flow at critical pinch points in the north-east at the confluence have similar widths.

The only difference of note is in the central scheme area, area A. Geomaps shows more flooding while the new model is more defined in the channels due to a specific site survey of the stream being modelled. In general, the new model appears to replicate the Geomaps floodplain.

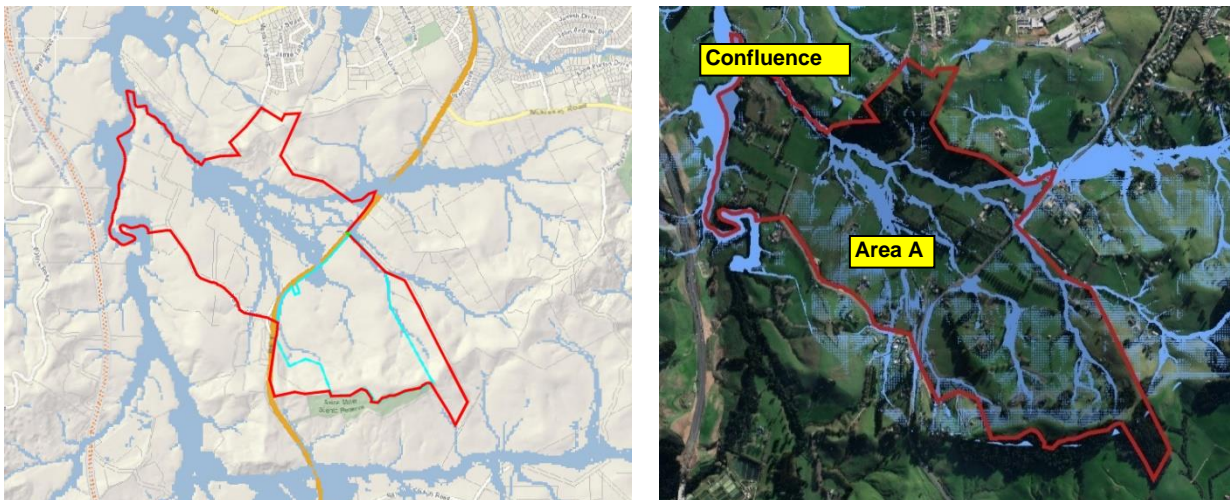


Figure 3.2 – Floodplain comparison – 100yr-storm

### 3.5 FLOW CHECK

All watercourse arrive at the point of confluence from the east making up about 332ha. The estimate TP108 graphical method 100-year peak flow is estimated at 47m<sup>3</sup>/s. This is the existing rainfall and land-use. The modelled peak flow at this point is 53m<sup>3</sup>/s. The model is higher than what TP108 estimates catchment run-off should be. However, the 2D terrain model uses a Manning's n of 0.1 which might be smoother than reality, but it also encourages higher flows. Importantly the model gives reasonable peak flows even though the finite volume method in HEC-RAS has pockets of water "stuck in hollows" inside the 5m grid. However, this does not affect the peak flow.

### 3.6 HYDRAULIC GRADE LINE

Figure 3.3 shows the HGL along the Mahurangi River on the west boundary of the scheme plan. The 100-year developed scenario ranges from RL32.4m to RL26.6m NZVD at a grade of 1 in 240. The range of water levels from 10-year to 100-year is about 1.3m.

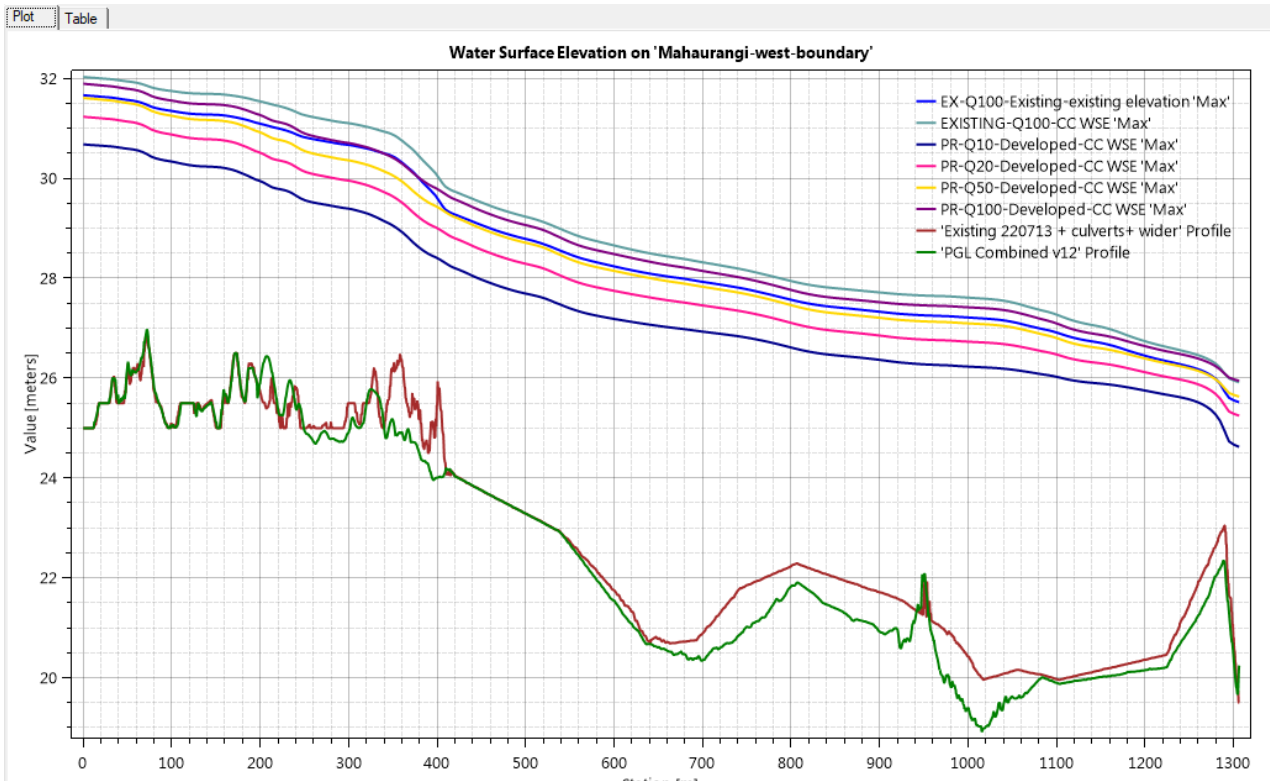
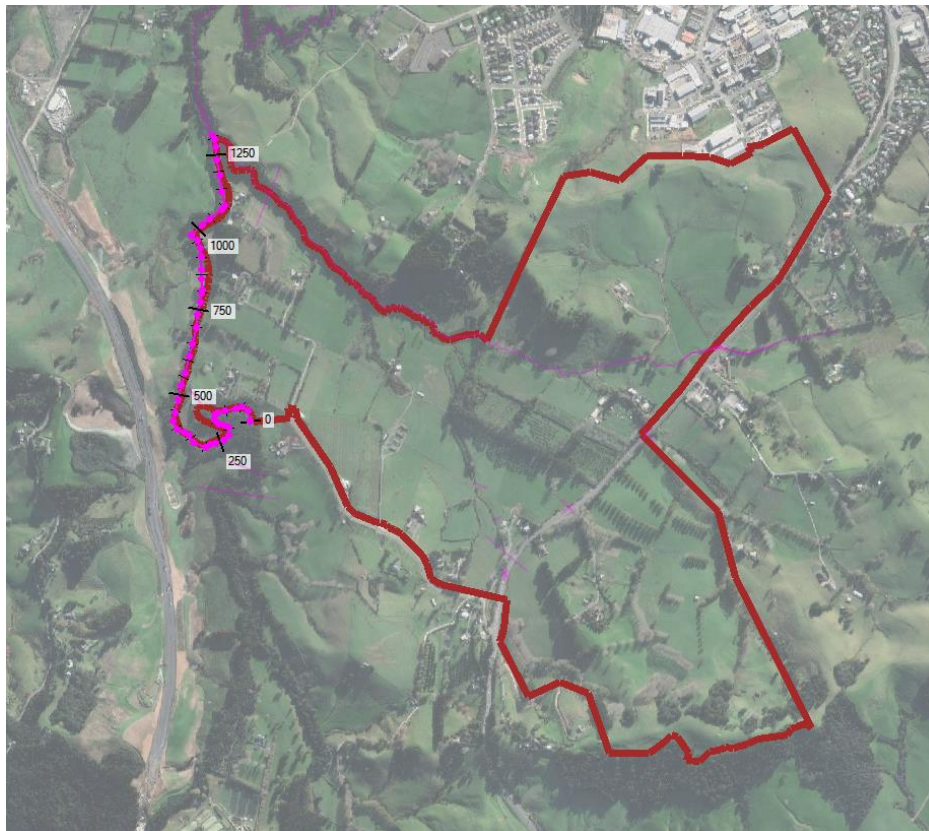


Figure 3.3 – HGL– Mahurangi River boundary (NZVD)



Figure 3.4 shows the HGL along the main scheme stream from east of the SH1 culverts across to the confluence with the Mahurangi River. The SH1 Culverts will cause a pond that is 200m long and 135m wide.

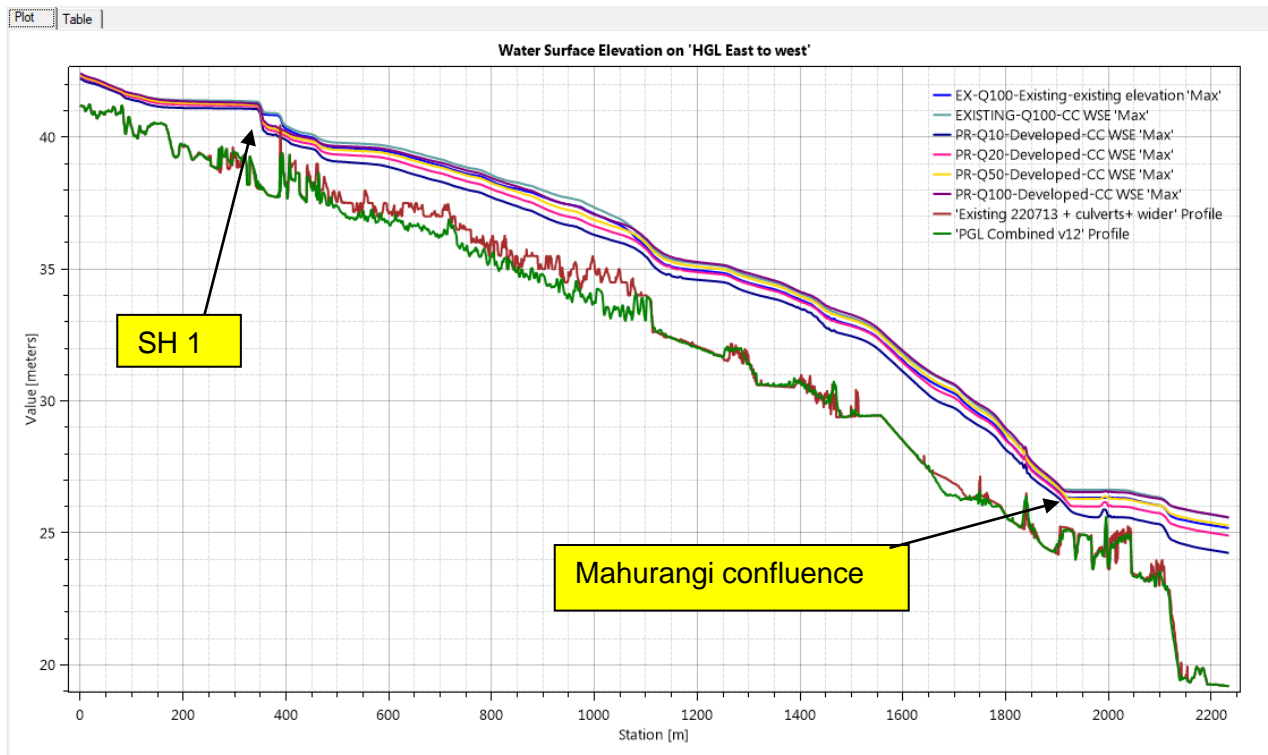
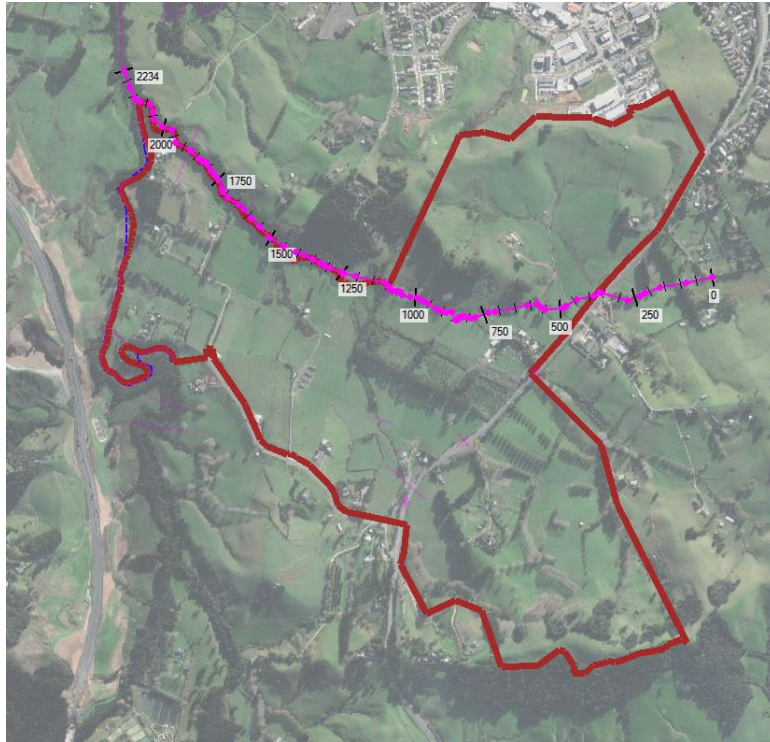


Figure 3.4 – HGL– Main east to west stream (NZVD)

### 3.7 FLOW HYDROGRAPHS

Figure 3.5 shows the flow hydrographs in the east-west stream just before the connection to the Mahurangi River. The existing 100-year storm without climate change flows (blue) is approximately equivalent to a 20-year storm with climate change flow (yellow) due to the development. And the existing 100-year storm with climate change is approximately equivalent to a 50-year storm post development.

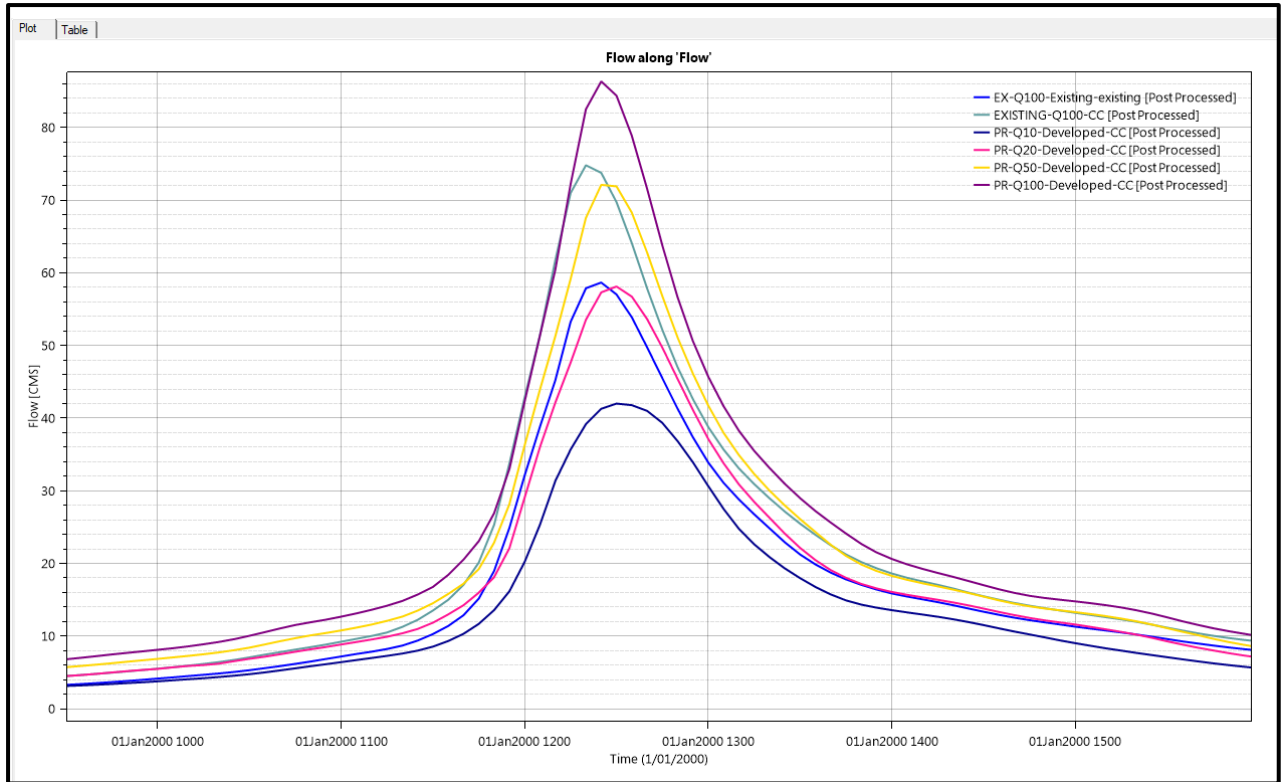


Figure 3.5 – Flow hydrograph for east-west stream outlet

### 3.8 CHECK ON DOWNSTREAM LEVEL

The model grid stops at Woodcocks Road bridge. The road deck of the bridge has been surveyed to be RL23.75. The peak 1% AEP event with climate change flow level at this location has been calculated to be RL 22.17 and 22.16 for the existing and proposed scenario respectively.

The manning calculation may be found in appendix E. It is noted that the contours used for the manning calculation have been obtained from Auckland Council Geomaps. As the Lidar contours indicate the water surface elevation (rather than the streambed) of the stream baseflow it is noted that the flow depth may be considered conservative. Therefore, the bridge is not impeding flow.

### 3.9 CULVERTS CAPACITY ASSESSMENT

Cross sections showing water surface elevations across the four culverts through SH 1 maybe found in appendix B.

Cross sections indicate the two northern culverts (names Culvert north and Culvert mid) are under capacity and are overtopped for all modelled storm events (10yr through to 100yr). The next adjacent culvert to the south (culvert south) is show to only overtop during a 100yr event for the developed scenario. The southern most culvert is shown to have sufficient capacity for the developed scenario.

It is noted that the entire section of SH1 which is shown to be have under capacity culvert is proposed to be upgraded in the future by Auckland Transport once the road is eventually repurposed as an arterial road and the opening of Ara Tūhono – Pūhoi to Warkworth statehighway.

### 3.10 OUTFLOW VOLUME VALIDATION

HEC-RAS uses an Implicit Finite Volume Algorithm. The consequence of this is to have small volumes of water in the base of a cell that does not escape. A method to remove the potential holding back of water is to run the models with low flows in the initial stages to fill the hollows. The main storm run-off can then flow over the top. This is not a problem as long as there is volume continuity.

Figure 3.6 shows the volume generated in HEC-HMS for the existing land-use and 100-year historical rainfall. The volume is 4,942,000m<sup>3</sup>. Figure 3.7 shows the volume accumulated at the HEC-RAS downstream boundary after 36 hours of simulation. The volume is 4,908,000m<sup>3</sup>. This is an error of 0.0007% which is extremely small. The volume integrity is excellent.

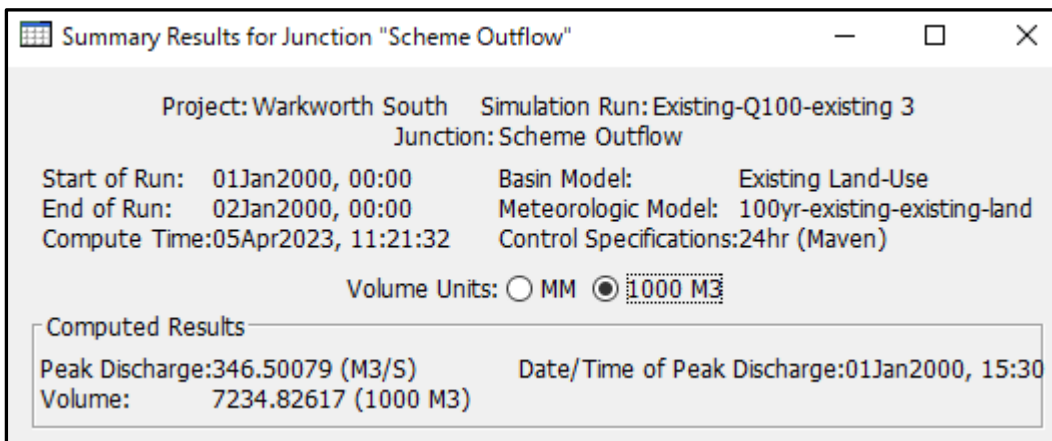


Figure 3.6 – HEC-HMS volume of run-off

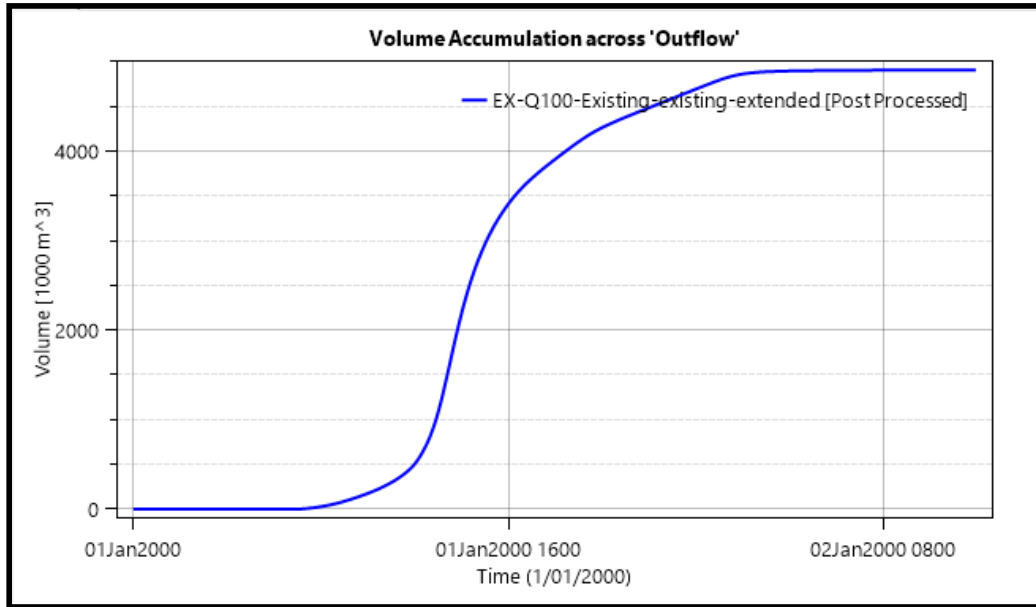


Figure 3.7 – HEC-RAS outflow boundary cumulative volume

### 3.11 HEALTHY WATERS MODELLING

Auckland Council HealthyWaters have supplied flow data of their modelling of the Warkworth catchment, for Mahurangi River. A comparison of this reports results and Healthywaters are summarised in the table below;

Scenario	XS 95 - SCHEME INFLOW (m3/s)					
	10yr Developed CC			100yr Developed CC		
	Peak Time	Peak Flow	Water level	Peak Time	Peak Flow	Water level
Healthy waters	13:40	199	31.34	13:40	340	32.88
Maven	14:45	142	31.78	14:40	273	33.05

Scenario	XS87 - SCHEME OUTFLOW (m3/s)					
	10yr Developed CC			100yr Developed CC		
	Peak Time	Peak Flow	Water level	Peak Time	Peak Flow	Water Level
Healthy waters	14:10	223	22.16	14:15	326	23.74
Maven	15:10	151	22.12	15:15	285	23.89

Figure 3.8 MPD Modelling results comparison to Healthy waters model

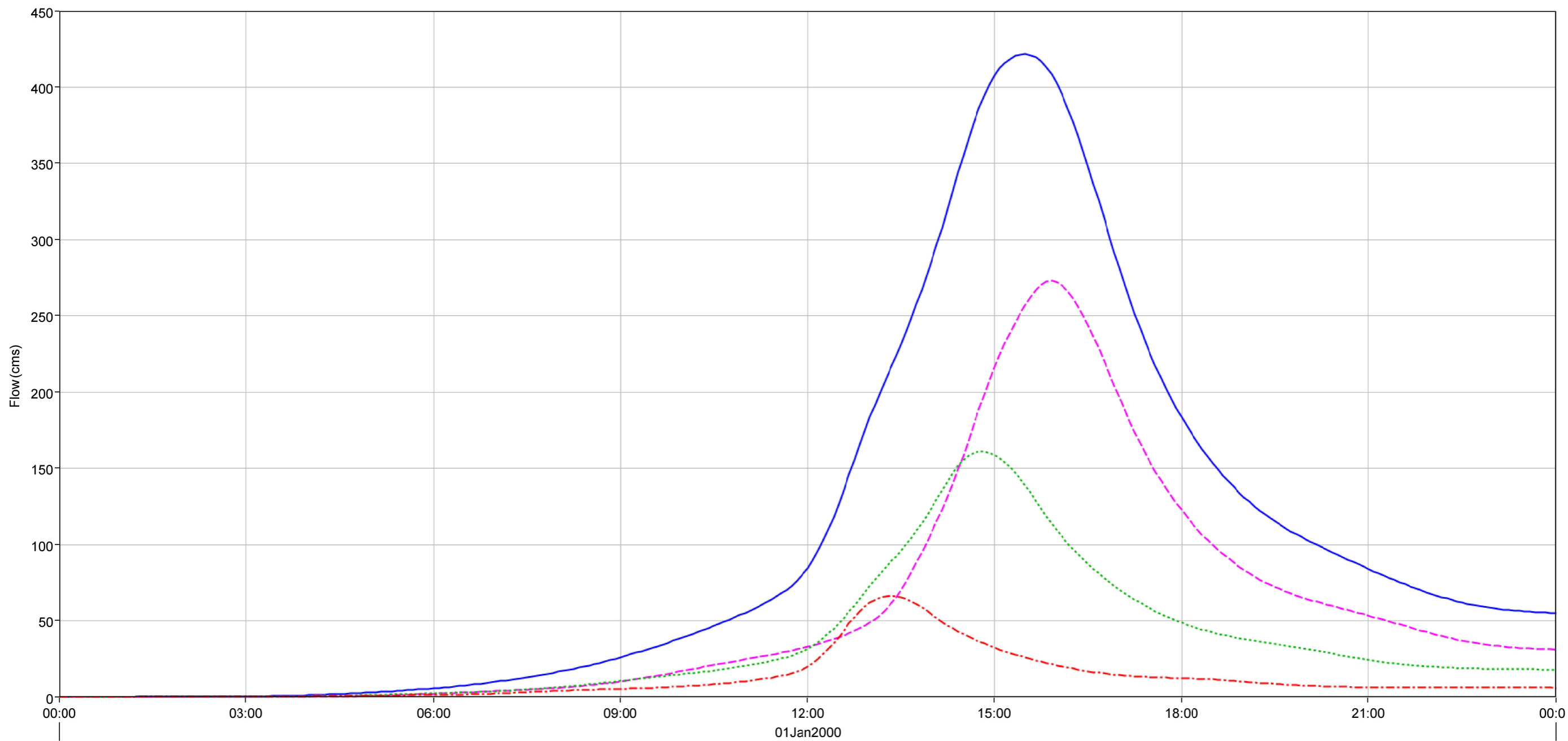
Two notable points of comparison of modelling results are the times of peak flows and the water levels. The peak flow times produced in the model are generally 1 hour later than that from the Healthywaters model. A comparison of the water levels show similar peak flood levels with a difference of upto to 0.32m. This discrepancy is likely a result of the difference in terrain model used. As the terrain used in this reports model uses a combination of site survey and drone data, it has a higher degree of accuracy in comparison to the Lidar survey used in the Healthwaters model.

---

**APPENDIX A – 100YR YEAR FLOW HYDROGRAPH**



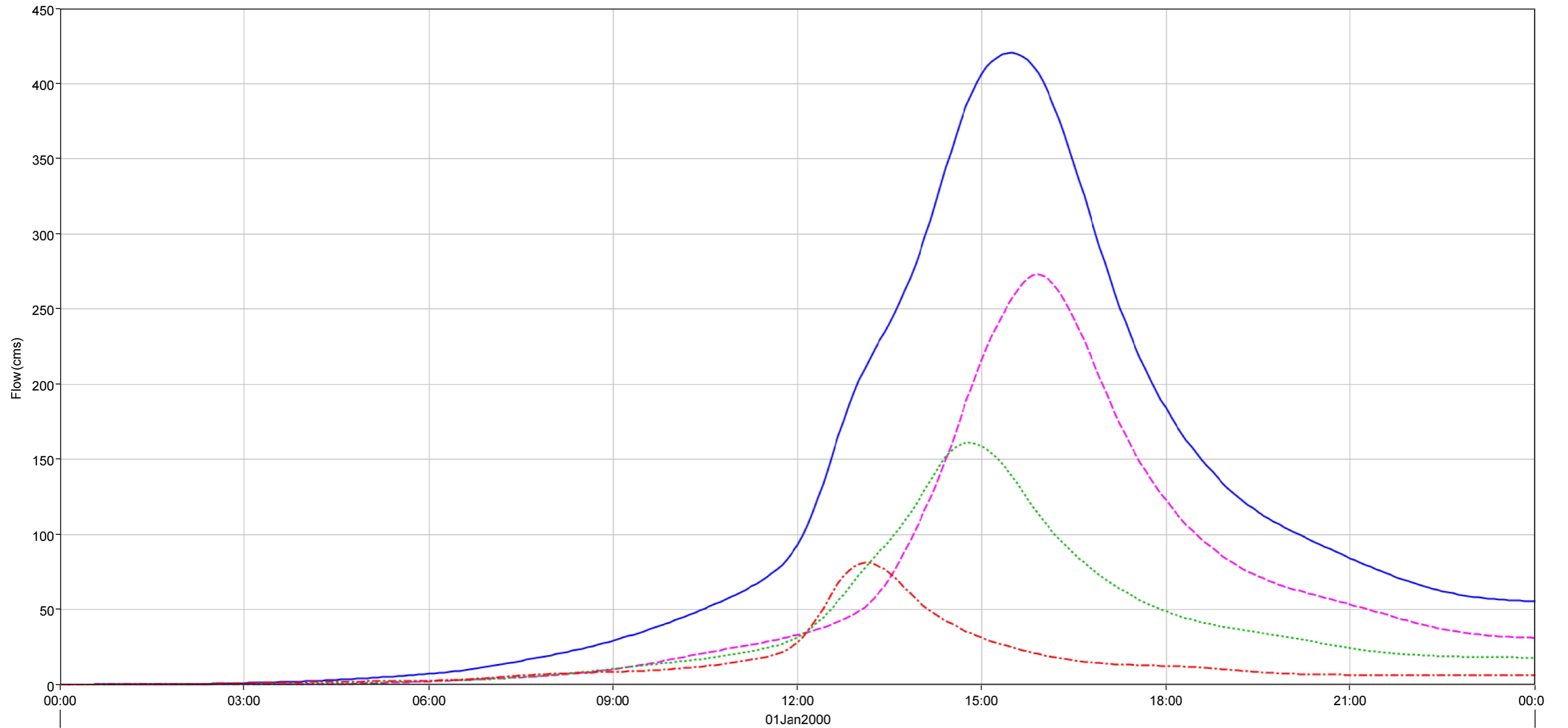
Junction "Scheme Outflow" Results for Run "Existing-Q100-existing-CC 3"



Legend (Compute Time: 05Apr2023, 11:38:12)

- Run:Existing-Q100-existing-CC 3 Element:Scheme Outflow Result:Outflow
- Run:Existing-Q100-existing-CC 3 Element:Reach-4 Result:Outflow
- Run:Existing-Q100-existing-CC 3 Element:Junction-AB Result:Outflow
- Run:Existing-Q100-existing-CC 3 Element:Rain Result:Outflow

Junction "Scheme Outflow" Results for Run "Developed-Q100-CC 2"



Legend (Compute Time: 05Apr2023, 11:45:30)

Run:Developed-Q100-CC 2 Element:Scheme Outflow Result:Outflow

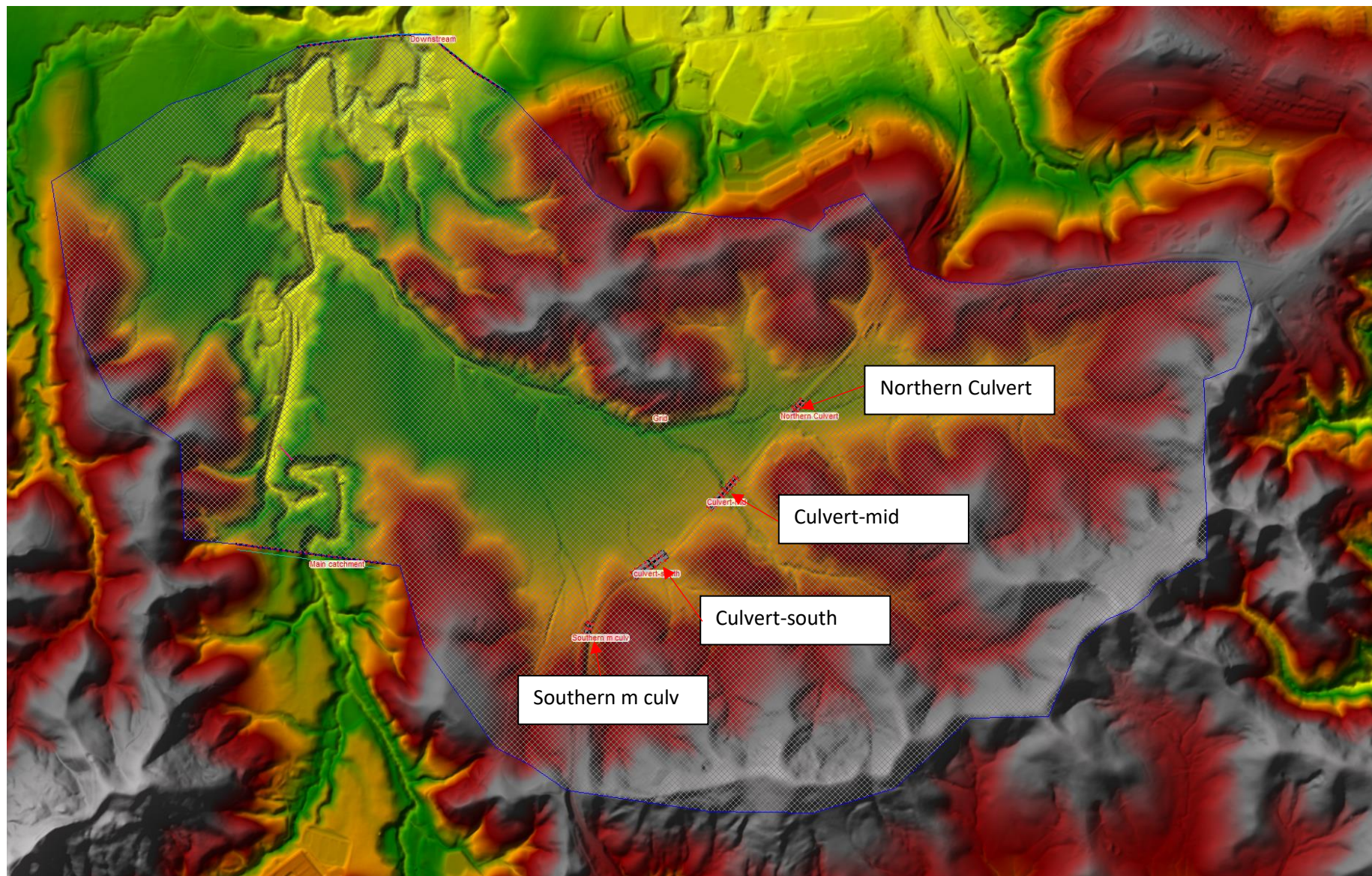
Run:Developed-Q100-CC 2 Element:Reach-4 Result:Outflow

Run:Developed-Q100-CC 2 Element:Junction-AB Result:Outflow

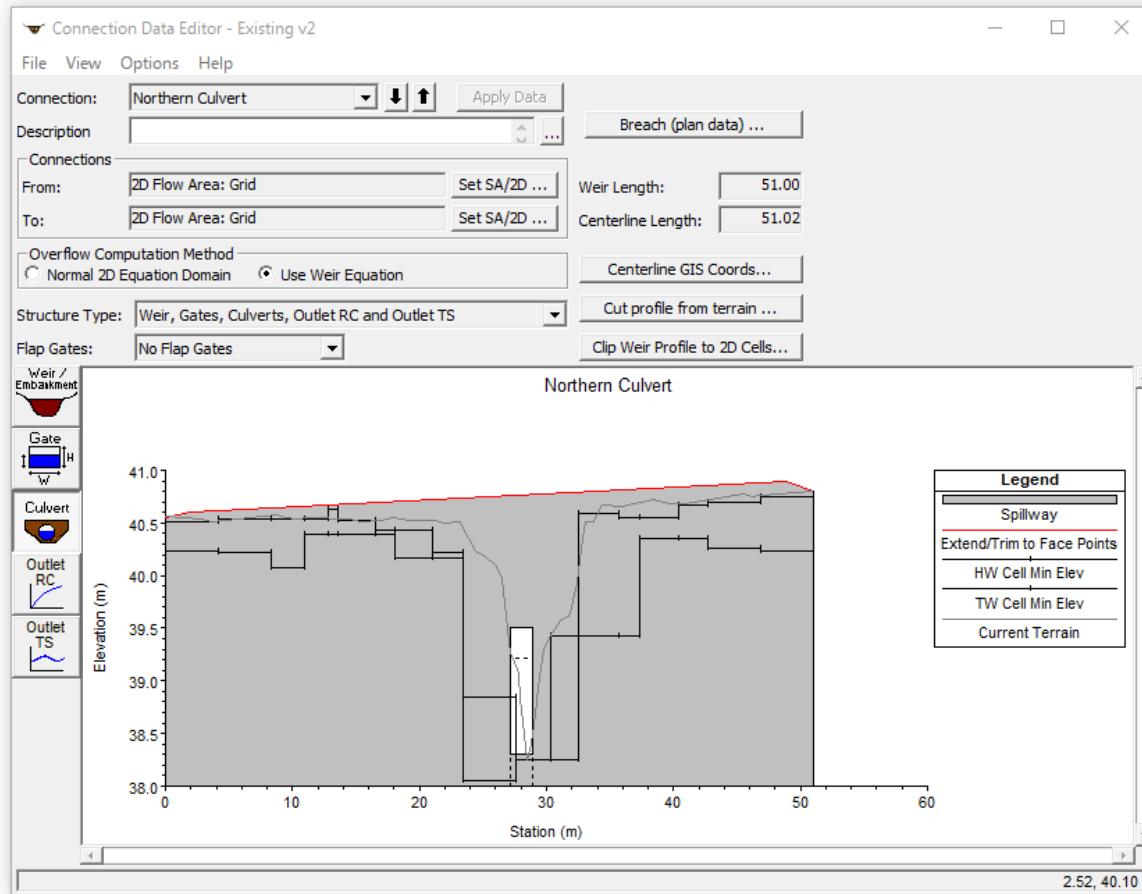
Run:Developed-Q100-CC 2 Element:Rain Result:Outflow

**APPENDIX B – HEC RAS CULVERT DETAIL**





## Northern Culvert - Details



Culvert Data Editor

Culvert Group: Culvert #1 [Down Arrow] [Up Arrow] [Print] [Close] [Help]

Solution Criteria: Computed Flow Control

Shape: Box Span: 1.8 Rise: 1.21

Chart #: 8 - flared wingwalls

Scale #: 1 - Wingwall flared 30 to 75 deg.

Culvert Length: 26 Depth to use Bottom n: 0

Entrance Loss Coeff: 1 [?] Depth Blocked: 0

Exit Loss Coeff: 1 [?] Upstream Invert Elev: 38.3

Manning's n for Top: 0.013 [?] Downstream Invert Elev: 38

Manning's n for Bottom: 0.013

Culvert Barrel Data

Barrel Centerline Stations # Barrels: 1

Barrel GIS Data: Barrel #1 Length: 26.2

Barrel Name	US Sta	DS Sta	GIS Sta	X	Y
1 Barrel #1	28	28	28	1748267.29	968519.723
2				1748241.56	968524.526
3					
4					
5					

Individual Barrel Centerlines ... [Show on Map] [OK] [Cancel] [Help]

Select culvert to edit



## Culvert mid - Details

### Connection Data Editor - Existing v2

File View Options Help

Connection: **Culvert-mid** Apply Data

Description: Breach (plan data) ...

Connections:

From: 2D Flow Area: Grid Set SA/2D ... Weir Length: 122.97

To: 2D Flow Area: Grid Set SA/2D ... Centerline Length: 122.97

Overflow Computation Method:

Normal 2D Equation Domain  Use Weir Equation Centerline GIS Coords...

Structure Type: **Weir, Gates, Culverts, Outlet RC and Outlet TS** Cut profile from terrain ...

Flap Gates: **No Flap Gates** Clip Weir Profile to 2D Cells...

Weir / Embankment

Gate

Culvert

Outlet RC

Outlet TS

14.56, 40.35

### Culvert Data Editor

Culvert Group: **Culvert #1**

Solution Criteria: **Computed Flow Control**

Shape: **Circular** Span: 1.8 Diameter: 1.8

Chart #: **1 - Concrete Pipe Culvert**

Scale #: **1 - Square edge entrance with headwall**

Culvert Length: 20 Depth to use Bottom n: 0

Entrance Loss Coeff: 1 Depth Blocked: 0

Exit Loss Coeff: 1 Upstream Invert Elev: 39.8

Manning's n for Top: 0.013 Downstream Invert Elev: 38.9

Manning's n for Bottom: 0.013

Culvert Barrel Data

Barrel Centerline Stations # Barrels: 1

Barrel Name	US Sta	DS Sta	GIS Sta
1 Barrel#1	71.5	71.5	71.5
2			
3			
4			
5			

Barrel GIS Data: Barrel#1 Length: 20.4

	X	Y
1	.748048.167	968268.798
2	.748034.812	968284.229
3		
4		
5		

Individual Barrel Centerlines ... Show on Map OK Cancel Help

Select culvert to edit

## Culvert south - Details

**Connection Data Editor - Existing v2**

File View Options Help

Connection: **culvert-south** [Apply Data]

Description: [Breach (plan data) ...]

Connections

From: 2D Flow Area: Grid [Set SA/2D ...] Weir Length: 116.48

To: 2D Flow Area: Grid [Set SA/2D ...] Centerline Length: 116.48

Overflow Computation Method  
 Normal 2D Equation Domain  Use Weir Equation [Centerline GIS Coords...]

Structure Type: Weir, Gates, Culverts, Outlet RC and Outlet TS [Cut profile from terrain ...]

Flap Gates: No Flap Gates [Clip Weir Profile to 2D Cells...]

**culvert-south**

17.30, 43.84

**Culvert Data Editor**

Culvert Group: Culvert #1

Solution Criteria: Computed Flow Control

Shape: Circular Span: 0.825 Diameter: 0.825

Chart #: 1 - Concrete Pipe Culvert

Scale #: 1 - Square edge entrance with headwall

Culvert Length: 29 Depth to use Bottom n: 0

Entrance Loss Coeff: 1 Depth Blocked: 0

Exit Loss Coeff: 1 Upstream Invert Elev: 41.9

Manning's n for Top: 0.013 Downstream Invert Elev: 41.6

Manning's n for Bottom: 0.013

Culvert Barrel Data

Barrel Centerline Stations				# Barrels:	Barrel GIS Data: Barrel #1		
Barrel Name	US Sta	DS Sta	GIS Sta	Length: 29.6	X	Y	
1 Barrel#1	63.3	63.3	64.7	2	1 .747833.208	1968047.569	
2 Barrel#2	64.3	64.3	62.6		2 .747813.551	1968069.739	
3					3		
4					4		
5					5		

Individual Barrel Centerlines ... Show on Map OK Cancel Help

Select culvert to edit

## Culvert m south - Details

### Connection Data Editor - Existing v2

File View Options Help

Connection: Southern m culv Apply Data

Description: Breach (plan data) ...

Connections

From: 2D Flow Area: Grid Set SA/2D ... Weir Length: 37.10

To: 2D Flow Area: Grid Set SA/2D ... Centerline Length: 37.13

Overflow Computation Method

Normal 2D Equation Domain  Use Weir Equation Centerline GIS Coords...

Structure Type: Weir, Gates, Culverts, Outlet RC and Outlet TS Cut profile from terrain ...

Flap Gates: No Flap Gates Clip Weir Profile to 2D Cells...

Southern m culv

**Legend**

- Spillway
- Extend/Trim to Face Points
- HW Cell Min Elev
- TW Cell Min Elev
- Current Terrain

13.94, 48.27

### Culvert Data Editor

Culvert Group: Culvert #1

Solution Criteria: Computed Flow Control

Shape: Circular Span: 0.8 Diameter: 0.8

Chart #: 1 - Concrete Pipe Culvert

Scale #: 1 - Square edge entrance with headwall

Culvert Length: 18 Depth to use Bottom n: 0

Entrance Loss Coeff: 1 Depth Blocked: 0

Exit Loss Coeff: 1 Upstream Invert Elev: 46.8

Manning's n for Top: 0.013 Downstream Invert Elev: 46.58

Manning's n for Bottom: 0.013

Culvert Barrel Data

Barrel Centerline Stations # Barrels: 1

	Barrel Name	US Sta	DS Sta	GIS Sta
1	Barrel #1	18.1	18.1	18.1
2				
3				
4				
5				

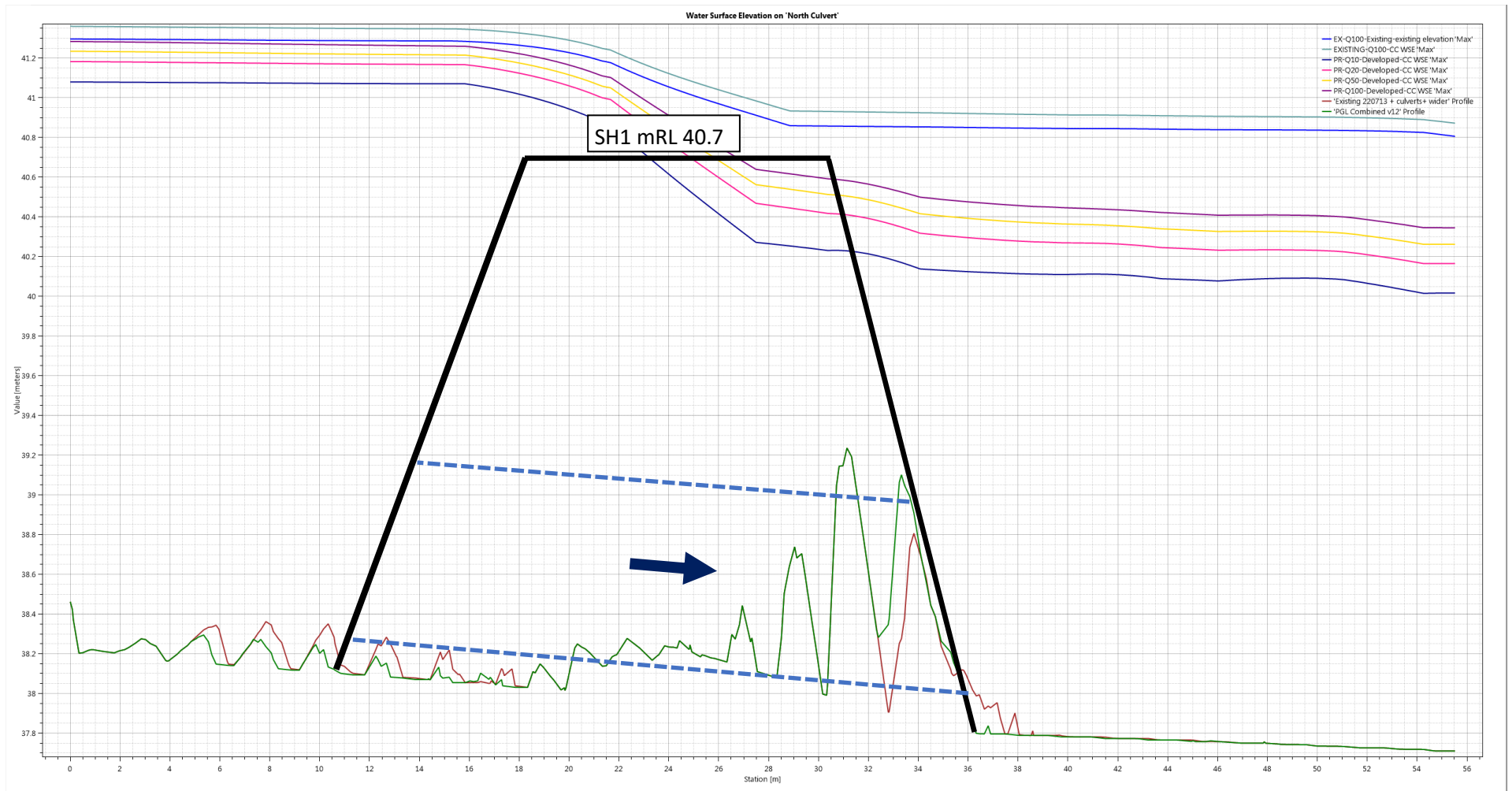
Barrel GIS Data: Barrel #1		
Length: 39.5		
	X	Y
1	.747645.555	1967853.886
2	.747625.105	5967887.71
3		
4		
5		

Individual Barrel Centerlines ... Show on Map OK Cancel Help

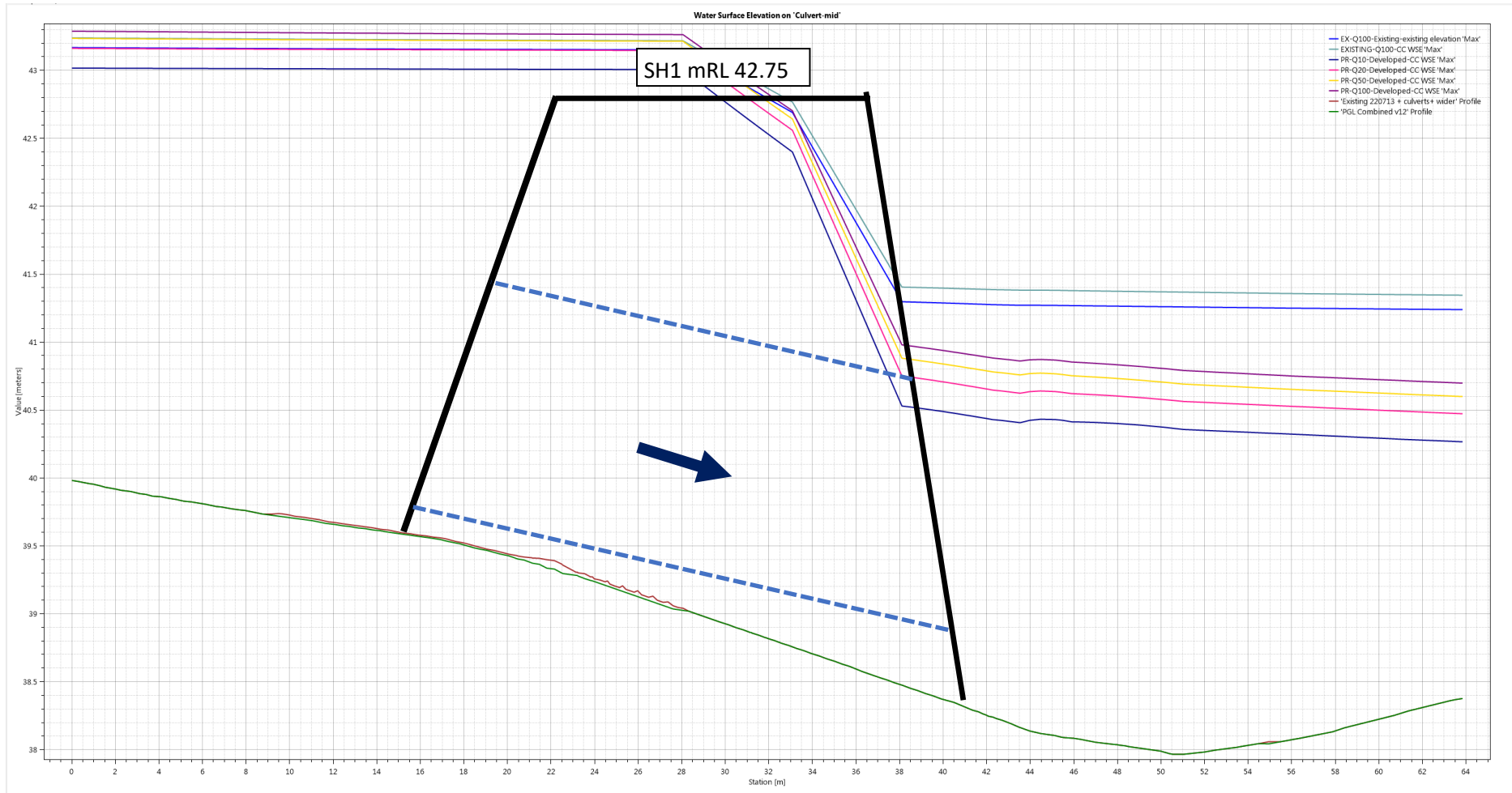
Select culvert to edit



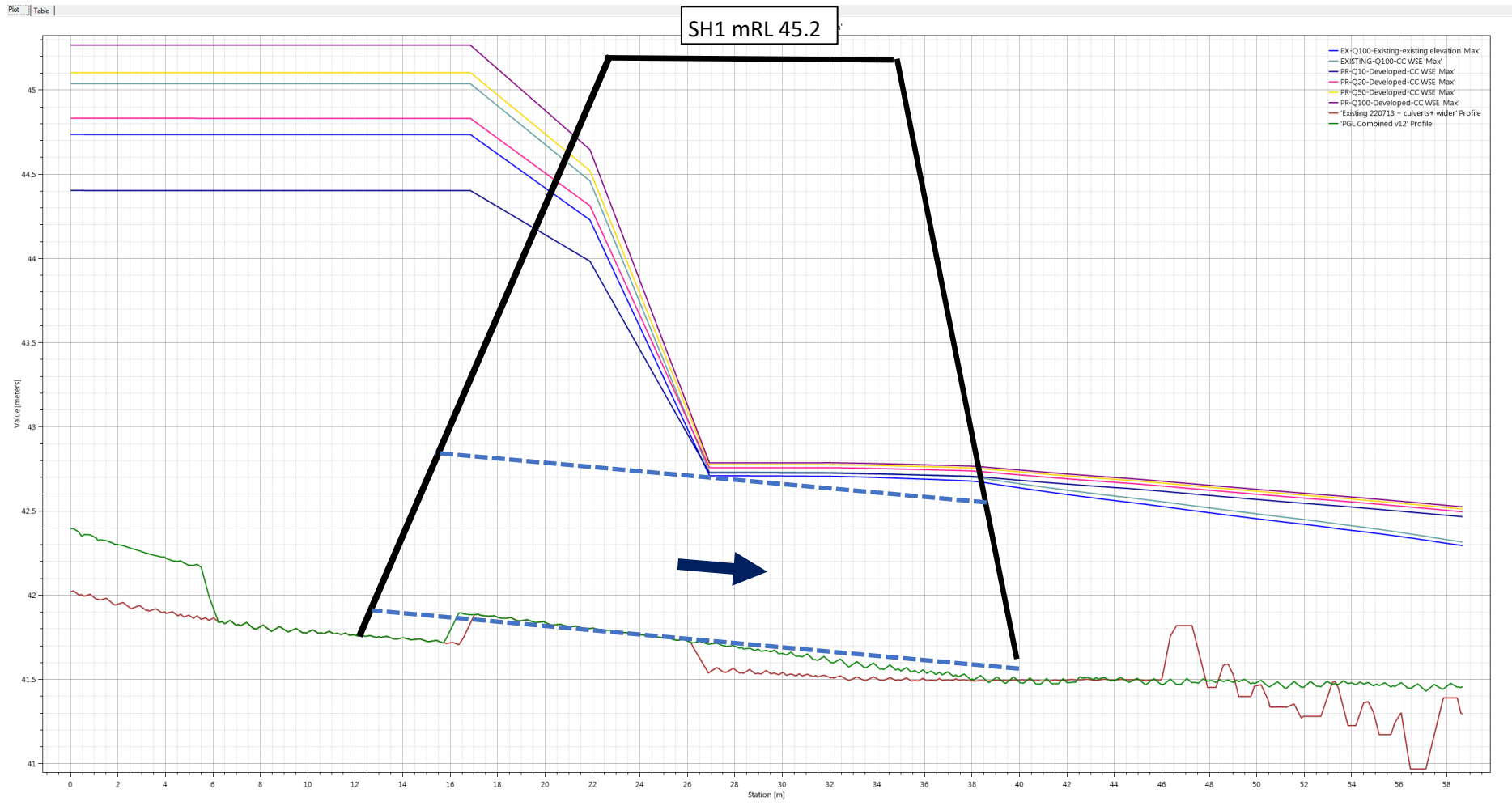
# Northern Culvert (1800mm x 1200mm box)



# Mid Culvert (1800mm circular)

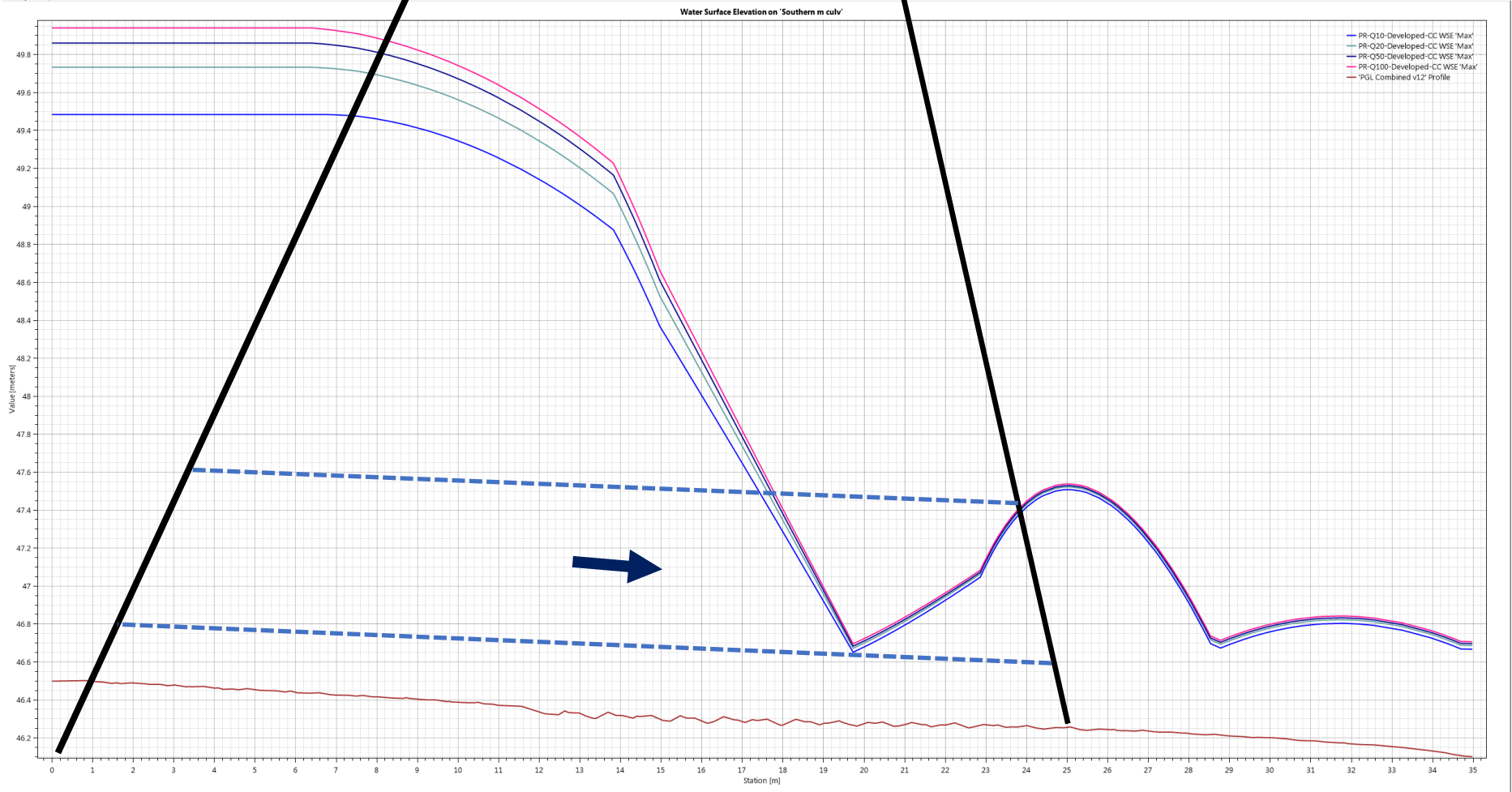


# Culvert South (825mm circular x 2)

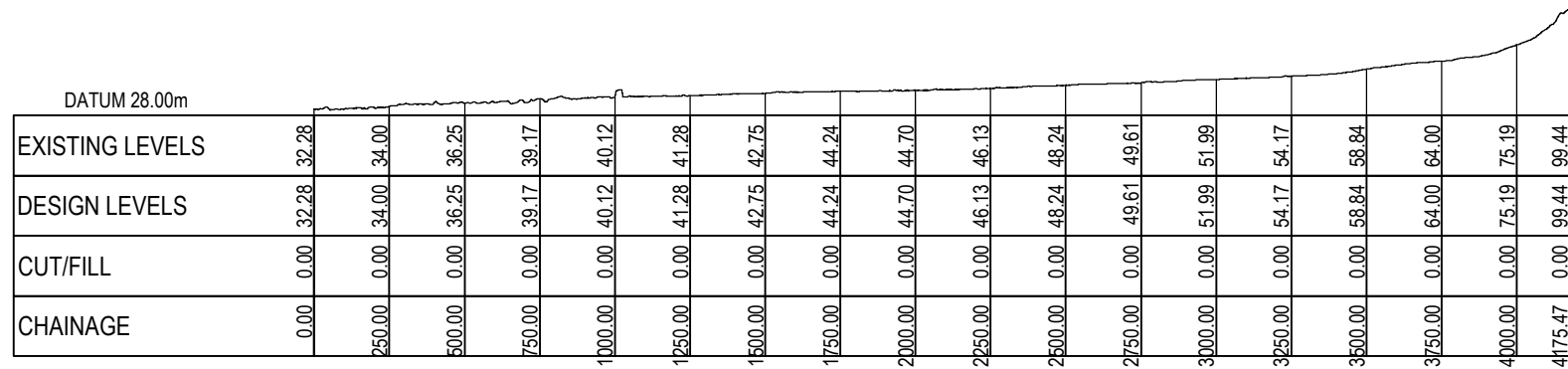


# Culvert m south (800mm circular)

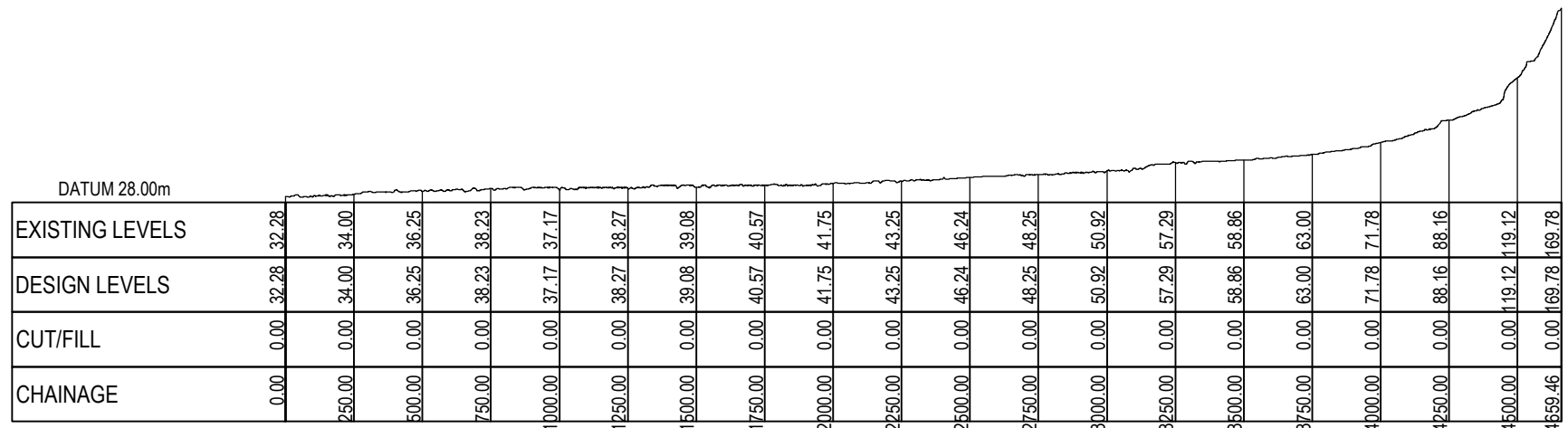
SH1 mRL 50.2



**APPENDIX C – Preliminary Pre & Post Development Flood Extent Plan**



Upstream A Longsection  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3



Upstream B Longsection  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

Notes  
1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023

**M** Maven Associates  
09 571 0050  
info@maven.co.nz  
www.maven.co.nz  
5 Owens Road, Epsom  
Auckland 1023

Project  
**WARKWORTH SOUTH  
PLAN CHANGE FOR  
KA WAIMANAWA LP &  
STEPPING TOWARDS  
FAR LTD**

Title  
**FLOOD MODELLING  
CATCHMENT  
LONGSECTION PLAN**

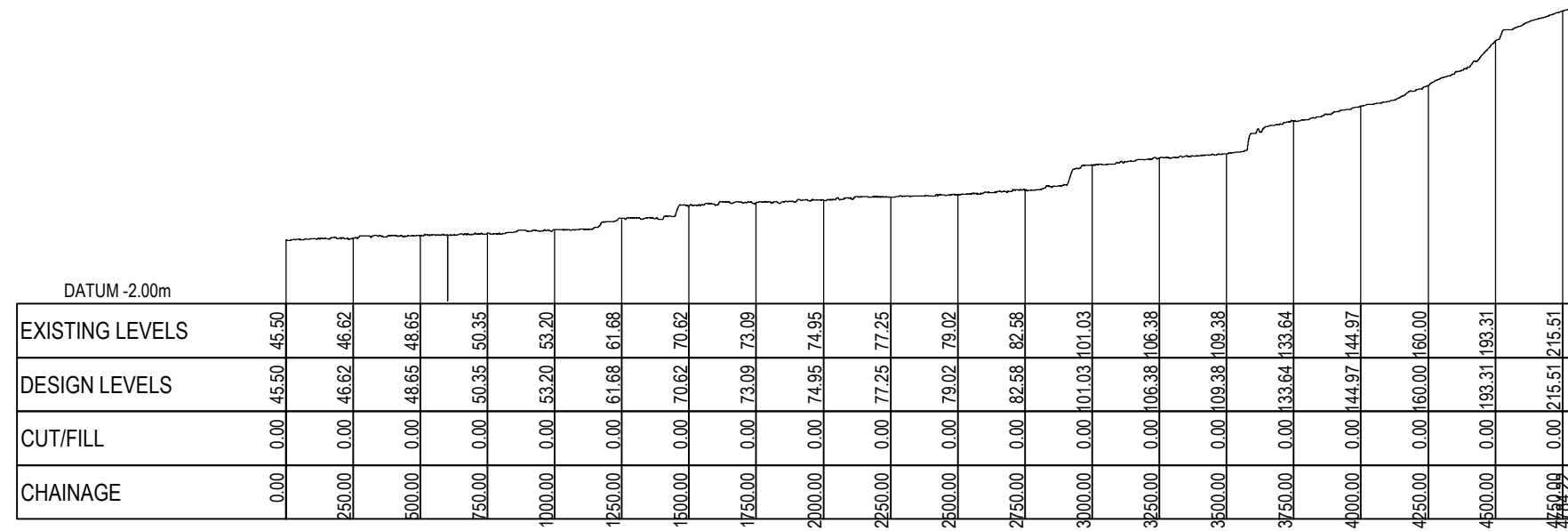
Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDEN2K.DWG
Drawing no.	SK001
Rev	<b>A</b>

**DRAFT FOR REVIEW**

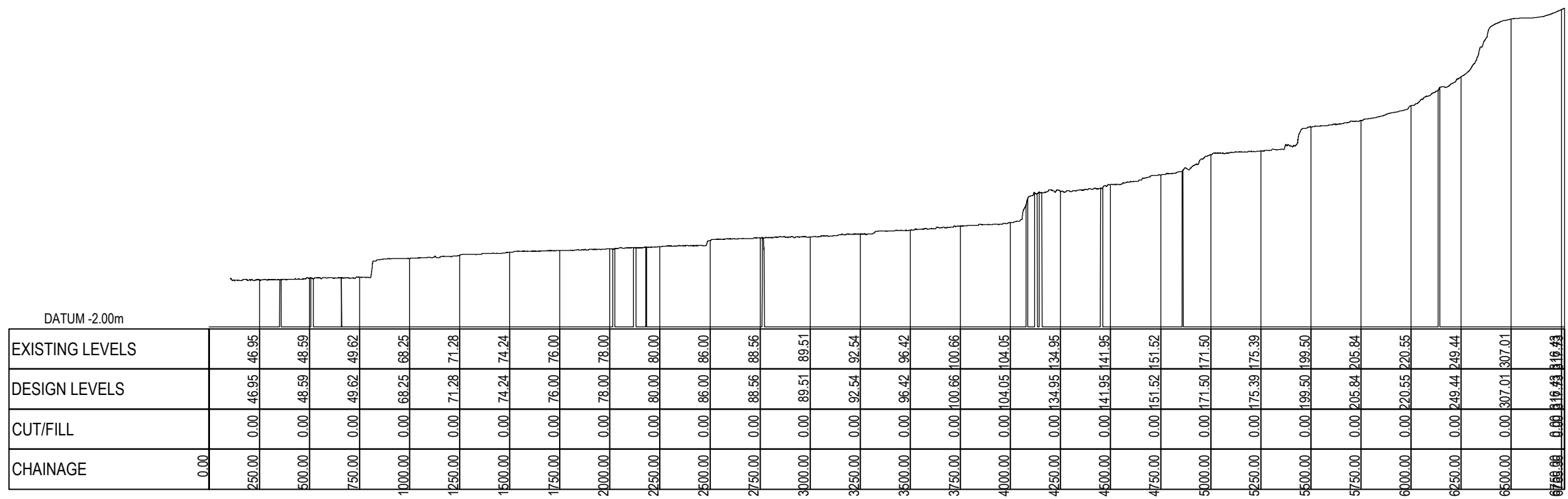
DATE: 4/8/23



Notes  
 1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.



Upstream C Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3



Upstream D Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023

**M** Maven Associates  
 09 571 0050  
 info@maven.co.nz  
 www.maven.co.nz  
 5 Owens Road, Epsom  
 Auckland 1023

Project  
**WARKWORTH SOUTH  
 PLAN CHANGE FOR  
 KA WAIMANAWA LP &  
 STEPPING TOWARDS  
 FAR LTD**

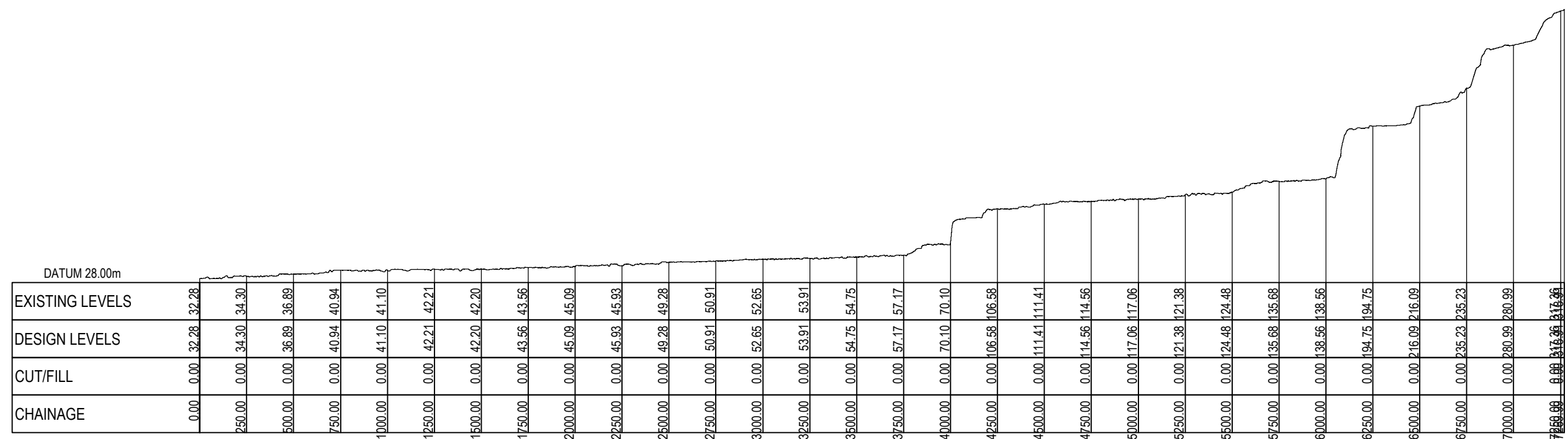
Title  
**FLOOD MODELLING  
 CATCHMENT  
 LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDENZK.DWG
Drawing no.	SK002
Rev	<b>A</b>

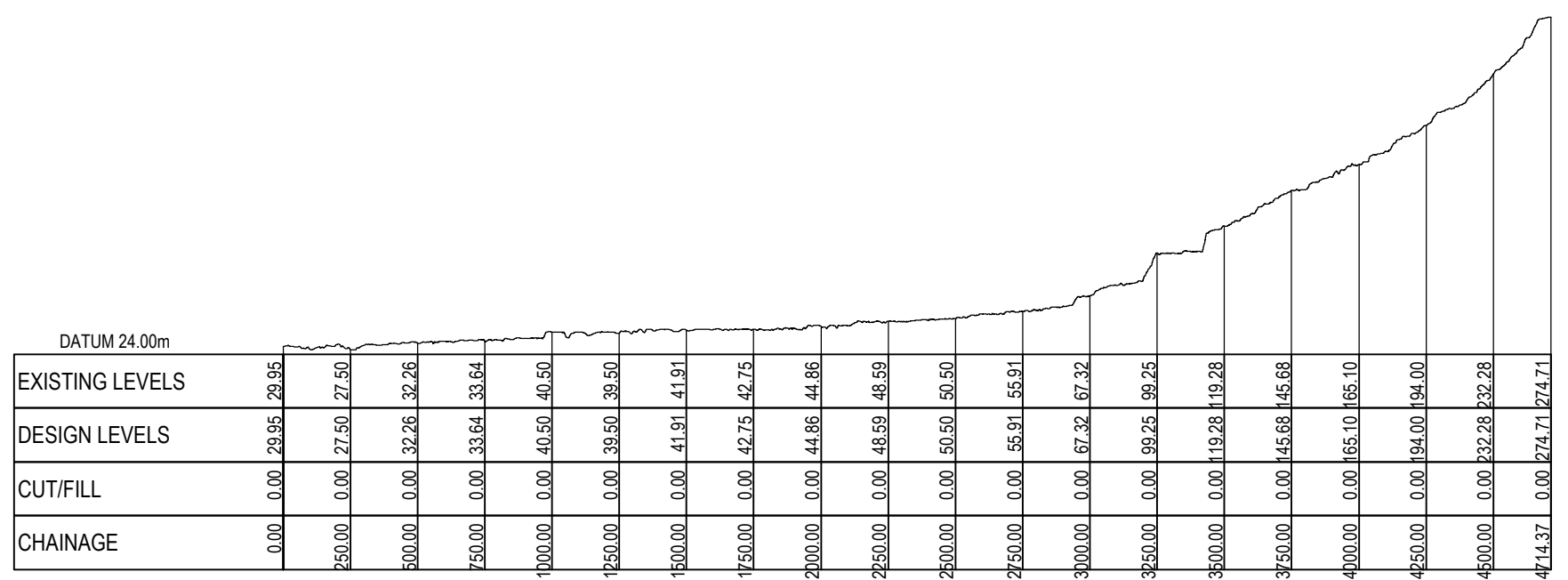
**DRAFT FOR REVIEW**

DATE: 4/8/23

Notes  
 1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.



Upstream E Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3



Upstream F Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3

**DRAFT FOR REVIEW**

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023
Survey	##		MM/YYYY
Design	##		MM/YYYY
Drawn	BY		MM/YYYY
Checked	##	###	

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Project  
**WARKWORTH SOUTH  
 PLAN CHANGE FOR  
 KA WAIMANAWA LP &  
 STEPPING TOWARDS  
 FAR LTD**

Title  
**FLOOD MODELLING  
 CATCHMENT  
 LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDEN2K.DWG
Drawing no.	SK003
Rev	<b>A</b>

DATE: 4/8/23

DATUM 24.00m

EXISTING LEVELS							
DESIGN LEVELS							
CUT/FILL							
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1186.53	

Upstream Reach ABE to Inflow  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

DATUM 28.00m

EXISTING LEVELS													
DESIGN LEVELS													
CUT/FILL													
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1250.00	1500.00	1750.00	2000.00	2250.00	2500.00	2750.00	2864.59

Upstream Reach CD TO ABE  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

Notes  
1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023
Survey	##		MM/YYYY
Design	##		MM/YYYY
Drawn	BY		MM/YYYY
Checked	##	###	

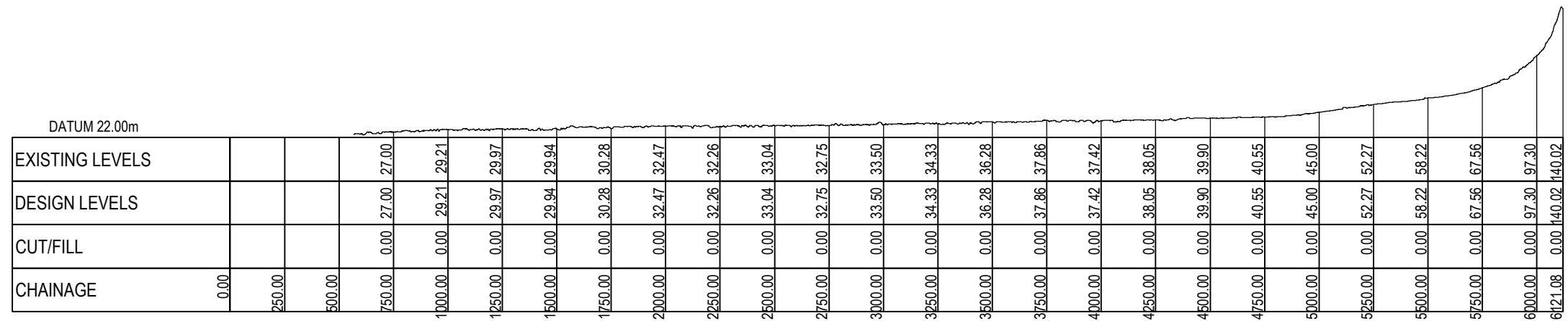


Project  
**WARKWORTH SOUTH  
PLAN CHANGE FOR  
KA WAIMANAWA LP &  
STEPPING TOWARDS  
FAR LTD**

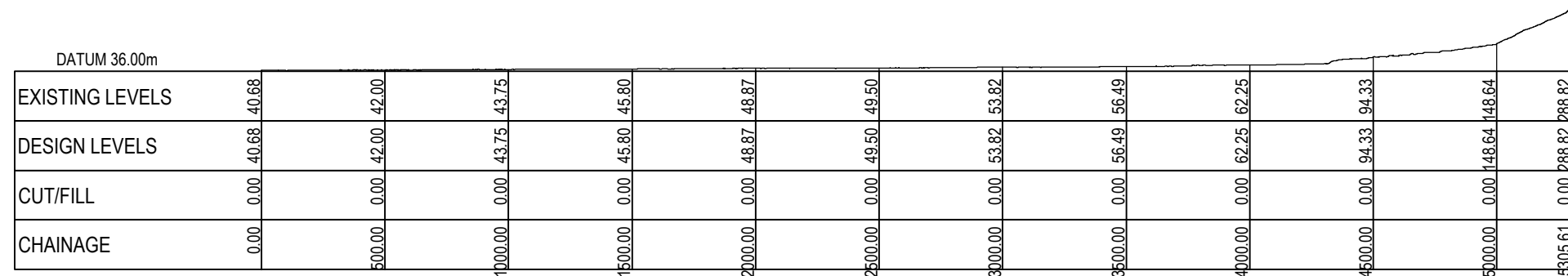
Title  
**FLOOD MODELLING  
CATCHMENT  
LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDEN2K.DWG
Drawing no.	SK004
Rev	<b>A</b>

**DRAFT FOR REVIEW**



Downstream A Longsection  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3



Downstream B Longsection  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

Notes  
1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023
Survey	##		MM/YYYY
Design	##		MM/YYYY
Drawn	BY		MM/YYYY
Checked	##	###	



Project  
**WARKWORTH SOUTH  
PLAN CHANGE FOR  
KA WAIMANAWA LP &  
STEPPING TOWARDS  
FAR LTD**

Title  
**FLOOD MODELLING  
CATCHMENT  
LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDENZK.DWG
Drawing no.	SK005
Rev	<b>A</b>

**DRAFT FOR REVIEW**

DATE: 4/8/23

DATUM 8.00m

EXISTING LEVELS	11.10	12.01	15.07	15.32	25.65	27.77
DESIGN LEVELS	11.10	12.01	15.07	15.32	25.65	27.77
CUT/FILL	0.00	0.00	0.00	0.00	0.00	0.00
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1077.43

Downstream C Longsection

HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

DATUM 8.00m

EXISTING LEVELS	11.10	15.39	16.50	18.89	20.21	21.24	23.25	26.41	29.58	31.42	32.24	33.74	43.47	79.73
DESIGN LEVELS	11.10	15.39	16.50	18.89	20.21	21.24	23.25	26.41	29.58	31.42	32.24	33.74	43.47	79.73
CUT/FILL	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1250.00	1500.00	1750.00	2000.00	2250.00	2500.00	2750.00	3000.00	3331.05

Downstream D Longsection

HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

DATUM 6.00m

EXISTING LEVELS	8.98	11.99	10.36	18.24	26.47	34.17	44.66	57.08
DESIGN LEVELS	8.98	11.99	10.36	18.24	26.47	34.17	44.66	57.08
CUT/FILL	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1250.00	1500.00	1695.06

Downstream E Longsection

HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

- Notes
- Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023
Survey	##		MM/YYYY
Design	##		MM/YYYY
Drawn	BY		MM/YYYY
Checked	##	###	

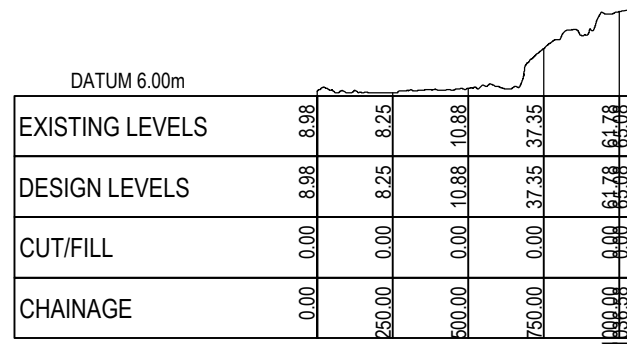
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Project  
**WARKWORTH SOUTH  
PLAN CHANGE FOR  
KA WAIMANAWA LP &  
STEPPING TOWARDS  
FAR LTD**

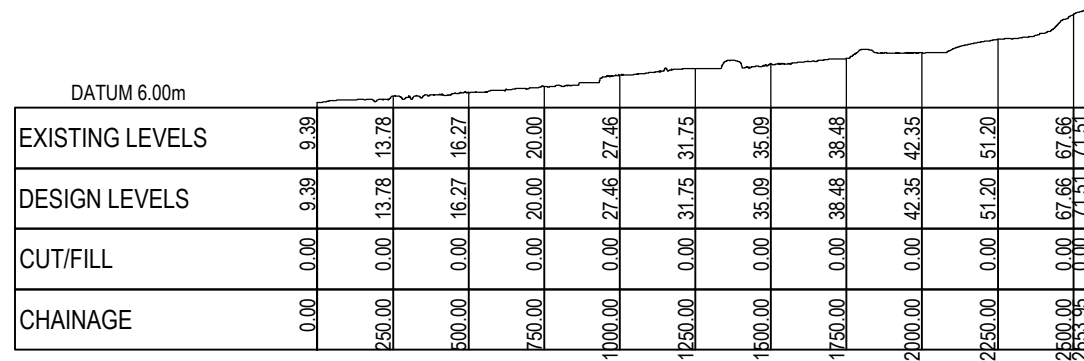
Title  
**FLOOD MODELLING  
CATCHMENT  
LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDENZK.DWG
Drawing no.	SK006
Rev	<b>A</b>

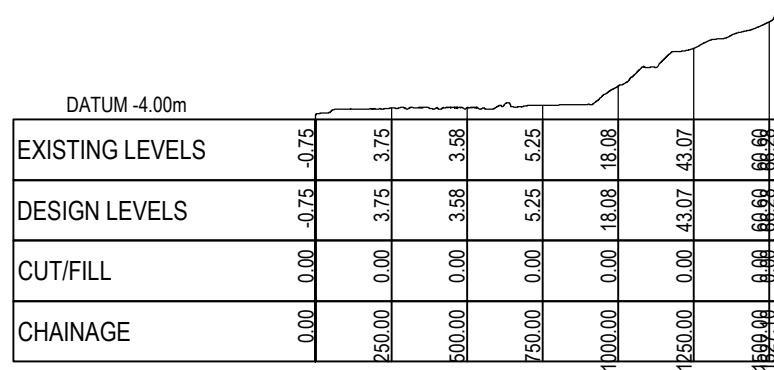
**DRAFT FOR REVIEW**



Downstream F Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3



Downstream G Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3



Downstream H Longsection  
 HORIZONTAL SCALE 1:25000 @ A3  
 VERTICAL SCALE 1:5000 @ A3

Notes  
 1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023

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Project  
**WARKWORTH SOUTH  
 PLAN CHANGE FOR  
 KA WAIMANAWA LP &  
 STEPPING TOWARDS  
 FAR LTD**

Title  
**FLOOD MODELLING  
 CATCHMENT  
 LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDENZK.DWG
Drawing no.	SK007
Rev	<b>A</b>

**DRAFT FOR REVIEW**

DATE: 4/8/23



DATUM 12.00m

EXISTING LEVELS	0.00	15.00	15.00	16.00	16.25	16.25	18.76	18.76	19.25	19.25	21.21	21.21	22.75	22.75	21.19	21.19	25.66	25.66	23.47	23.47	25.60	25.60	26.79	26.79	31.12	31.12
DESIGN LEVELS	0.00	15.00	16.00	16.25	16.25	18.76	18.76	19.25	19.25	21.21	21.21	22.75	22.75	21.19	21.19	25.66	25.66	23.47	23.47	25.60	25.60	26.79	26.79	31.12	31.12	
CUT/FILL	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1250.00	1500.00	1750.00	2000.00	2250.00	2500.00	2750.00	3000.00	3250.00	3500.00	3750.00	4000.00	4250.00	4500.00	4750.00	5000.00	5250.00	5500.00	5750.00	6000.00	

Reach Scheme In to Scheme Out  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

DATUM 12.00m

EXISTING LEVELS				29.43	29.86	30.15	31.76	33.75	32.25	33.75	33.58	33.34	34.85	35.73	37.29	37.03	38.50	37.84	40.12	42.08	41.68
DESIGN LEVELS	15.02	23.42																			
CUT/FILL																					
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1250.00	1500.00	1750.00	2000.00	2250.00	2500.00	2750.00	3000.00	3250.00	3500.00	3750.00	4000.00	4250.00	4500.00	4750.00	5000.00

Downstream Reach B to A Scheme Out  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

Notes  
1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023
Survey	##		MM/YYYY
Design	##		MM/YYYY
Drawn	BY		MM/YYYY
Checked	##	###	



Project  
**WARKWORTH SOUTH  
PLAN CHANGE FOR  
KA WAIMANAWA LP &  
STEPPING TOWARDS  
FAR LTD**

Title  
**FLOOD MODELLING  
CATCHMENT  
LONGSECTION PLAN**

Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDEN2K.DWG
Drawing no.	SK008
Rev	<b>A</b>

**DRAFT FOR REVIEW**

DATE: 4/8/23

DATUM 8.00m

EXISTING LEVELS	11.10	11.77	15.05	15.33	15.00
DESIGN LEVELS	11.10	11.77	15.05	15.33	15.00
CUT/FILL	0.00	0.00	0.00	0.00	0.00
CHAINAGE	0.00	250.00	500.00	750.00	1006.00

Downstream Reach Scheme Out B to CD  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

DATUM 6.00m

EXISTING LEVELS	9.38	9.81	9.88	11.71	11.00	11.02
DESIGN LEVELS	9.38	9.81	9.88	11.71	11.00	11.02
CUT/FILL	0.00	0.00	0.00	0.00	0.00	0.00
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1003.55

Downstream Reach CD to EFG  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

DATUM 0.00m

EXISTING LEVELS	2.75	3.92	3.45	5.74	8.00	9.41
DESIGN LEVELS	2.75	3.92	3.45	5.74	8.00	9.41
CUT/FILL	0.00	0.00	0.00	0.00	0.00	0.00
CHAINAGE	0.00	250.00	500.00	750.00	1000.00	1078.37

Downstream Reach EFH to Out  
HORIZONTAL SCALE 1:25000 @ A3  
VERTICAL SCALE 1:5000 @ A3

Notes  
1. Co-ordinates in terms of NZ Geodetic Datum Mt Eden 2000. Levels in terms of the Auckland Vertical Datum 1946.

Rev	Description	By	Date
A	FOR INFORMATION	YW	03/2023
Survey	##	MM/YYYY	
Design	##	MM/YYYY	
Drawn	BY	MM/YYYY	
Checked	##	###	



Project  
**WARKWORTH SOUTH  
PLAN CHANGE FOR  
KA WAIMANAWA LP &  
STEPPING TOWARDS  
FAR LTD**

Title  
**FLOOD MODELLING  
CATCHMENT  
LONGSECTION PLAN**

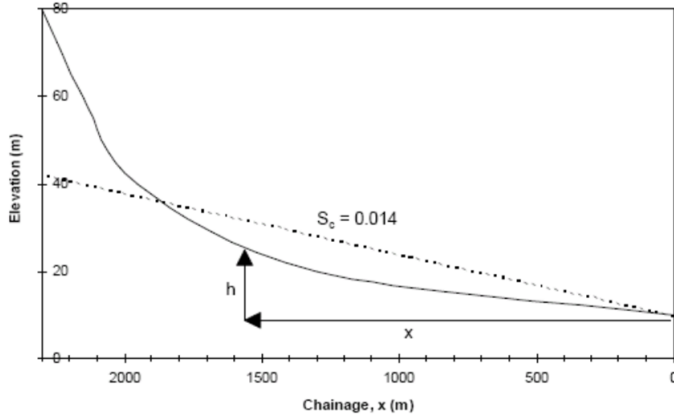
Project no.	211001
Scale	
Cad file	CATCHMENT BODYS & LENGTHS EDEN2K.DWG
Drawing no.	SK009
Rev	<b>A</b>

**DRAFT FOR REVIEW**

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Catchment A	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	32.3	0	0			
2	42.75	10.45	1500	1500	5.225	7837.5
3	52	19.7	3000	1500	15.075	22612.5
4	64	31.7	3750	750	25.7	19275
5	99.4	67.1	4175	425	49.4	20995
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			4175	<b>TOTAL =</b>		70720

$S_c =$  0.008

**Post-development**

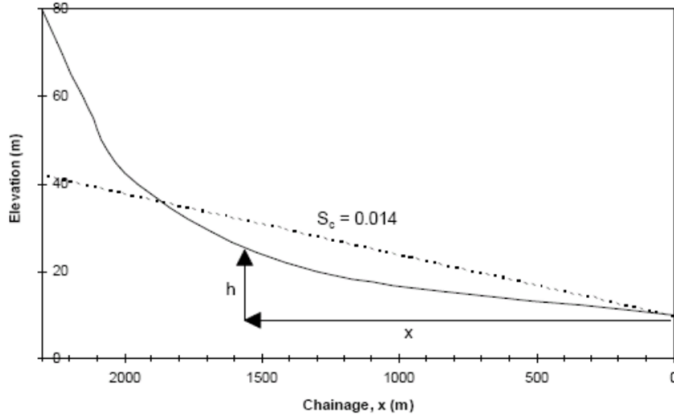
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Catchment B	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	32.3	0	0			
2	40.6	8.3	1750	1750	4.15	7262.5
3	50.9	18.6	3000	1250	13.45	16812.5
4	57.3	25	3250	250	21.8	5450
5	88	55.7	4250	1000	40.35	40350
6	169.8	137.5	4660	410	96.6	39606
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			4660	<b>TOTAL =</b>		109481

$S_c =$  0.010

**Post-development**

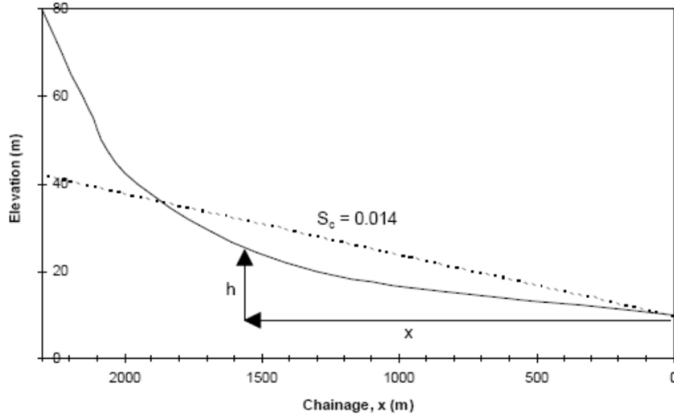
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Catchment C	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	45.5	0	0			
2	70.6	25.1	1500	1500	12.55	18825
3	82.6	37.1	2750	1250	31.1	38875
4	101	55.5	3500	750	46.3	34725
5	133.6	88.1	3750	250	71.8	17950
6	215.5	170	4750	1000	129.05	129050
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			4750	<b>TOTAL =</b>		239425

$S_c =$  0.021

**Post-development**

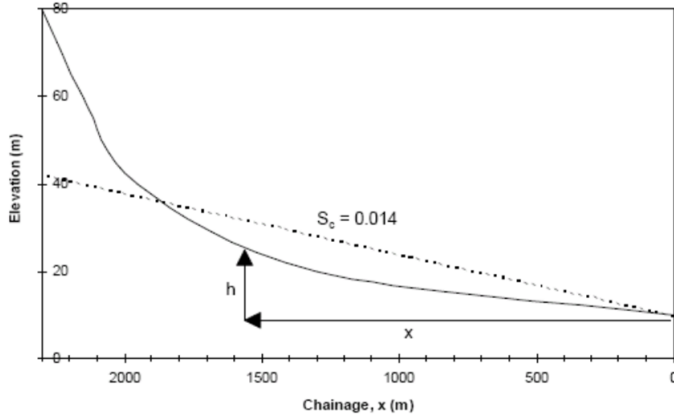
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Catchment D	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	46.95	0	0			
2	76	29.05	1750	1750	14.525	25418.75
3	92.5	45.55	3250	1500	37.3	55950
4	142	95.05	4500	1250	70.3	87875
5	205.84	158.89	5750	1250	126.97	158712.5
6	317	270.05	6750	1000	214.47	214470
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			6750	<b>TOTAL =</b>		542426.25

$S_c =$  0.024

**Post-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

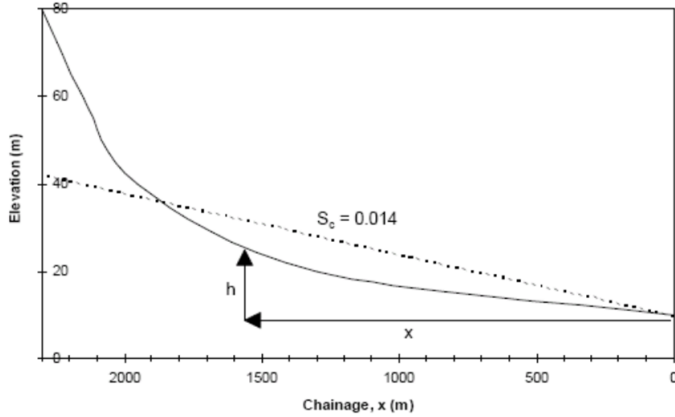
$S_c =$  #DIV/0!



<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Catchment E	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	32.3	0	0			
2	45.1	12.8	2000	2000	6.4	12800
3	57.2	24.9	3750	1750	18.85	32987.5
4	111.4	79.1	4500	750	52	39000
5	135.7	103.4	5750	1250	91.25	114062.5
6	194.8	162.5	6250	500	132.95	66475
7	317	284.7	7250	1000	223.6	223600
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			7250	<b>TOTAL =</b>		488925

$S_c =$  0.019

**Post-development**

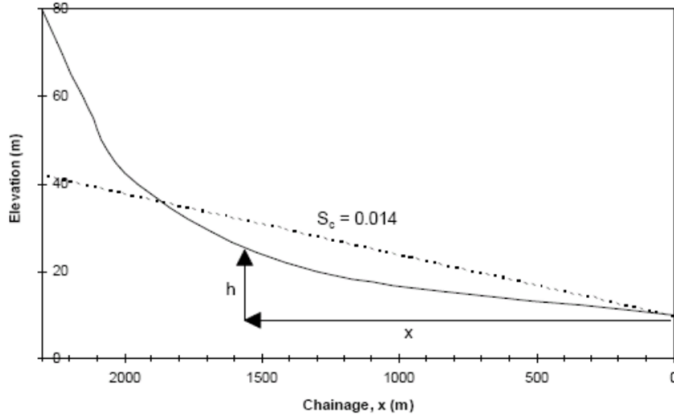
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Catchment F	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	29.95	0	0			
2	44.86	14.91	2000	2000	7.455	14910
3	55.91	25.96	2750	750	20.435	15326.25
4	99.25	69.3	3250	500	47.63	23815
5	165.1	135.15	4000	750	102.225	76668.75
6	274.7	244.75	4714.4	714.4	189.95	135700.28
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			4714.4	<b>TOTAL =</b>		266420.28

$S_c =$  0.024

**Post-development**

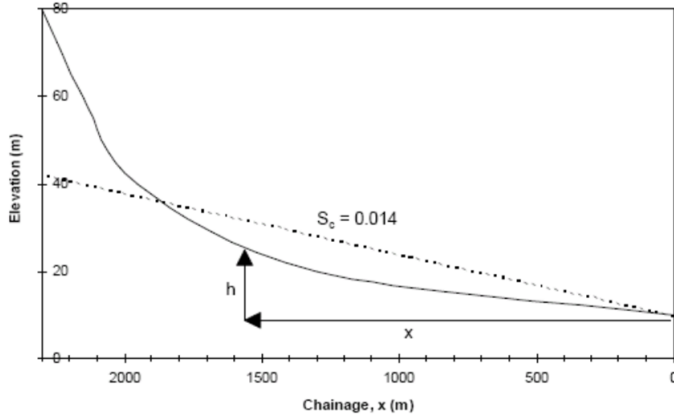
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Reach ABE to Scheme Inflow	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	29.95	0	0			
2	31.9	1.95	500	500	0.975	487.5
3	32.01	2.06	750	250	2.005	501.25
4	32.28	2.33	1186	436	2.195	957.02
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1186		<b>TOTAL =</b>	1945.77

**S<sub>c</sub> =** 0.003

**Post-development**

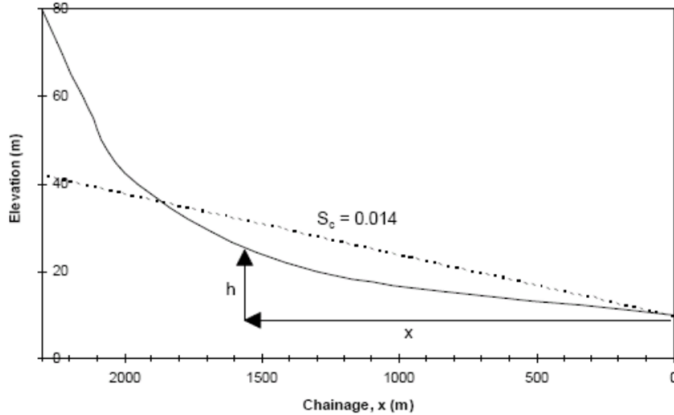
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0		<b>TOTAL =</b>	0

**S<sub>c</sub> =** #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Upstream Reach CD to ABE	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells   
 Result cells

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	32.3	0	0			
2	37.2	4.9	1000	1000	2.45	2450
3	39.1	6.8	1500	500	5.85	2925
4	42.1	9.8	2250	750	8.3	6225
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			2250	<b>TOTAL =</b>		11600

$S_c =$  0.005

**Post-development**

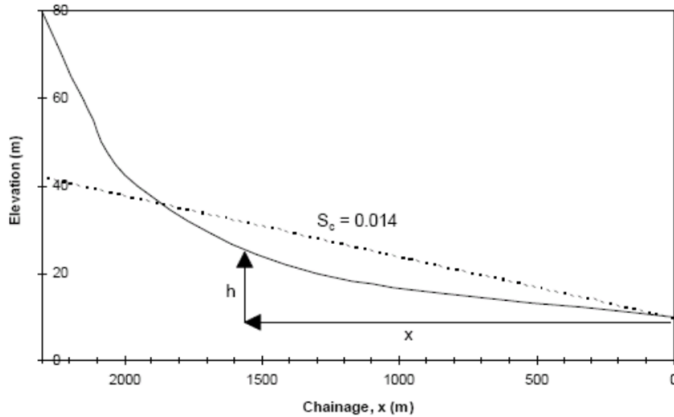
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream A	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	27	0	0			
2	33.5	6.5	3000	3000	3.25	9750
3	39.9	12.9	4500	1500	9.7	14550
4	67.56	40.56	5750	1250	26.73	33412.5
5	140	113	6120	370	76.78	28408.6
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			6120	<b>TOTAL =</b>		86121.1

**S<sub>c</sub> =** 0.005

**Post-development**

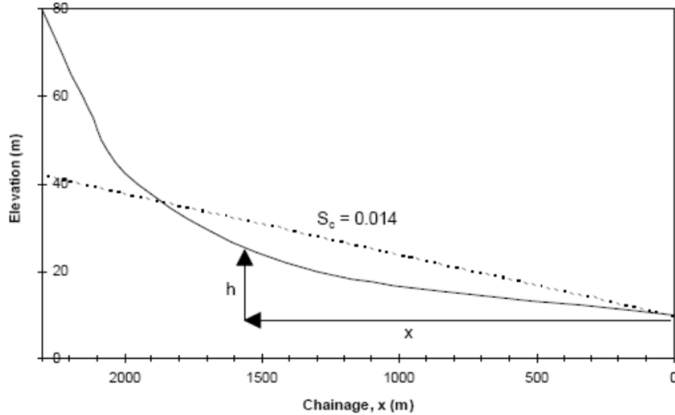
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

**S<sub>c</sub> =** #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream B	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	40.7	0	0			
2	48.87	8.17	2000	2000	4.085	8170
3	56.5	15.8	3500	1500	11.985	17977.5
4	94.3	53.6	4500	1000	34.7	34700
5	148.64	107.94	5000	500	80.77	40385
6	288.82	248.12	5306	306	178.03	54477.18
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			5306	<b>TOTAL =</b>		155709.68

$S_c =$  0.011

**Post-development**

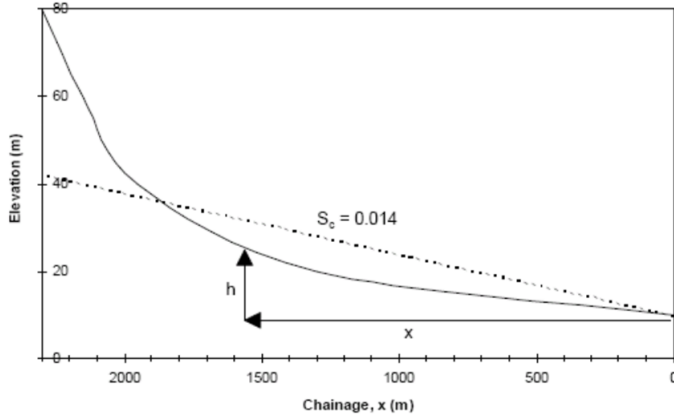
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream C	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	11.1	0	0			
2	15.07	3.97	500	500	1.985	992.5
3	15.3	4.2	750	250	4.085	1021.25
4	25.67	14.57	1000	250	9.385	2346.25
5	27.77	16.67	1077	77	15.62	1202.74
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1077		<b>TOTAL =</b>	5562.74

$S_c =$  0.010

**Post-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0		<b>TOTAL =</b>	0

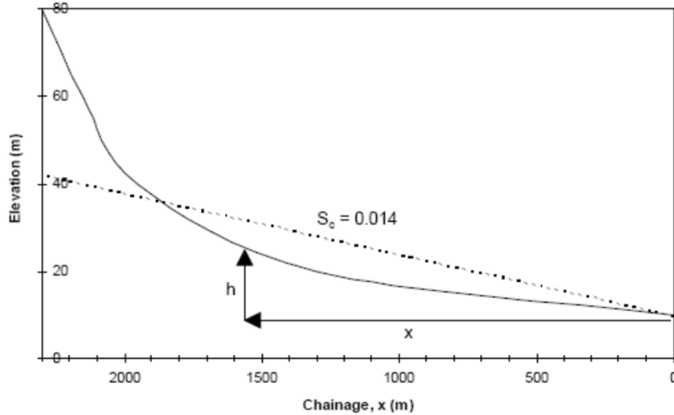
$S_c =$  #DIV/0!



<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream D	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	11.1	0	0			
2	20.2	9.1	1000	1000	4.55	4550
3	29.6	18.5	2000	1000	13.8	13800
4	33.74	22.64	2750	750	20.57	15427.5
5	43.5	32.4	3000	250	27.52	6880
6	79.7	68.6	3250	250	50.5	12625
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			<b>3250</b>	<b>TOTAL =</b>		<b>53282.5</b>

$S_c =$  0.010

**Post-development**

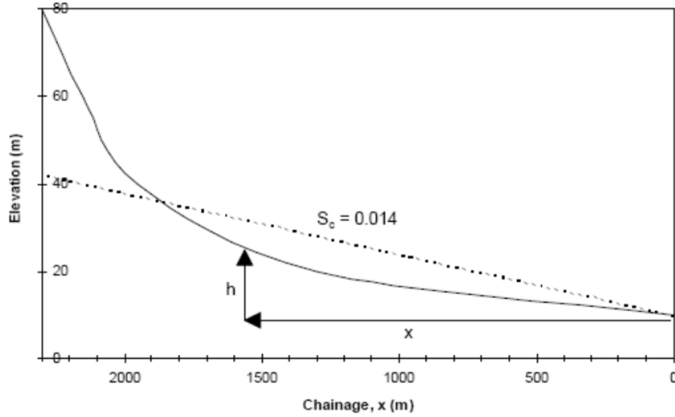
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			<b>0</b>	<b>TOTAL =</b>		<b>0</b>

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream E	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	8.9	0	0			
2	10.36	1.46	500	500	0.73	365
3	18.24	9.34	750	250	5.4	1350
4	34.2	25.3	1250	500	17.32	8660
5	57.1	48.2	1695	445	36.75	16353.75
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1695	<b>TOTAL =</b>		26728.75

$S_c =$  0.019

**Post-development**

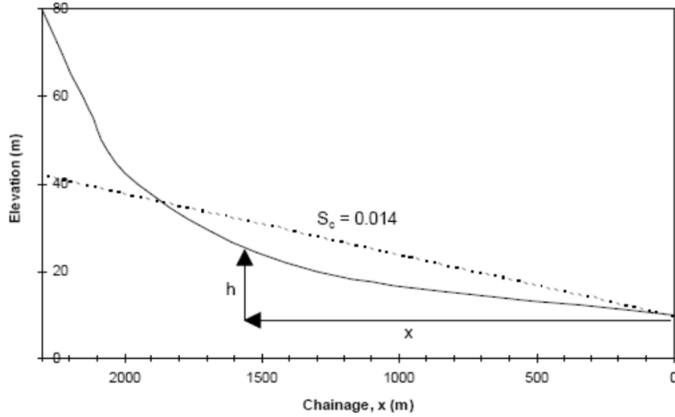
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream F	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	8.98	0	0			
2	10.88	1.9	500	500	0.95	475
3	37.35	28.37	750	250	15.135	3783.75
4	65	56.02	1036	286	42.195	12067.77
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1036	<b>TOTAL =</b>		16326.52

$S_c =$  0.030

**Post-development**

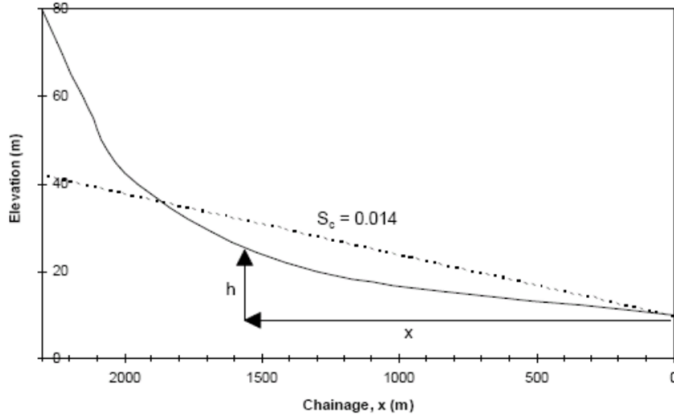
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream G	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	9.4	0	0			
2	27.5	18.1	1000	1000	9.05	9050
3	38.5	29.1	1750	750	23.6	17700
4	71.5	62.1	2553	803	45.6	36616.8
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			2553		<b>TOTAL =</b>	63366.8

$S_c =$  0.019

**Post-development**

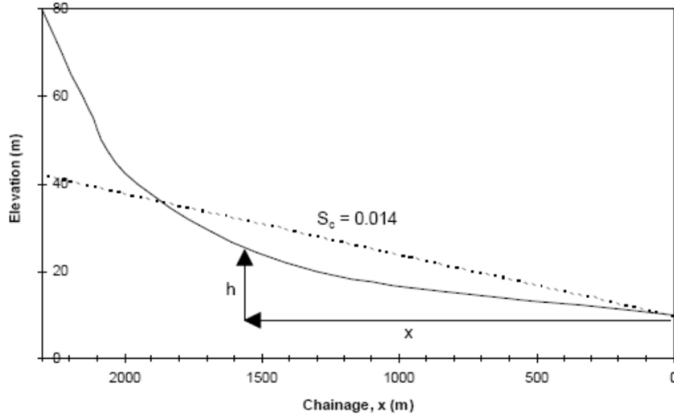
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0		<b>TOTAL =</b>	0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream H	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	0	0	0			
2	3.75	3.75	250	250	1.875	468.75
3	5.25	5.25	750	500	4.5	2250
4	18.1	18.1	1000	250	11.675	2918.75
5	43.1	43.1	1250	250	30.6	7650
6	60	60	1520	270	51.55	13918.5
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1520		<b>TOTAL =</b>	27206

$S_c =$  0.024

**Post-development**

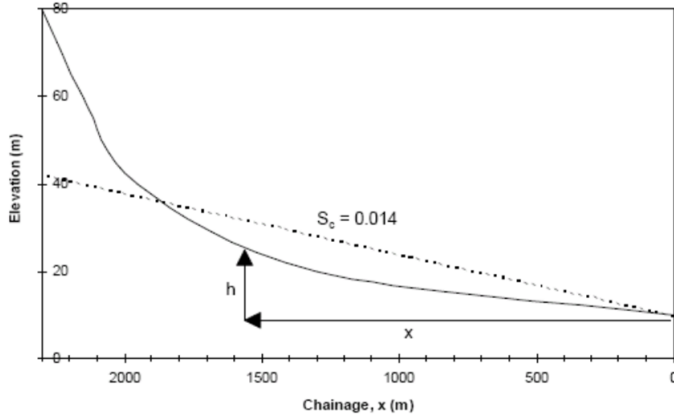
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0		<b>TOTAL =</b>	0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream Reach Scheme In to Out	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	15	0	0			
2	19.25	4.25	1000	1000	2.125	2125
3	25.66	10.66	2000	1000	7.455	7455
4	31.1	16.1	2973	973	13.38	13018.74
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			2973		<b>TOTAL =</b>	22598.74

$S_c =$  0.005

**Post-development**

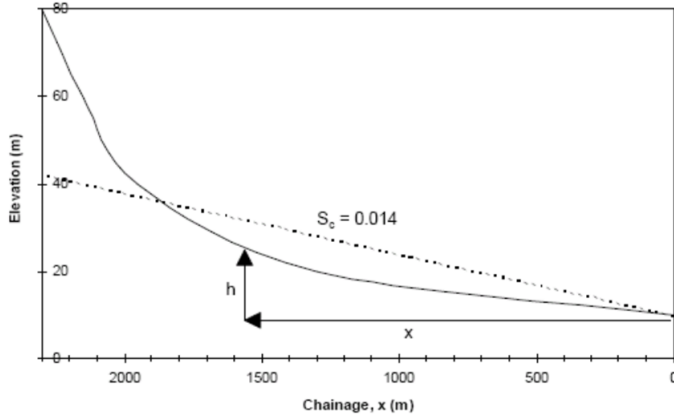
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0		<b>TOTAL =</b>	0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream Reach Bto Scheme Out	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	15	0	0			
2	29.43	14.43	750	750	7.215	5411.25
3	32.25	17.25	2000	1250	15.84	19800
4	35.73	20.73	3250	1250	18.99	23737.5
5	41.7	26.7	4850	1600	23.715	37944
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			4850	<b>TOTAL =</b>		86892.75

$S_c =$  0.007

**Post-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

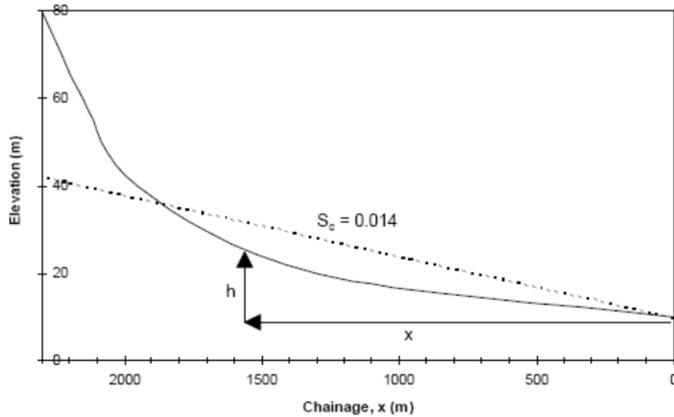
$S_c =$  #DIV/0!



<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream Reach Scheme Out B to CD	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	11.1	0	0			
2	11.77	0.67	250	250	0.335	83.75
3	15	3.9	1000	750	2.285	1713.75
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>				1000	<b>TOTAL =</b>	1797.5

$S_c =$  0.004

**Post-development**

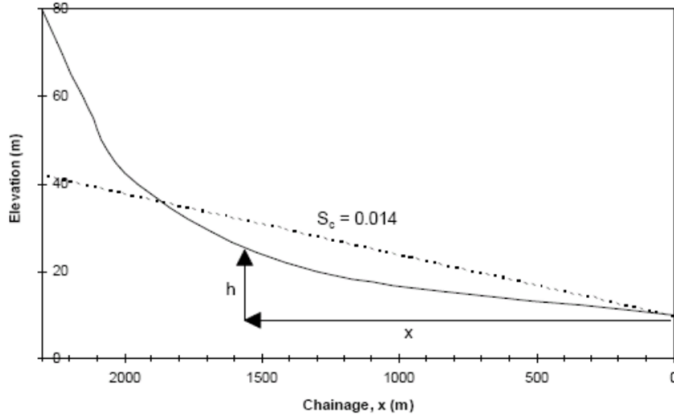
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>				0	<b>TOTAL =</b>	0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream Reach CD to EFG	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	9.38	0	0			
2	9.8	0.42	250	250	0.21	52.5
3	11	1.62	1000	750	1.02	765
4	11.11	1.73	1303	303	1.675	507.525
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1303	<b>TOTAL =</b>		1325.025

$S_c =$  0.002

**Post-development**

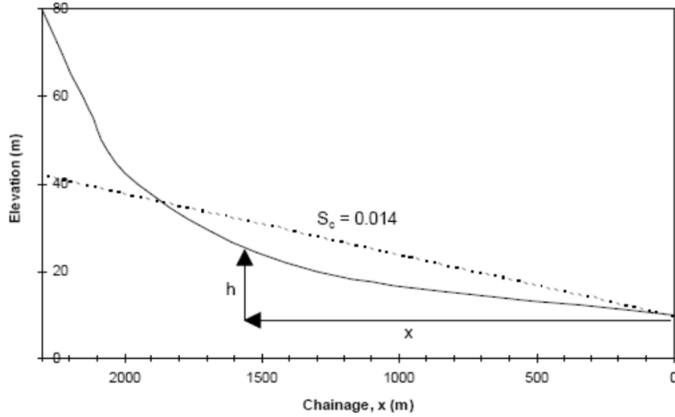
Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

<b>JOB NAME:</b> Warkwork South Plan Change	<b>DATE:</b> 30/03/2023
<b>JOB NO:</b> 211001	<b>DES BY:</b> YW
<b>SUBJECT:</b> Downstream Reach EFG TO OUT	<b>CHKD BY:</b>

**Catchment Slope**

(Calculating the Slope (Sc) using the equal area method)



$$S_c = \frac{2A}{L^2}$$

Data Entry Cells    
 Result cells  

(This graph is from the ARC TP 108, April 1999, pg.14)

**Pre-development**


Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1	2.75	0	0			
2	3.45	0.7	500	500	0.35	175
3	8	5.25	1000	500	2.975	1487.5
4	9.41	6.66	1080	80	5.955	476.4
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			1080	<b>TOTAL =</b>		2138.9

$S_c =$  0.004

**Post-development**

Point	RL (m)	(m) <i>h</i>	(m) <i>x</i>	(m) $\Delta x$	(m) $\bar{h}$	$\Delta A (= \bar{h} \Delta x)$
1		0				
2		0		0	0	0
3		0		0	0	0
4		0		0	0	0
5		0		0	0	0
6		0		0	0	0
7		0		0	0	0
8		0		0	0	0
9		0		0	0	0
10		0		0	0	0
11		0		0	0	0
12		0		0	0	0
<b>TOTAL =</b>			0	<b>TOTAL =</b>		0

$S_c =$  #DIV/0!

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Catchment A</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>30/03/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	6.0077	588.75
C	Total Pervious	74	327.7623	24254.41
* from Appendix B			Totals =	333.770 24843.16

CN (weighted) =  $\frac{\text{total product} = \frac{\text{#####}}{\text{#####}} = 74.4}{\text{total area}}$

Ia (average) =  $\frac{5 \times \text{pervious area} = \frac{5 \times 327.7623}{333.770} = 4.9 \text{ mm}}{\text{total area}}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.117 km (along drainage path)


Catchment Slope Sc = 0.008 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.4}{200 - 74.4} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 2.54 1.33 4.26 = 2.02 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = \underline{1.35}$  hrs

OK  
use  
2.0218088 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Catchment B</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>30/03/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	10.3517	1014.47
C	Total Pervious	74	401.8183	29734.55
* from Appendix B			Totals =	412.170 30749.02

CN (weighted) =  $\frac{\text{total product} = \frac{\text{#####}}{\text{#####}} = 74.6}{\text{total area}}$

Ia (average) =  $\frac{5 \times \text{pervious area} = \frac{5 \times 401.8183}{412.170} = 4.9 \text{ mm}}{\text{total area}}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.29 km (along drainage path)

Catchment Slope Sc = 0.01 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.6}{200 - 74.6} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 2.61 1.33 3.98 = 1.94 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.30 \text{ hrs}$

OK  
use  
1.939068 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Catchment C</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	3.8056	372.95
C	Total Pervious	74	287.7544	21293.83
* from Appendix B			Totals =	291.560    21666.77

CN (weighted) =  $\frac{\text{total product} = \frac{\text{#####}}{\text{#####}} = 74.3}{\text{total area}}$

Ia (average) =  $\frac{5 \times \text{pervious area} = \frac{5 \times 287.7544}{291.560} = 4.9 \text{ mm}}{\text{total area}}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.89 km (along drainage path)

Catchment Slope Sc = 0.021 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.3}{200 - 74.3} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14    1    2.85    1.34    3.19    = 1.70 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c$  = 1.14 hrs

OK  
use  
1.6979653 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Catchment D</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	4.8100	471.38
C	Total Pervious	74	505.2700	37389.98
* from Appendix B			Totals =	510.080    37861.36

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{37861.36}{510.080} = \underline{74.2}$$

$$\text{Ia (average)} = \frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 505.2700}{510.080} = 5.0 \text{ mm}$$

**2. Time of Concentration**

Channelisation factor      C = 1 (From Table 4.2)

Catchment length          L = 6.687 km (along drainage path)

Catchment Slope          Sc = 0.024 m/m (by equal area method)

$$\text{Runoff factor, } \frac{\text{CN}}{200 - \text{CN}} = \frac{74.2}{200 - 74.2} = \underline{0.59}$$


$$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$$

$$= 0.14 \quad 1 \quad 3.50 \quad 1.34 \quad 3.06 = \underline{2.01} \text{ hrs}$$

SCS Lag for HEC-HMS....       $t_p = 2/3 t_c = \underline{1.35} \text{ hrs}$

OK  
use  
2.0076355 hrs



	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Catchment E</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>30/03/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	5.7200	560.56
C	Total Pervious	74	567.5700	42000.18
* from Appendix B			Totals =	573.290 42560.74

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{42560.74}{573.290} = 74.2$$

$$\text{Ia (average)} = \frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 567.5700}{573.290} = 5.0 \text{ mm}$$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 7.153 km (along drainage path)

Catchment Slope Sc = 0.019 m/m (by equal area method)


$$\text{Runoff factor, } \frac{\text{CN}}{200 - \text{CN}} = \frac{74.2}{200 - 74.2} = 0.59$$

$$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$$

$$= 0.14 \quad 1 \quad 3.66 \quad 1.34 \quad 3.28 = 2.25 \text{ hrs}$$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.51 \text{ hrs}$

OK  
use  
2.2509428 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Catchment F</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	4.8193	472.29
C	Total Pervious	74	299.1107	22134.19
* from Appendix B			Totals =	303.930 22606.48

CN (weighted) =  $\frac{\text{total product} = 22606.48}{\text{total area} = 303.930} = 74.4$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 299.1107}{\text{total area} = 303.930} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.596 km (along drainage path)


Catchment Slope Sc = 0.024 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.4}{200 - 74.4} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 2.74 1.33 3.06 = 1.56 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.05 \text{ hrs}$

OK  
use  
1.5646387 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Post Development</b> <b>Upstream Catchment F</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	15.1030	1480.09
C	Total Pervious	74	288.8270	21373.20
* from Appendix B			Totals =	303.930 22853.29

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{22853.29}{303.930} = 75.2$$

$$\text{Ia (average)} = \frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 288.8270}{303.930} = 4.8 \text{ mm}$$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.596 km (along drainage path)

Catchment Slope Sc = 0.024 m/m (by equal area method)


$$\text{Runoff factor, } \frac{\text{CN}}{200 - \text{CN}} = \frac{75.2}{200 - 75.2} = 0.60$$

$$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$$

$$= 0.14 \times 1 \times 2.74 \times 1.32 \times 3.06 = 1.55 \text{ hrs}$$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.04 \text{ hrs}$

OK  
use  
1.5497843 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>pstream Reach Catchment ABE - Inflow</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>30/03/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	22.0794	2163.78
C	Total Pervious	74	1297.1506	95989.14
* from Appendix B			Totals =	1319.230 98152.93

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{\text{#####}}{\text{#####}} = 74.4$

Ia (average) =  $\frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 1297.1506}{1319.230} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1.186 km (along drainage path)


Catchment Slope Sc = 0.003 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.4}{200 - 74.4} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 1.12 1.33 5.71 = 1.19 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 0.80 \text{ hrs}$

OK  
use  
1.193873089 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Upstream Reach CD to ABE</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	8.6156	844.33
C	Total Pervious	74	793.0244	58683.81
* from Appendix B			Totals =	801.640 59528.13

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{\text{#####}}{\text{#####}} = 74.3$

Ia (average) =  $\frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 793.0244}{801.640} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 2.25 km (along drainage path)

Catchment Slope Sc = 0.005 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.3}{200 - 74.3} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 1.71 1.34 4.90 = 1.57 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.05 \text{ hrs}$

OK  
use  
1.5656087 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 30/03/2023

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	9.4054	921.73
C	Total Pervious	74	575.1346	42559.96
* from Appendix B			Totals =	584.540 43481.69

CN (weighted) =  $\frac{\text{total product} = 43481.69}{\text{total area} = 584.540} = 74.4$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 575.1346}{\text{total area} = 584.540} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.848 km (along drainage path)

Catchment Slope Sc = 0.005 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.4}{200 - 74.4} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 2.83 1.33 4.90 = 2.59 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.74 \text{ hrs}$

OK  
use  
2.5945274 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 30/03/2023

**Warkworth South Plan Change  
TP108 Calculation - Pre-Development  
Downstream Catchment B**

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	12.0476	1180.66
C	Total Pervious	74	836.7124	61916.72
* from Appendix B			Totals =	848.760 63097.38

CN (weighted) =  $\frac{\text{total product} = 63097.38}{\text{total area} = 848.760} = 74.3$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 836.7124}{\text{total area} = 848.760} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 5.031 km (along drainage path)


Catchment Slope Sc = 0.011 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.3}{200 - 74.3} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
 $= 0.14 \times 1 \times 2.90 \times 1.33 \times 3.87 = 2.10 \text{ hrs}$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.41 \text{ hrs}$

OK  
use  
2.0998325 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Downstream Catchment C</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>30/03/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	7.3200	717.36
C	Total Pervious	74	16.2500	1202.50
* from Appendix B			Totals =	1919.86

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{1919.86}{23.570} = 81.5$$

$$\text{Ia (average)} = \frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 16.2500}{23.570} = 3.4 \text{ mm}$$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1.018 km (along drainage path)

Catchment Slope Sc = 0.01 m/m (by equal area method)

$$\text{Runoff factor, } \frac{\text{CN}}{200 - \text{CN}} = \frac{81.5}{200 - 81.5} = 0.69$$


$$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$$

$$= 0.14 \times 1 \times 1.01 \times 1.23 \times 3.98 = 0.69 \text{ hrs}$$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 0.46 \text{ hrs}$

OK  
use  
0.6932339 hrs



 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 18/01/2023

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	8.4586	828.94
C	Total Pervious	74	275.4014	20379.70
* from Appendix B			Totals =	283.860    21208.65

CN (weighted) =  $\frac{\text{total product} = 21208.65}{\text{total area} = 283.860} = 74.7$   
 Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 275.4014}{\text{total area} = 283.860} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)  
 Catchment length L = 3.056 km (along drainage path)  
 Catchment Slope Sc = 0.01 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.7}{200 - 74.7} = 0.60$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
 $= 0.14 \times 1 \times 2.09 \times 1.33 \times 3.98 = 1.55 \text{ hrs}$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.04 \text{ hrs}$

OK  
use  
1.5480986 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Down Catchment E</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	45.2715	4436.61
C	Total Pervious	74	26.8785	1989.01
* from Appendix B			Totals =	72.150      6425.62

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{6425.62}{72.150} = 89.1$$

$$\text{Ia (average)} = \frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 26.8785}{72.150} = 1.9 \text{ mm}$$

**2. Time of Concentration**

Channelisation factor      C = 1 (From Table 4.2)

Catchment length      L = 1.665 km (along drainage path)

Catchment Slope      Sc = 0.019 m/m (by equal area method)

$$\text{Runoff factor, } \frac{\text{CN}}{200 - \text{CN}} = \frac{89.1}{200 - 89.1} = 0.80$$


$$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$$

$$= 0.14 \times 1 \times 1.40 \times 1.13 \times 3.28 = 0.73 \text{ hrs}$$

SCS Lag for HEC-HMS....       $t_p = 2/3 t_c = 0.49 \text{ hrs}$

OK  
use  
0.7262942 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Downstream Catchment F</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	0.2200	21.56
C	Total Pervious	74	21.5700	1596.18
* from Appendix B			Totals =	21.790 1617.74

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{1617.74}{21.790} = 74.2$$

$$\text{Ia (average)} = \frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 21.5700}{21.790} = 4.9 \text{ mm}$$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1.076 km (along drainage path)

Catchment Slope Sc = 0.03 m/m (by equal area method)


$$\text{Runoff factor, } \frac{\text{CN}}{200 - \text{CN}} = \frac{74.2}{200 - 74.2} = 0.59$$

$$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$$

$$= 0.14 \times 1 \times 1.05 \times 1.34 \times 2.86 = 0.56 \text{ hrs}$$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 0.38 \text{ hrs}$

OK  
use  
0.5621775 hrs

 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 18/01/2023

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2= 1ha	Product of CN x area
C	Total Impervious	98	67.7100	6635.58
C	Total Pervious	74	59.8900	4431.86
* from Appendix B			Totals =	127.600    11067.44

CN (weighted) =  $\frac{\text{total product} = 11067.44}{\text{total area} = 127.600} = 86.7$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 59.8900}{\text{total area} = 127.600} = 2.3 \text{ mm}$

**2. Time of Concentration**

Channelisation factor            C = 1 (From Table 4.2)

Catchment length                L = 2.412 km (along drainage path)


Catchment Slope                Sc = 0.019 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{86.7}{200 - 86.7} = 0.77$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
 = 0.14            1            1.79            1.16            3.28            = 0.95 hrs

SCS Lag for HEC-HMS....             $t_p = 2/3 t_c$             = 0.64 hrs

OK  
use  
0.9519511 hrs

 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 18/01/2023

**Warkworth South Plan Change  
TP108 Calculation - Pre-Development  
Downstream Catchment H**

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	23.6020	2313.00
C	Total Pervious	74	25.5680	1892.03
* from Appendix B			Totals =	49.170      4205.03

CN (weighted) =  $\frac{\text{total product} = 4205.03}{\text{total area} = 49.170} = 85.5$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 25.5680}{\text{total area} = 49.170} = 2.6 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1.471 km (along drainage path)

Catchment Slope Sc = 0.024 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN} = 85.5}{200 - \text{CN} = 200 - 85.5} = 0.75$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14      1      1.29      1.17      3.06      = 0.65 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 0.43 \text{ hrs}$

OK  
use  
0.6491606 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Reach Scheme in to out</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>18/01/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	30.9143	3029.60
C	Total Pervious	74	2111.7450	156269.13
* from Appendix B			Totals =	2142.659 159298.73

CN (weighted) =  $\frac{\text{total product} = \text{#####}}{\text{total area} = 2142.659} = 74.3$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 2111.7450}{\text{total area} = 2142.659} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 2.973 km (along drainage path)


Catchment Slope Sc = 0.005 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.3}{200 - 74.3} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 2.05 1.33 4.90 = 1.88 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.26 \text{ hrs}$

OK  
use  
1.87974775 hrs

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change TP108</b> <b>Calculation - Pre-Development</b> <b>Downstream Reach B to SCHEME OUT</b>	Author YW	Date 30/03/2023

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m <sup>2</sup> = 1ha	Product of CN x area
C	Total Impervious	98	12.0476	1180.66
C	Total Pervious	74	836.7124	61916.72
* from Appendix B			Totals =	848.760    63097.38

CN (weighted) =  $\frac{\text{total product} = 63097.38}{\text{total area} = 848.760} = 74.3$

Ia (average) =  $\frac{5 \times \text{pervious area} = 5 \times 836.7124}{\text{total area} = 848.760} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 4.85 km (along drainage path)


Catchment Slope Sc = 0.007 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.3}{200 - 74.3} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14    1    2.84    1.33    4.43    = 2.35 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 1.57 \text{ hrs}$

OK  
use  
2.3473116 hrs

 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 18/01/2023

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	68.7906	6741.48
C	Total Pervious	74	3992.4987	295444.90
* from Appendix B			Totals =	4061.289 302186.38

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{\text{#####}}{4061.289} = \underline{\underline{74.4}}$

Ia (average) =  $\frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 3992.4987}{4061.289} = \underline{\underline{4.9 \text{ mm}}}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1 km (along drainage path)

Catchment Slope Sc = 0.004 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.4}{200 - 74.4} = \underline{\underline{0.59}}$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 1.00 1.33 5.24 = 0.98 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = \underline{\underline{0.66}}$  hrs

OK  
use  
0.97848894 hrs



 <b>MAVEN ASSOCIATES</b>	<b>Job Number</b> 211001	<b>Sheet</b> 1	<b>Rev</b> A
	<b>Job Title</b> <b>Calc Title</b>	<b>Author</b> YW	<b>Date</b> 18/01/2023

Warkworth South Plan Change  
TP108 Calculation - Pre-Development  
Downstream Reach CD to EFG

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	84.5692	8287.78
C	Total Pervious	74	4284.1501	317027.11
* from Appendix B			Totals =	4368.719 325314.89

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{\text{#####}}{4368.719} = 74.5$

Ia (average) =  $\frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 4284.1501}{4368.719} = 4.9 \text{ mm}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1 km (along drainage path)

Catchment Slope Sc = 0.002 m/m (by equal area method)


Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{74.5}{200 - 74.5} = 0.59$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
 $= 0.14 \times 1 \times 1.00 \times 1.33 \times 6.45 = 1.20 \text{ hrs}$

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = 0.81 \text{ hrs}$

OK  
use  
1.2038381 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

	MAVEN ASSOCIATES	Job Number 211001	Sheet 1	Rev A
	<b>Job Title</b> <b>Calc Title</b>	<b>Warkworth South Plan Change</b> <b>TP108 Calculation - Pre-Development</b> <b>Downstream Reach EFG to Outlet</b>	<b>Author</b> <b>YW</b>	<b>Date</b> <b>31/03/2023</b>

**1. Runoff Curve Number (CN) and initial Abstraction (Ia)**

Soil name and classification	Cover description (cover type, treatment, and hydrologic condition)	Curve Number CN*	Area (ha) 10000m2=1ha	Product of CN x area
C	Total Impervious	98	197.7707	19381.53
C	Total Pervious	74	4392.4886	325044.16
* from Appendix B			Totals =	4590.259 344425.69

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{\text{#####}}{4590.259} = \underline{\underline{75.0}}$

Ia (average) =  $\frac{5 \times \text{pervious area}}{\text{total area}} = \frac{5 \times 4392.4886}{4590.259} = \underline{\underline{4.8 \text{ mm}}}$

**2. Time of Concentration**

Channelisation factor C = 1 (From Table 4.2)

Catchment length L = 1.08 km (along drainage path)

Catchment Slope Sc = 0.004 m/m (by equal area method)

Runoff factor,  $\frac{\text{CN}}{200 - \text{CN}} = \frac{75.0}{200 - 75.0} = \underline{\underline{0.60}}$

$t_c = 0.14 C L^{0.66} (\text{CN}/200 - \text{CN})^{-0.55} S_c^{-0.30}$   
= 0.14 1 1.05 1.32 5.24 = 1.02 hrs

SCS Lag for HEC-HMS....  $t_p = 2/3 t_c = \underline{\underline{0.68 \text{ hrs}}}$

OK  
use  
1.02191107 hrs

**Worksheet 1: Runoff Parameters and Time of Concentration**

**APPENDIX D – TP108 and Time of concentration calculations**

# Channel Report

## WOODCOCK BRIDGE (Existing scenario 100yr no Climate Change)

### User-defined

Invert Elev (m) = 16.0500  
Slope (%) = 0.2000  
N-Value = 0.030

### Highlighted

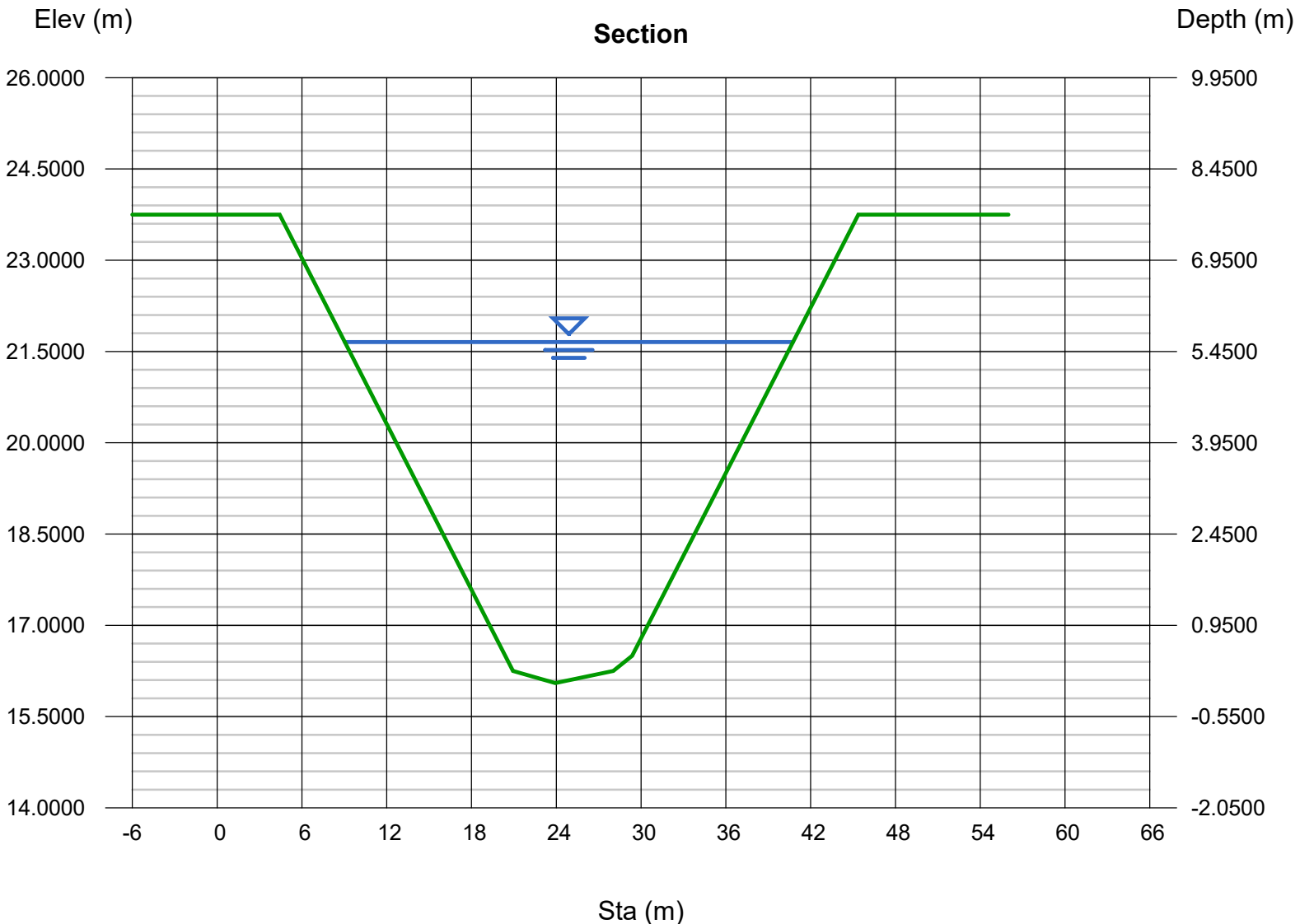
Depth (m) = 5.6054  
Q (cms) = 347  
Area (sqm) = 107.6504  
Velocity (m/s) = 3.2187  
Wetted Perim (m) = 34.0348  
Crit Depth, Yc (m) = 4.2215  
Top Width (m) = 31.7164  
EGL (m) = 6.1338

### Calculations

Compute by: Known Q  
Known Q (cms) = 346.5000

### (Sta, El, n)-(Sta, El, n)...

(0.0000, 23.7500)-(4.4200, 23.7500, 0.030)-(20.9300, 16.2500, 0.030)-(23.9300, 16.0500, 0.030)-(28.0300, 16.2500, 0.030)-(29.3700, 16.5000, 0.030)-(45.3700, 23.7500, 0.030)-(50.0000, 23.7500, 0.030)



# Channel Report

## WOODCOCK BRIDGE (Existing scenario 100yr + Climate Change)

### User-defined

Invert Elev (m) = 16.0500  
Slope (%) = 0.2000  
N-Value = 0.030

### Highlighted

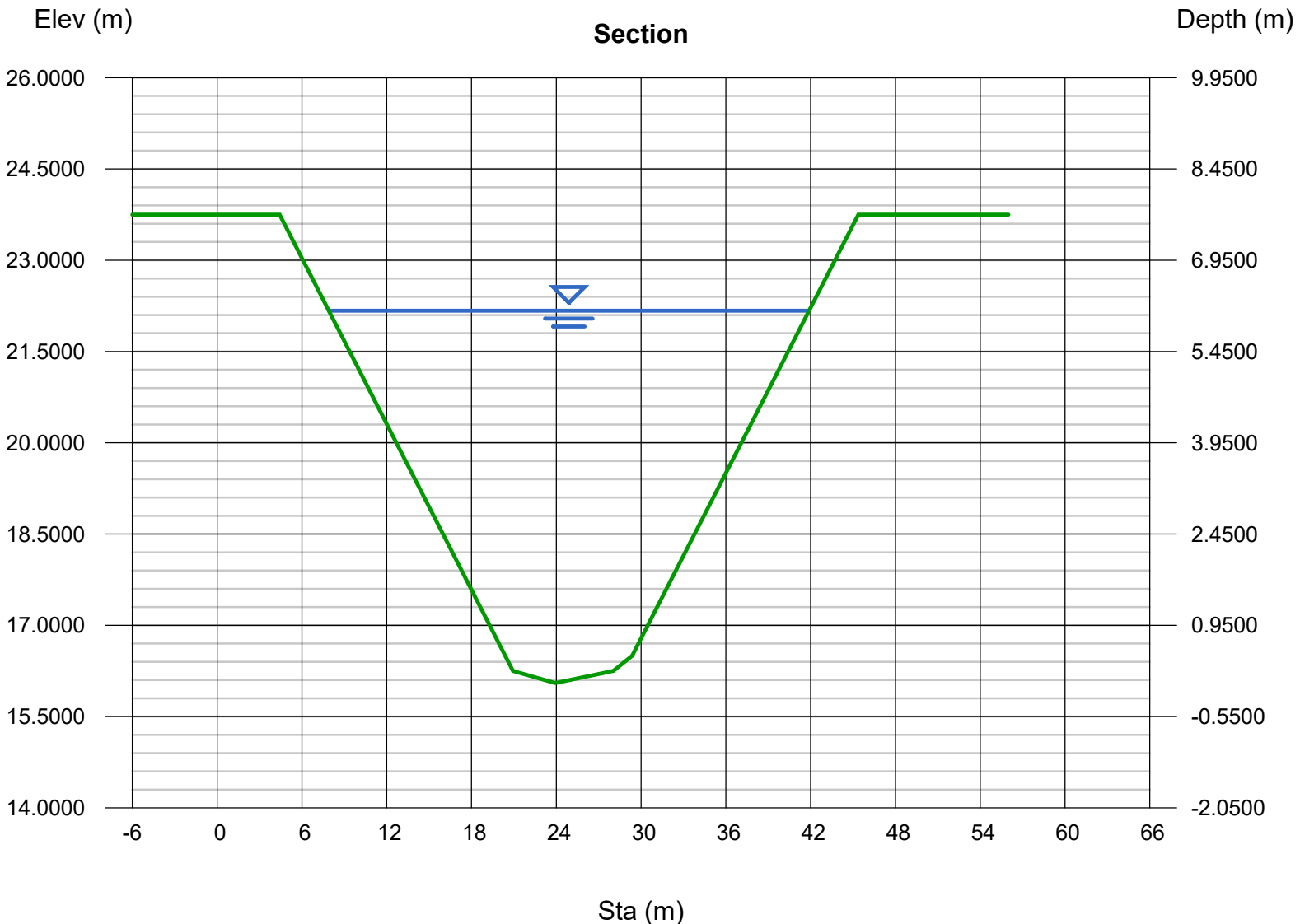
Depth (m) = 6.1205  
Q (cms) = 422  
Area (sqm) = 124.5731  
Velocity (m/s) = 3.3851  
Wetted Perim (m) = 36.5283  
Crit Depth, Yc (m) = 4.6483  
Top Width (m) = 33.9872  
EGL (m) = 6.7050

### Calculations

Compute by: Known Q  
Known Q (cms) = 421.7000

### (Sta, El, n)-(Sta, El, n)...

(0.0000, 23.7500)-(4.4200, 23.7500, 0.030)-(20.9300, 16.2500, 0.030)-(23.9300, 16.0500, 0.030)-(28.0300, 16.2500, 0.030)-(29.3700, 16.5000, 0.030)-(45.3700, 23.7500, 0.030)-(50.0000, 23.7500, 0.030)



# Channel Report

## WOODCOCK BRIDGE (Proposed scenario 100yr + Climate Change)

### User-defined

Invert Elev (m) = 16.0500  
Slope (%) = 0.2000  
N-Value = 0.030

### Highlighted

Depth (m) = 6.1144  
Q (cms) = 421  
Area (sqm) = 124.3660  
Velocity (m/s) = 3.3819  
Wetted Perim (m) = 36.4988  
Crit Depth, Yc (m) = 4.6422  
Top Width (m) = 33.9603  
EGL (m) = 6.6978

### Calculations

Compute by: Known Q  
Known Q (cms) = 420.6000

### (Sta, El, n)-(Sta, El, n)...

(0.0000, 23.7500)-(4.4200, 23.7500, 0.030)-(20.9300, 16.2500, 0.030)-(23.9300, 16.0500, 0.030)-(28.0300, 16.2500, 0.030)-(29.3700, 16.5000, 0.030)-(45.3700, 23.7500, 0.030)-(50.0000, 23.7500, 0.030)

