APPENDIX D - STORMWATER REPORT

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Stormwater Report for Resource Consent Application - Stage 1

State Highway 16: Huapai to Waimauku Safety Improvements

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1 INTRODUCTION

The State Highway 16 (SH16) corridor between Brigham Creek to Waimauku was identified within the *Safer Journeys - Delivering State Highway Safer Roads and Roadsides Programme Business Case (PBC) March 2014* as a rural state highway to be investigated further based on criteria within the High Risk Rural Roads Guide (HRRRG).

The PBC is based on a strategy of reducing the number of New Zealanders that are killed or seriously injured on our roads annually, minimising the social harm and economic impact of road crashes, by delivering against the NZ Transport Agency's (NZTA) commitments to safer roads and roadsides. Benefits have been identified within the strategic and programme business cases, to improve our roads and roadsides, and lead to a reduction of deaths and serious injuries.

The potential benefits of successful intervention have been identified as the following:

- **Benefit 1**: Reduction in Death and Serious Injuries (DSI) casualties on the SH16 Brigham Creek to Waimauku corridor (65%).
- Benefit 2: Maintain or improve travel time between Kumeu and Brigham Creek Road (35%).

1.1 Purpose and Scope

This report has been prepared on behalf of the NZTA to accompany an application for resource consent. This report summaries the stormwater related features of the safety improvement works and highlights the measures proposed to mitigate stormwater effects as guided by the Auckland Unitary Plan.

This report documents the Stage 1 (Section E) safety improvements treatments along SH16 between Huapai and Waimauku and the stormwater related features of the design to accommodate the works. The safety improvement treatments considered to address the safety issues identified in the Business Case for Section E are:

- Posted limit change (100 km/h and 70km/h) east of Wintour Road, Waimauku;
- New median and shoulder safety barrier, with safe turnaround facilities specific locations;
- Widened shoulders where required to accommodate new barriers; and
- Improved road vertical and horizontal geometry to enhance visibility.

1.2 Corridor Overview

The SH16: Brigham Creek to Waimauku corridor extends from the end of the Auckland North-Western Motorway at the intersection (roundabout) of SH16, Brigham Creek Road and Fred Taylor Drive through to the posted speed limit change (to 70 km/h) east of Waimauku.

The corridor is divided into five sections:

- Section A: Brigham Creek roundabout through to Coatesville-Riverhead Highway intersection:
- Section B: Coatesville-Riverhead Highway intersection;

- Section C: Coatesville-Riverhead Highway intersection through to Taupaki Road / Old North Rd intersection;
- Section D: Taupaki Road / Old North Road intersection through to the posted speed limit change (80km/h and 60km/h) east of Old Railway Road intersection, Kumeu; and
- Section E: From Station Road intersection, Huapai to the posted limit change (100 km/h and 70km/h) east of Wintour Road, Waimauku.

This assessment relates only to Section E.



Figure 1: The State Highway 16 Safety Improvements - section locality map

1.3 Existing Condition

Stage 1 of the project traverses the Kumeu Huapai catchment which drains to Kumeu River and into the Kaipara Harbour. The total catchment area upstream of Stage 1 is approximately 89km².

The Stage 1 section is underlain by Tauranga group Holocene soils (alluvium/colluvium, estuarine, lacustrine, fan and swamp deposits) and late Pleistocene soils (alluvium/colluvium and fan deposits). This soil type is free draining, however, not as porous as basalt or high gravel content soils. A curve number of 74 has been adopted for pervious portions of this catchment to reflect this soil type in accordance with TP108.

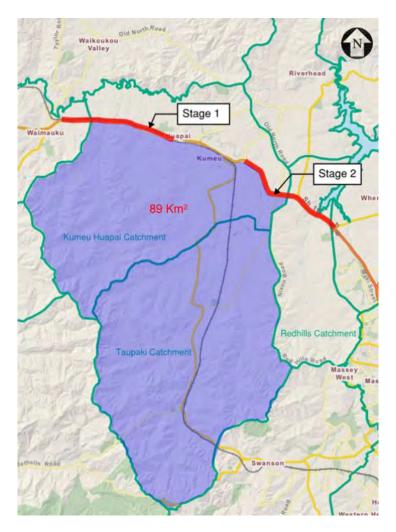


Figure 2: Catchments Upstream of Stage 1

Under the current Unitary Plan, the upper catchments that are intercepted by the Stage 1 portion of the project are generally zoned for Future Urban and Rural. Only a small portion of the Kumeu Huapai catchment is zoned for Residential - Single House Zone for areas closer to the town ship. Refer to the Unitary Plan for details and definitions of the various zones.

Observations based on existing aerial photographs confirms that most of the area under the Future Urban and Rural within the catchments are currently mostly rural and undeveloped, and are assumed to continue to be so for the purpose of this project.

Based on visual assessments of the existing aerial photographs, the rural catchments are assumed to have 7% of impervious area for runoff calculations.

1.3.1 Flood Risk to Road and Others

It is identified through review of the existing Auckland Council GIS (AC GIS) maps that there are many locations along the road or in adjacent private properties that could be inundated with stormwater during a large rainfall event. Refer to Appendix D for a copy of the AC GIS flood map. As per AC GIS flood map, key risk areas identified are shown in Table 1.

Table 1: Existing key flood risk areas as per AC GIS

REFERENCE PROPERTY ADDRESS (OR LOT DESCRIPTION)	REFERENCE CHAINAGE	
529 State Highway 16 Huapai 0891	Ch 196750	
Lot 2 DP 168981, Tapu Road Huapai 0810	Cf 196750	
551 State Highway 16 Huapai 0891	Ch 197200 to 197300	
601 State Highway 16 Huapai 0891	Ch 197550	
623 State Highway 16 Huapai 0891	Ch 197760	
647/653 State Highway 16 Huapai 0891	Ch 100140	
665 State Highway 16 Huapai 0891	Ch 198140	
695 State Highway 16 Huapai 0891		
23 Foster Road Huapai 0891	Ch 198650 to 198700	
727 State Highway 16 Huapai 0883		
757 State Highway 16 Waimauku 0883	Ch 199190	
771 State Highway 16 Waimauku 0883		
779 State Highway 16 Waimauku 0883		
791 State Highway 16 Waimauku 0883	Ch 1003C0 to 1008F0	
805 State Highway 16 Waimauku 0883	Ch 199360 to 199850	
Lot 2 DP 311972, State Highway 16 Waimauku 0883		
815 State Highway 16 Waimauku 0883		
2 Cloverfields Drive Waimauku 0812	Ch 200080	

1.3.2 Cross Culverts

Existing culverts have been identified using a combination of survey, RAMM, GIS and aerial photography and the locations of these are shown on the plans (SR1003-01-VE-2101 to 2123) and sketch SR1003-01-VE-9004. Appendix C contains figures showing the catchment of each culvert.

Due to lack of detailed survey information of location and invert levels of these culverts, the existing hydraulic performance of each culvert is an estimate only.

A summary of these existing culverts and initial assessment of flow capacity is presented in Table E in Appendix E. Based on the assessments presented, most of the culverts identified are undersized and are not capable of passing through a Q10 peak flow without surcharging and overtopping the road.

These undersized existing culverts are considered to be a key factor to the flooding identified in Section 1.3.1 above.

1.3.3 Primary Conveyance and Treatment

Based on the RAMM data and MSL survey provided the existing drainage consists of piped systems and swales along the corridor. This directs stormwater runoff and discharges into nearby watercourses through outlet structures.

No existing proprietary treatment devices have been identified on the RAMM data.

1.4 Reference Documents

The following reference documents have been used as the basis for design development.

- Cunningham, A., Colibaba, A., Hellberg, B., Silyn Roberts, G., Symcock, R., Vigar, N and Woortman, W (2017) Stormwater management devices in the Auckland region. Auckland Council guideline document, GD2017/001
- Auckland Council, 2016. Auckland Unitary Plan Operative in Part 15 November 2016, Chapter E Auckland-wide.
- Auckland Regional Council, 1999. Guidelines for stormwater runoff modelling in the Auckland Region (TP108).

1.5 Source of Existing information

The following information are used as inputs for this phase of the design for the development of concept design.

- Auckland Council GIS data:
 - Stormwater assets extracted at Business Case phase in late 2016
 - LiDAR ground contour extracted in March 2018
- NZTA RAMM data, initially provided at Business Case phase in late 2016 and again in March 2018.

Localised detailed survey has been undertaken in areas where topographic information is missing or needs to be refined. However, due to traffic management, programme and accessibility reasons, there are a large number of existing stormwater structures that cannot be surveyed. This includes most of the cross-culvert pipes.

2 BASIS OF DESIGN

The basis of design for new stormwater drainage design as follows:

- The objective for stormwater treatment is to treat all the new impervious road surface from road widening and as much as possible for existing surfaces;
- The preferred approach is to use natural stormwater systems (such as wetlands and swales) to treat the runoff from the corridor prior to discharging it to the receiving environment (where practical);
- Land take requirements are to be minimised when designing stormwater conveyance and treatment systems. To aid with this, the runoff from the corridor may also be treated by proprietary treatment devices;
- Where land take opportunities are possible and assessed to be appropriate, the use of stormwater ponds, wetlands or wetland swales will be considered and implemented where practical;
- Where the existing pipe is deemed sufficient for serviceability and capacity, no further improvements have been considered. However, if the existing pipe is deemed no longer suitable for the proposed road geometry, replacement is proposed;
- Where existing drainage systems are to be adopted/connected to, it may not be practical or feasible to upgrade/replace the existing infrastructure to meet the current standards.

In these cases, the proposed design should minimise any changes to the existing level of service; and

• Construction of any permanent assets within KiwiRail land are to be avoided.

Table 2 below sets out a summary of quantitative stormwater management design criteria applicable to the project.

Table 2: Summary of Stormwater Design Criteria

ITEM		CRITERIA
Design event allowances		
	100-year ARI (1% AEP), 24-hour	245mm
Design event allowances Rainfall (incl. 2.1°C climate	10-year ARI (10% AEP), 24-hour	147mm
change increase) ¹	5-year ARI (20% AEP), 24-hour	107mm
	2-year ARI (50% AEP), 24-hour	83mm
Example 12	95 th percentile rainfall	35mm
Frequent raintail	90 th percentile rainfall	25mm
Stormwater quality rainfall ⁴	As per NZTA requirement (2 year, 1-hour, but not greater than 30mm)	28mm
Catchment characteristics		
200	Impervious areas	98
SCS curve number	Pervious areas	74
Road catchment	Existing and proposed	100% impervious
Upper catchment ³	Existing and proposed	Rural Zones: 7% impervious, 93% pervious Urban (Residential) Zones: 60% impervious, 40% pervious
Design criteria		
SMAF 1 - Retention	Impervious areas	Retention (volume reduction) of 5mm runoff depth for the impervious are for which hydrology mitigation is required
SMAF 1 - Detention	Impervious areas	Detention (temporary storage) and a drain down period of 24 hours for the difference between the pre-development and post-development runoff volumes from the 95 th percentile, 24-hour rainfall event minus the 5mm retention volume, over the impervious area for which hydrology mitigation is required
Quality ⁴	Road > 5000 vpd	Treatment of runoff as per the following guidelines, in order of precedence: NZTA treatment guideline GD01 guideline TP10 guideline
Conveyance	Primary	10% AEP

ITEM	CRITERIA			
	Secondary 1% AEP less primary capacity.			
	Carriage way	Inundation of road is kept to as per the existing condition		
Flood risk	Offsite properties ⁵	The diversion and discharge must not result in or increase the following: • flooding of other properties in rainfall events up to the 10 per cent annual exceedance probability (AEP); • inundation of buildings on other properties in events up to the 1 per cent annual exceedance probability (AEP).		

Notes:

- 1. Rainfall date extracted from HIRDS with allowance of 2.1°C climate change increase
- 2. Interpolated from Figure 13 and 14 of Auckland Council Technical Report 2013/035
- 3. Existing pervious area as defined from AC GIS and aerial photograph
- 4. Stormwater quality rainfall varies depending on the referenced guideline (other values listed below). 28mm is adopted for the project as NZTA design guideline is preferred and is greater than GD01, and not significantly different to TP10.
 - a. 29mm as per TP10 (2 year, 24-hour, divide by 3)
 - b. 25mm as per GD01 (90th percentile rainfall)
- 5. As per Section E8.6.1 (3) of the Unitary Plan.

2.1 Other Design Considerations

- New culverts, pipelines, manholes and other hydraulic structures beneath paved surfaces shall have a design and hydraulic performance life, and durability performance of not less than 100 years, considering the site ground conditions, potential settlement, and depth of cover, type of bedding and backfill and method of installation;
- Access to pipelines, hydraulic structures and treatment devices must be provided to enable safe and convenient inspection and maintenance;
- A minimum pipe diameter of 300mm is adopted across the design for reduced risk of blockage, and minimum of 375mm pipe crossing under vehicle access is required as per NZTA standards;
- Where piped networks are proposed, new inlets are spaced such that gutter flow will not encroach into the nearest traffic lane in a 10% AEP event;
- One lane must remain open for traffic each way during a 1% AEP storm event, where practical subject to existing flood conditions along SH16;
- Existing culverts are to be extended to suit the new road geometry as a minimum.
 Additional culverts will be provided should the extension of culvert result in an adverse effect of flooding to others on the upstream side;
- All treatment swales are to be planted to minimise footprint;
- The success of implementation new wetlands and ponds is largely dependent on the catchment areas draining to each device is larger than 4Ha;
- All discharges to sensitive receiving environments (waterways and wetlands) shall have energy dissipaters and appropriately designed permanent erosion control; and

• Clean stormwater runoff from permeable surfaces are diverted away from treatment devices where practicable.

2.2 Proposed Design

The proposed design will result in an increased impervious road area due to the widened road geometry works. The existing road is typically 11m along the corridor, whereas the new works proposed typically results in a total carriageway width range from 11.5m to 13.5m, and up to 16m in some localised sections.

Based on the existing road impervious area as defined from MLS survey and aerial photographs, the increase in impervious area of the road within Stage 1 (Section E) is 22% as per shown in Table 3.

Table 3: Existing and Proposed Impervious Area

	EXISTING IMPERVIOUS AREA (M²)	PROPOSED IMPERVIOUS AREA (M²)	% INCREASE
Stage 1 (Section E)	41100	52534	22%

The change in the overall imperviousness in catchment is tabulated in Table F in Appendix F.

2.2.1 Flood Risk

The effects of the proposed widening works are assessed in four different areas as below. It should be noted that the assessment of the effects on the various areas of flood risks are limited to desktop studies based on information gathered from AC GIS and existing RAMM data. Assumptions have been made where there are gaps in the existing information, such as stream cross-section profiles, pipe sizes and invert levels.

2.2.1.1 Flood Risk to Road

As discussed previously, most of the existing culverts are found to be under capacity under existing condition and will overtop the road under the 10% AEP design rainfall event. As there is only a minor increase in imperviousness and the road vertical geometry is not going to be changed significantly, the effects of the proposed widening works to the existing flood risk on the road are considered to be minor.

The new stormwater conveyance pipe network is proposed to capture road surface runoff, which should have a beneficial effect to the existing flooding situation on the road, as the proposed pipe network will able to provide some storage volume within the pipes.

Works to improve any existing flooding conditions to the road are currently outside the scope of the project.

2.2.1.2 Flood Risk to Others - Downstream

The cumulative effect of the proposed works for Stage 1 on downstream properties and buildings have been assessed at a discharge point on the Cane Road and Waikoukou Valley Road within the Kaipara River (refer to Figure 3).

The existing 1% AEP peak discharge to this point from a total catchment of 89.5km² is estimated to be 329m³/s. The total cumulative increase in 1% AEP peak discharge from the widening works is less than 1 m³/s. Based on the existing cross section at this discharge point, an increase of 1 m³/s of peak flow will approximately increase the water level in the river at this point by about 5 mm.

The effects are also reviewed at the two streams that are crossed by Stage 1 works and the results are summarised in the table below. As shown Table 4, the effects of the 1% AEP peak discharge in increase on the water level at each stream is less than 1mm. The additional discharge from the proposed works presents no increase to flood levels at these streams and the increase of flood risk and lateral loading to the bridge structure to the existing bridges are small.

Q100 Flow (m^3/s) Q100 Water Level (m) Stream Name Bridge Name | Catchment area (ha) Pre Change **Post** Change **Post** Pre Kumeu #2 380.9 47.313 47.317 0.004 20.986 0.000 Coopers Creek 20.986 115.683 115.688 Ahukuramu Stream Berrys 1295.3 0.005 16.861 16.861 0.000

Table 4: Estimated Water Level at Bridges

Therefore, the increase in flood extent and depth to downstream buildings and properties due to the minor increase in peak flow are expected to be minimal at these key crossing points.



Figure 3: Discharge point for assessment of Stage 1 effects flood risk on downstream property (Modified, source: Auckland GIS viewer, May 2018)

2.2.1.3 Flood Risk to Others - Upstream at Culverts

Extension of existing culverts will influence the head water level (HWL) at each culvert and could therefore have an increase in potential flood risk effects to immediate upstream properties and buildings.

As discussed previously, most of the existing culverts are found to be under capacity and will overtop the road under the 10% AEP design rainfall event. Given that the road is wide, small increases in flow from the road widening will have an insignificant effect on upstream flood levels at these overflow locations. Only three culverts have been identified to have sufficient capacity for the 10% AEP design rainfall event. For these culvert locations a change of HWL has been calculated based on the additional of length of culvert required for the new road width and this is shown in Table 7 below.

Catchment Name	Name (Ch)	Change in length (m)	Existing 10% AEP HWL (RL m)	10% AEP HWL after extension (RL m)	Change in HWL (m)
D	197550	0	23.024	23.024	0
Н	198220	0	30.540	30.540	0
N	100580	5.00	12 550	12 562	0.005

Table 5: Change in HWL at culverts with capacity for 10% AEP rainfall

The increases in HWL are all very small and therefore the effects of the proposed extension work on the nearby properties upstream of these culverts are insignificant.

2.2.1.4 Afflux at Bridges

The bridge widening works proposed for Berry Creek Bridge and Kumeu No.2 Bridge is limited to the extension of the bridge deck. The cross-section area defined by the stream channel and bridge structure at each stream will not be changed as the additional superstructure proposed is a replication of the existing structure.

On this basis, the effects of afflux at both streams are negligible.

2.2.2 Cross Drainage

The capacity assessment results have indicated that most of the existing culverts are potentially undersized when compared with current design standards. Increasing these culverts to accommodate a larger flow is not within the scope of works for this project.

Existing culverts that pass under SH16 will be extended to suit the new road geometry, as shown on the plans. The effect of culvert lengthening is discussed in Section 2.2.1.3 (the proposed design should be no worse than the existing level of service).

2.2.3 Stormwater Discharge and Diversion

Refer to drawings SR1003-01-CE-2101 to 2123 and Table 6 for the proposed stormwater drainage layout for primary and secondary conveyance systems. A set of detailed catchment plans are also included in Appendix D showing all the discharge points (DP)

New manholes and pipes will be typically located behind wire rope barriers in the road verge area. Catchpit spacing are placed to meet the requirement of keeping the 10% AEP stormwater runoff within shoulder.

Secondary flow from the carriageway will be typically contained within the road carriageway between kerbs or within roadside conveyance channels and/or swales.

Clean water diversion drains on either side of the road corridor have been designed to replicate the existing road side open channels. This will divert clean runoff from upstream external catchment, so no proposed treatment devices will be overloaded with clean water runoff or be unnecessarily oversized. No redirecting of catchments is proposed.

Table 6: Proposed Stormwater Discharge Points and Conveyance Systems

DISCHARGE POINT	DISCHARGE POSITION CHAINAGE (M)		RECEIVING ENVIRONMENT	CAPTURE AND CONVEYANCE APPROACH	
		From	То		
DP 1	WB, South	196600 (Trigg Road)	169760	Artificial watercourse	Kerb and channel with pits and pipes
DP 2	EB, North	169760	196920	Artificial watercourse	Kerb and channel with pits and pipes
DP 3	WB, South	196920	197390	River or Stream (Coopers Creek)	Kerb and channel with pits and pipes and discharge into retention swale before final discharge
DP 4	EB, North	197200	197390	River or Stream (Coopers Creek)	Kerb and channel with pits and pipes
DP 5	EB, North	197390	197870	River or Stream (Coopers Creek)	Kerb and channel with pits and pipes
DP 6	EB, North	197870	198050	Artificial watercourse	Kerb and channel with pits and pipes
DP 7	EB, North	198050	198140	Artificial watercourse	Kerb and channel with pits and pipes
DP 8	WB, South	198140	198280	Artificial watercourse	Existing roadside swale
DP 9	WB, South	198280	198660	River or Stream (Ahukuramu Stream)	Existing roadside swale
DP 10	WB, South	198660	199050	River or Stream (Ahukuramu Stream)	Existing roadside swale
DP 11	EB, North	198600	198660	River or Stream (Ahukuramu Stream)	Kerb and channel with pits and pipes
DP 12A	WB, South	199050	199200	Artificial watercourse	Existing roadside swale
DP 12B	EB, North	199050	199200	Artificial watercourse	Kerb and channel with pits and pipes
DP 12	EB, North	199200	199580 (Joyce Adams Place)	River or Stream	Kerb and channel with pits and pipes
DP 13	EB, North	199580 (Joyce Adams Place)	199780	River or Stream	Kerb and channel with pits and pipes
DP 14	WB, South	199780	200020 (Wintour Road)	River or Stream	Kerb and channel with pits and pipes and discharge into swale before final discharge
DP 15	WB, South	200020 (Wintour Road)	200130	River or Stream	Kerb and channel with pits and pipes

As shown in Table 6, there are multiple discharge points directly discharging runoff into stream receiving environments. Detention and retention volumes are required to be provided

as per the AUP-OP, Rule E8.6.3. for any controlled activities when discharge to natural streams. This rule has been interpreted to require providing detention and retention volumes, to which is the requirement as the SMAF 1 overlay. The discharge points (DP) are tabulated in Table 7 below summarised with commentaries on the provision of detention and retention below.

Table 7: Discharge points within stream environments

DISCHARGE POINT	COMMENTS
DP 3	Part of the overall SMAF 1 area as detailed in Section 2.2.5.
DP 4	Part of the overall SMAF 1 area as detailed in Section 2.2.5.
DP 5	Outside of the overall SMAF 1 area as detailed in Section 2.2.5, however required retention volume are provided within the proposed retention swale design for DP 3 and DP 4.
DP 9	Heavily constrained by existing locations of utilities and power poles and land availability for provisions of any retention and/or detention devices
DP 10	Rip-rap swales are provided for conveyance and energy dissipation. Retention and/or detention are provided as per detail below.
DP 11	Heavily constrained by existing adjacent rail embankment and land availability for provisions of any retention and/or detention devices
DP 12	Heavily constrained by existing adjacent rail embankment and land availability for provisions of any retention and/or detention devices
DP 13	Heavily constrained by existing adjacent rail embankment and land availability for provisions of any retention and/or detention devices
DP 14	Discharges into an existing stormwater pond where the identified stream passes through. Provision of any retention and/or detention devices is likely to require works within this existing pond which is unlikely to provide any betterment or improvement on the existing scenario.
DP 15	Discharges into an existing stormwater pond where the identified stream passes through. Provision of any retention and/or detention devices is likely to require works within this existing pond which is unlikely to provide any betterment or improvement on the existing scenario.

Requirements of SMAF retention and detention is dealt with under Section 2.2.5. The retention and detention required under E8 of the Unitary Plan can only be provided at DP 10, for which the required hydraulic mitigation volumes are summarised in the Table 8 below. Other DPs that are not included in Table 8 will not have any detention or retention provided as it has proven to be not practical to do so as per commentaries provided in Table 7 above.

Table 8: Detention and retention volume required

DISCHARGE TOTAL IMPERVIOUS POINT AREA (M²)		ADDITIONAL IMPERVIOUS AREA (M²) ^(NOTE 1)	RETENTION VOLUME (M ³)	DETENTION VOLUME (M ³)	TOTAL DETENTION VOLUME (M ³) (NOTE 2)
DP 10 8134		1789	40.7	0.4	41.1
2. Total of	•	are based the average ad lculated as per SMAF1 re sons.	the state of the s		

Under the Unitary Plan, Table E10.6.3.1, the retention volume is calculated using the impervious area of the whole site whereas detention volume is only for the difference of imperviousness between the post- and pre-development state.

The proposed road widening works means that the retention volume required also meets the requirement of provision of detention volumes (i.e. the required total detention volume is similar to the required retention volume). Therefore no specific detention volume is required to be provided.

Hydrology mitigation volumes are provided using retention storage volume under the swale (50m long) with check dams, the results are summarised in Table 9. This is the same approach adopted to meet the requirement of the SMAF overlay, per detailed in Section 2.2.5 of the report.

Table 9: Retention swales summary

DETENTION AND RETENTION VOLUMES	
DP 10	
Zone 1 (Detention)	
Ponding depths (including mulch layer) (mm)	0.200
Check dam spacing (m)	16
Total ponded water volume (m³)	12.5
Zone 2 (Retention)	
Storage layer depth (m)	1
Storage volume provided (m³)	30

As shown in Table 9, a total of 42.5m³ of hydrology mitigation volume is provide for DP 10, which is more than the total required.

2.2.4 Stormwater Treatment

All stormwater runoff generated from any new high use road surfaces will be treated before discharge. Not all runoff from existing road surface can be captured and treated as there sections of the westbound carriageway edge are left unchanged. Approximately 84%

(44042m² of treated area out of 52534m² total impervious area) of the total impervious area are treated.

Road surface runoff are treated through a combination of natural green infrastructure and proprietary treatment devices along the corridor (refer drawings SR1003-01-CE-2101 to 2123 and summarised in Table 10).

Wetlands were considered initially at downstream discharge points where land is not an issue or opportunity for land take is present. However, initial assessment demonstrated that it is difficult to meet the required catchment area (>4 Ha) from the road surface catchments to make the wetland ecologically viable.

Proprietary treatment devices (such as Stormwater360 Stormfilters) are proposed as the road reserve is heavily constrained by land availability, treatment swales with varying base width are used for treatment purposes elsewhere.

Table 10: Proposed Stormwater Treatment Devices

Location (Ch.)			DISCHARGE POINT (DP)	CATCHMENT AREA (M ²)		CHANGE IN IMPERVIOUS AREA (%)	TREATMENT DEVICES	COMMENTS AND NOTES
From	То	WB or EB		Impervious	Pervious			
196600 (Trigg Road)	196700	WB	DP 1 (Ultimately)	588	0	0 %	None.	Stormwater network on WE shoulder caters for the existing runoff from upstream residential land from exiting.
196700	196730	WB (Turnaround)	DP 1	736	0	489 %	Proprietary treatment device (Stormwater360 Stormfilters, 2 No.)	See note 1
196730	196920	WB	DP 2 (Ultimately)	1043	0	0 %	None	Edge of seal remains unchanged.
196600 (Trigg Road)	196920	ЕВ	DP 2	2705	0	45 %	Proprietary treatment device (Stormwater360 Stormfilters, 6 No.)	See note 1
196920	196950	Both	DP 2	534	0	26 %	None	Edge of seal remains unchanged.
196950	197340	Both	DP 3	4465	22184	27 %	Planted Retention Swale	See section 2.2.5 for detail
197340	197400 (Berrys Creek)	Both	DP 4	1510	0	33 %	Proprietary treatment device (Stormwater360	See note 1

Location (Ch.)		DISCHARGE POINT (DP)	CATCHMENT AREA (M²)		CHANGE IN IMPERVIOUS AREA (%)	TREATMENT DEVICES	COMMENTS AND NOTES	
							Stormfilters, 3 No.)	
197400 (Berrys Creek)	197880	Both	DP 5	7200	0	33 %	Proprietary treatment device (Stormwater360 Stormfilters, 15 No.)	See note 1
197880	198050	Both	DP 6	2570	0	34 %	Proprietary treatment device (Stormwater360 Stormfilters, 6 No.)	See note 1
198050	198170	Both	DP 7	1900	0	14%	Proprietary treatment device (Stormwater360 Stormfilters, 4 No.)	See note 1
198170	198280	Both	DP 8	1235	0	15 %	None	Edge of seal remains unchanged.
198280	198420	WB	DP 9 (Ultimately)	804	0	0 %	None	Edge of seal remains unchanged.
198420	198520	WB	DP 9 (Ultimately)	500	2600	0 %	Proprietary treatment device (Stormwater360 Stormfilters, 3 No.)	See note 1
198520	198660	WB	DP 9	657	0	0 %	None	Edge of seal remains unchanged.
198280	198630	ЕВ	DP 11	3143	921	60 %	Proprietary treatment device (Stormwater360 Stormfilters, 7 No.)	See note 1
198630	199050	ЕВ	DP 10	5570	0	91 %	Proprietary treatment device (Stormwater360 Stormfilters, 11 No.)	Includes 1187m² of Froster Road catchment. See section 2.2.3 for retention and detention provision.
198660	199050	WB	DP 10	2250	0	0 %	None	Includes 470m² of Froster Road catchment.

Location (Ch.)		DISCHARGE POINT (DP)	CATCHMENT AREA (M²)		CHANGE IN IMPERVIOUS AREA (%)	TREATMENT DEVICES	COMMENTS AND NOTES	
								Edge of seal remains unchanged.
199050	199200	ЕВ	DP 12B	800	0	19%	Proprietary treatment device (Stormwater360 Stormfilters, 2 No.)	See note 1
199050	199200	WB	DP 12A	870	0	0 %	None	Edge of seal remains unchanged.
199200	199600	Both	DP 12	5870	0	20 %	Proprietary treatment device (Stormwater360 Stormfilters, 12 No.)	See note 1
199600	199780	Both	DP 13	2260	0	24%	Proprietary treatment device (Stormwater360 Stormfilters, 5 No.)	See note 1
199780	200020	Both	DP 14	3480	13260	17%	Planted Treatment Swale	See note 2
200020	200020	Both	DP 15	289	0	0 %	None	Includes part of Wintour Road. Edge of seal remains unchanged.
200020	200130	Both	DP 15	1567	0	0 %	Proprietary treatment device (Stormwater360 Stormfilters, 4 No.)	See note 1 Existing stormwater pond remains.

Notes:

- Proprietary products are to be confirmed and sized in accordance's with manufacture's guide lines and requirements.
 Stormwater360 Stormfilters are used for preliminary design purposes only and sizing of these are based on recommendation provided by the manufactures' web-based calculator, using a water quality rainfall intensity of 10mm/hr as per GD01.
- 2. Planted treatment swales are designed to NZTA's guideline to provide a residence time of 9 minutes or more.

2.2.5 SMAF Requirements- Retention and Detention

The SMAF 1 overlay mainly applies to the Kumeu Huapai township catchment and is applicable to a portion of Stage 1 of the project, for approximately 780m of SH16 measured west from Trigg Road. The required retention and detention volumes are given in Table 11 below.

Table 11: SMAF 1 volumes

TOTAL SMAF IMPERVIOUS AREA (M²)	RETENTION (M³)	VOLUME	DETENTION (M³)	VOLUME	TOTAL DETENTION VOLUME (M³)
11368	57		6		63

To cater for the required SMAF retention and detention volumes, is it proposed that the road runoff is collected by a pipe network and discharged into a retention swale. Constraints are imposed by the geographical nature of the alignment which is limited by the railway corridor to the north and private properties to the south. A typical section of retention swale is shown in Figure 4 below.

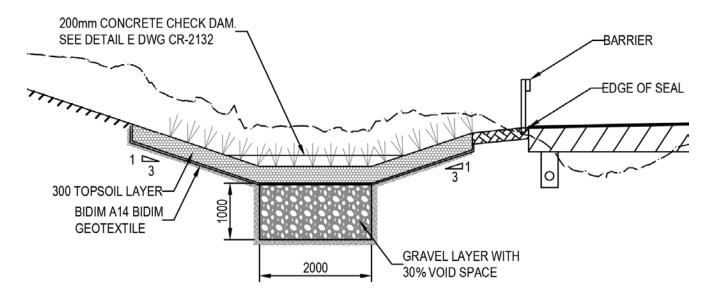


Figure 4: Typical section of retention swale

A retention swale is proposed on the westbound side of the road from approximately Ch. 197200 to 197400. Check dams are designed into this swale to encourage stormwater infiltration into the storage layer and provide detention volume within the ponded water behind check dams.

All of the required retention volume is provided by this retention swale. The proposed swale design parameters are presented in Table 12 and Figure 5.

Table 12: Retention swale result summary

Ітем					
Swale geometry and water quality (as per NZTA guideline)					
Swale base width (m)	2				
Side slope (1:x)	3				
Swale length (m)	180				
Swale longitudinal slope (%)	1.4				
WQ depth (m)	0.203				
Residence time (min)	23				
Detention and retention volume					
Zone 1 (Detention)					
Ponding depths (including mulch layer) (mm)	0.200				
Check dam spacing (m)	30				
Total ponded water volume (m³)	20				
Zone 2 (Retention)					
Storage layer depth (m)	1				
Storage volume provided (m³)	102				

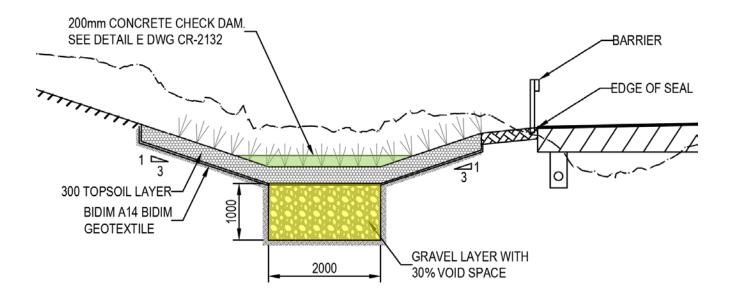


Figure 5: Proposed retention swale section

3 RESOURCE CONSENT APPLICATION SUMMARY

The State Highway 16 safety improvements project is being undertaken by the New Zealand Government to save lives on roads. The road widening works will create a small increase in runoff volume and increase pollution generating areas that flow to natural waterways in the Auckland Region. The methods of treatment, retention and detention proposed have been included within a space constrained corridor to mitigate the effects of the works before discharging runoff flows to the receiving stream.

The measures proposed in this report and as shown on the drawings in Appendix A shall adequately meet the Auckland Unitary Plan Requirements in mitigating effects arising from the works.

APPENDIX A DRAWINGS

ALL MANHOLE SHALL BE 1050mm DIAMETER UNLESS NOTED OTHERWISE

2. ALL PIPES SHALL BE RCRRJ UNLESS NOTED OTHERWISE WITH MINIMUM OF:

CLASS 6, 375mm OR LARGER FOR ALL VEHICLE CROSSING CULVERTS

- CLASS 4, 300mm OR LARGER FOR ALL OTHER STORMWATER PIPES
- 3. ALL FRAME AND LID SYSTEMS SHALL BE CLASS D LOAD RATING FOR TRAFFICABLE AREA
- 4. ALL CATCHPIT GRATES SHALL BE STANDARD 650x450 CYCLE FRIENDLY TYPE CATCHPIT GRATE & FRAME, FITTED WITH APPROVED CONCRETE BACK ENTRY LINTEL UNITS
- 5. ALL VEHICLE CROSSING CULVERT HEADWALLS SHALL BE EXTENDED BEYOND THE BARRIER END TERMINALS. REFER TO CE-1908 AND CE-1909 FOR DETAIL. FINAL INSTALLATION LOCATION SHALL BE CONFIRMED WITH THE ENGINEER.
- 6.1. CLASS 1 (D50=100mm) FOR RIP-RAP APRONS AT ALL DOWNSTREAM CULVERT HEADWALLS, UNLESS NOTED OTHERWISE.
- 6.2. CLASS 2 (D50=200mm) FOR RIP-RAP CHUTES, WITH GEOTEXTILE FABRIC AS PER SPECIFICATIONS, UNLESS NOTED OTHERWISE.
- ALL OPEN CHANNELS (SWALES AND DIVERSION DRAINS) SHALL BE PLANTED AND HAVE A SIDE SLOPE OF 3H:1V UNLESS NOTED OTHERWISE. REFER TO LANDSCAPE DRAWINGS FOR PLANTING DETAIL.
- 8. ALL CULVERT HEADWALLS SHALL BE PRECAST UNITS SUITABLE FOR THE DIAMETER OF PIPE SHOWN ON THE PLANS, UNLESS OTHER WISE NOTED.
- 8.1. ALL EXISTING HEADWALL INVERTS AND CROSS CULVERT SIZES AND LEVELS SHALL BE CONFIRMED AND VERIFIED ON SITE PRIOR
- 9. THE CONTRACTOR SHALL SETOUT ALL STORMWATER DRAINAGE ELEMENTS USING DIGITAL 3D SETOUT INFORMATION PROVIDED. SETOUT OF ALL STRUCTURES SHALL BE COORDINATED WITH ALL GEOMETRIC STRINGS AND ROADSIDE CONSTRUCTION ELEMENTS PRIOR TO UNDERTAKING ANY PERMANENT WORKS.
- 10. THE CONTRACTOR SHALL VERIFY ALL LEVELS AND OPERATIONAL STATUES OF EXISTING STORMWATER STRUCTURES PRIOR TO UNDERTAKING ANY PERMANENT WORKS. ANY VARIATIONS TO WHAT WERE SHOWN ON THE DRAWINGS SHALL BE NOTIFIED TO THE ENGINEER FOR REVIEW PRIOR TO PROCEEDING.
- 11. THE STORMFILTERS AND ASSOCIATED VAULTS AND MANHOLES ARE A PERFORMANCE BASED DESIGN COMPONENT OF THE PROJECT. THE DETAILS SHOWN ON THE PLAN ARE PRELIMINARY ONLY AND ARE BASED ON TREATMENT AREAS AS PER TABLE 1 BELOW. THE DETAILS SHOWN ON THE PLANS AND TABLE BELOW ARE BASED ON DEVICES WITH BUILT-IN PEAK FLOW DIVERSION STRUCTURES, WITH ZPG FILTER MEDIA IN 69cm TALL CARTRIDGES.
- 11.1. WHERE ASTERISKS (*) ARE MARKED NEXT TO THE QUANTITY OF CARTRIDGES, IT INDICATES THAT THE LOW PROFILE (30cm TALL) CARTRIDGES ARE REQUIRED.
- THE CONTRACTOR SHALL CONFIRM FINAL PRODUCT SELECTION AND DETAILS WITH THE MANUFACTURER AND INSTALL THEM AS PER MANUFACTURERS' RECOMMENDATIONS. SUBSTITUTION FOR ALTERNATIVE PRODUCTS ARE SUBJECT TO THE APPROVAL OF
- 12. THE CONTRACTOR SHALL CONSTRUCT ALL PERMANENT ASSETS AND STRUCTURES WITHIN PERMANENT LAND TAKE AREAS AS SHOWN ON THE LAND REQUIREMENT PLANS.

TABLE 1: TREAMENT DEVICE SIZING (REFER TO NOTE 11)

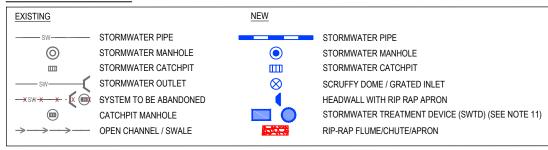
Treatment Devices	Catchment area (m2)		Treatment Devices			
	Impervious	Pervious				
SWTD AA	736	0	Proprietary treatment device (Stormwater360 Stormfilters, 2 No.)			
SWTD A	2704	0	Proprietary trealment device (Stormwater360 Stormfilters, 6 No.)			
SWTD B	1510	0	Proprietary treatment device (Stormwater360 Stormfilters, 3 No.)			
SWIDC	7200	0	Proprietary treatment device (Stormwater360 Stormfilters, 15 No.)			
SWTD D	2570	0	Proprietary treatment device (Stormwater360 Stormfilters, * 6 No.)			
SWTDE	1900	0	Proprietary treatment device (Stormwater360 Stormfilters, 4 No.)			
SWTD EE	500	2600	Proprietary treatment device (Stormwater360 Stormfilters, 3 No.)			
SWTDF	3140	920	Proprietary treatment device (Stormwater360 Stormfilters, 7 No.)			
SWTD FF	5570	0	Proprietary treatment device (Stormwater360 Stormfilters, 11 No.)			
SWTDG	5870	0	Proprietary treatment device (Stormwater360 Stormfilters, * 12 No.)			
SWTD GG	800	0	Proprietary treatment device (Stormwater360 Stormfilters, *2 No.)			
SWTDH	2260	0	Proprietary treatment device (Stormwater360 Stormfilters, 5 No.)			
SWTDI	1567	0	Proprietary treatment device (Stormwater360 Stormfilters, 4 No.)			

GENERAL LEGEND

- PROPERTY BOUNDARY — — ROAD / RAIL DESIGNATION — 196700 CHAINAGE BRIDGE WIDENING O 016-0019-B/6.5 ROUTE POSITION (RP)

NEW MAJOR AND MINOR CONTOURS 17.00 MAJOR CONTOURS 17.00 MINOR CONTOURS

STORMWATER LEGEND





→ → → PLANTED TREATMENT SWALE ->-->-->--> RETENTION SWALE

--- CONVEYANCE AND DIVERSION CHANNEL

- 1. ALL SWALES AND OPEN CHANNELS SHALL BE CONSTRUCTED AS PER DIMENSIONED ON THE PLANS AND DETAILS.
- 2. THE CONTRACTOR SHALL ALLOW FOR LOCALISED EARTHWORK AND GRADING IN AREAS WITH UNDULATING TERRAINS SUCH THAT THE CHANNELS ARE FLOWING AS INTENDED BY THE PLANS.

NEW EXTENT OF KERBS AND CHANNELS

SM1 - SEMI MOUNTABLE KERB ONLY SM2 - SEMI MOUNTABLE KERB AND CHANNEL 600mm WIDE V-SHAPED DISH CHANNEL BRIDGE EDGE TREATMENT

- 1. THE FINAL EXTENT OF DIFFERENT KERB VARIATIONS SHALL BE CONFIRMED WITH THE ENGINEER ON SITE.
- 2. REFER TO CIVIL DRAWINGS CE-2135 FOR KERB DETAILS.
- 3. REFER STRUCTURAL DRAWINGS FOR BRIDGE EDGE TREATMENTS AND

ORIGINAL DRAWING IN COLOUR

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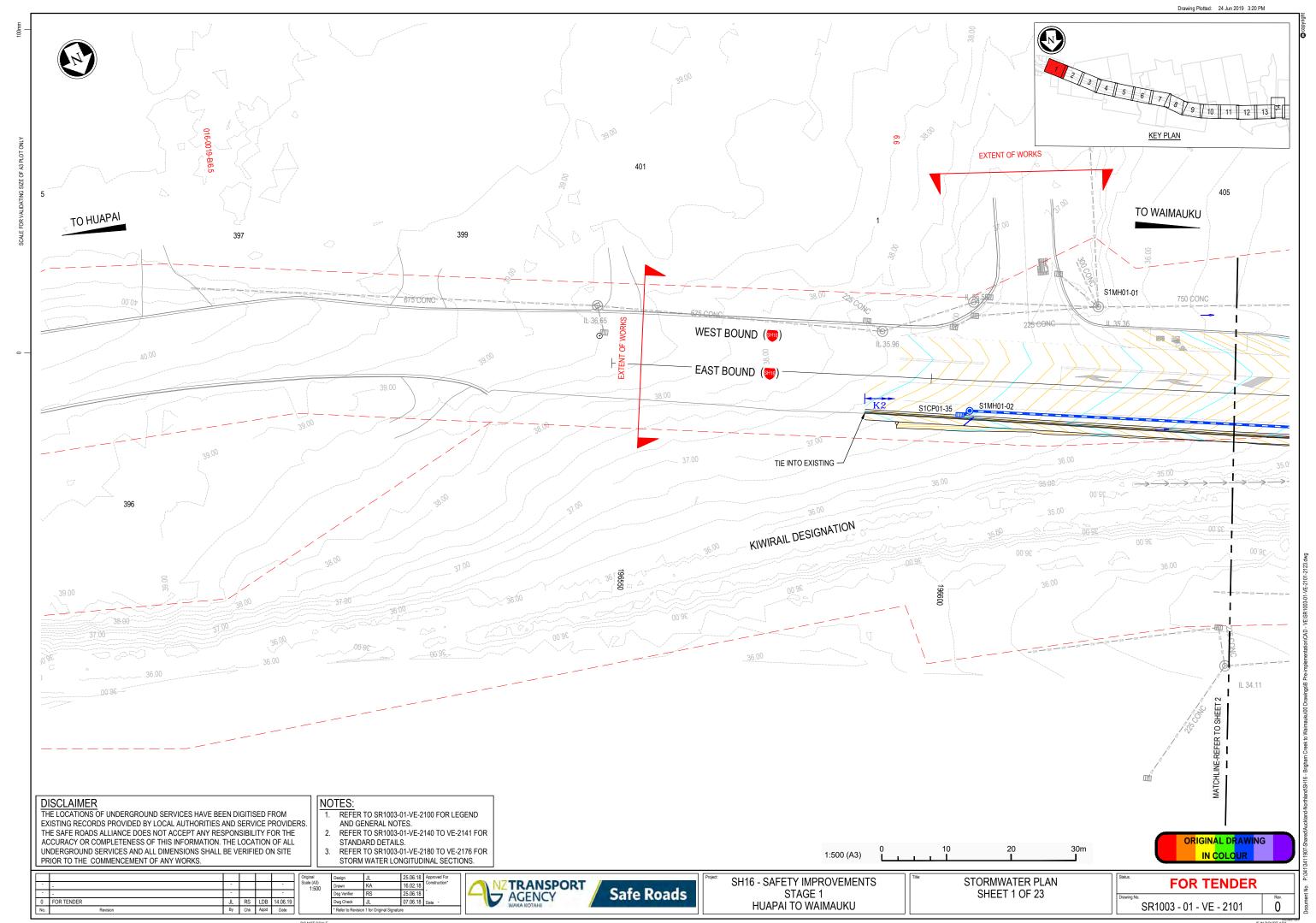


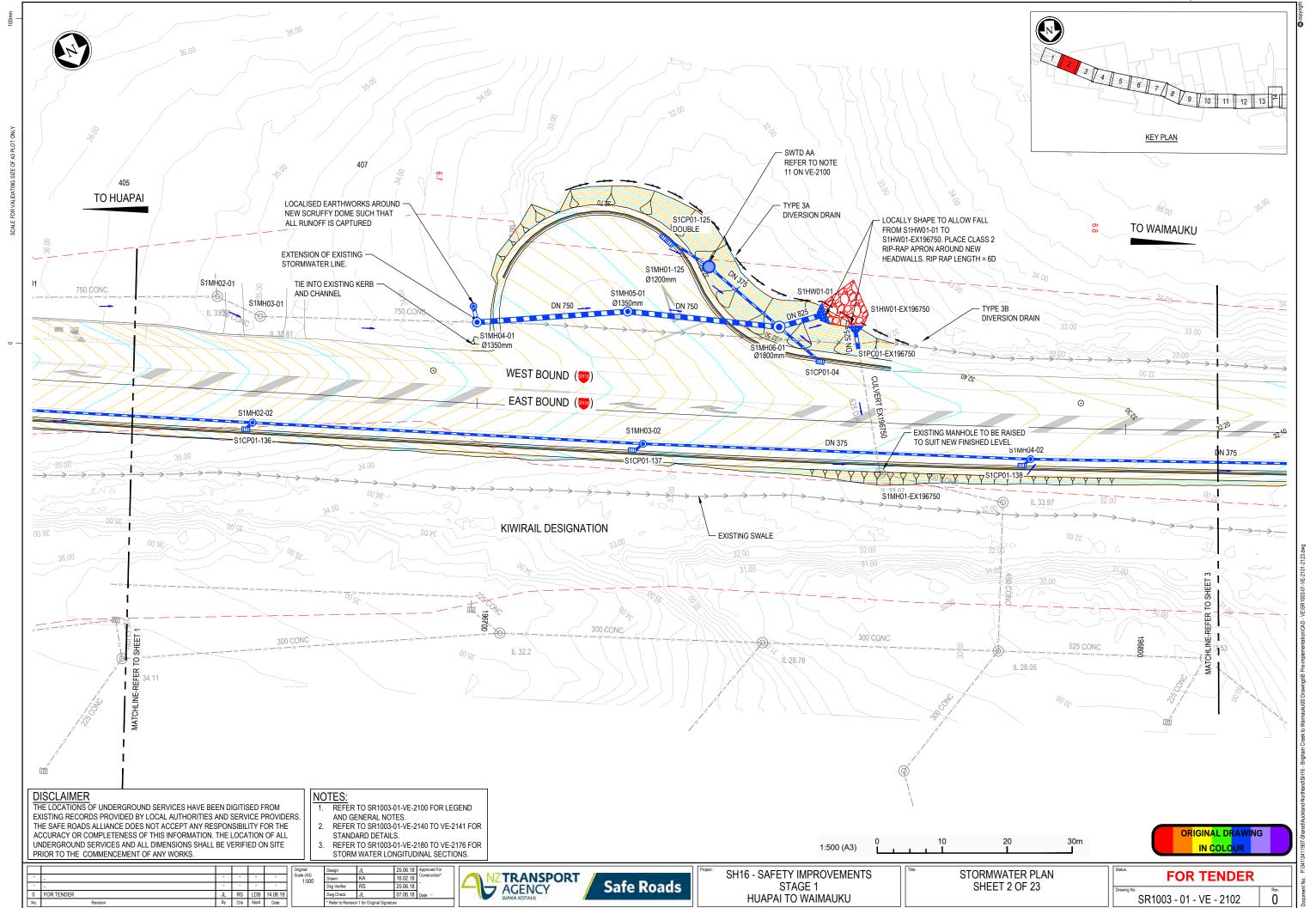
Safe Roads

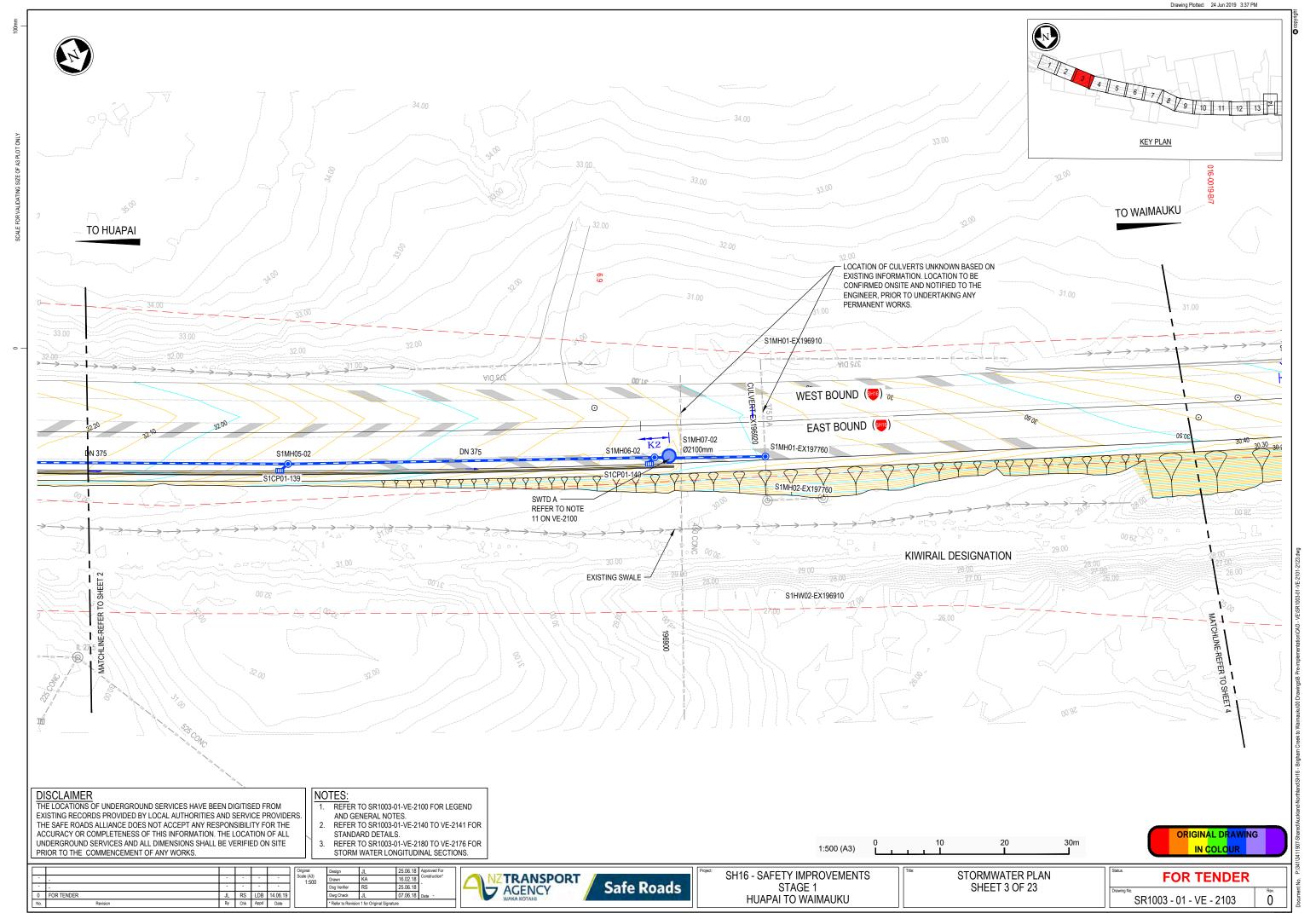
SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU

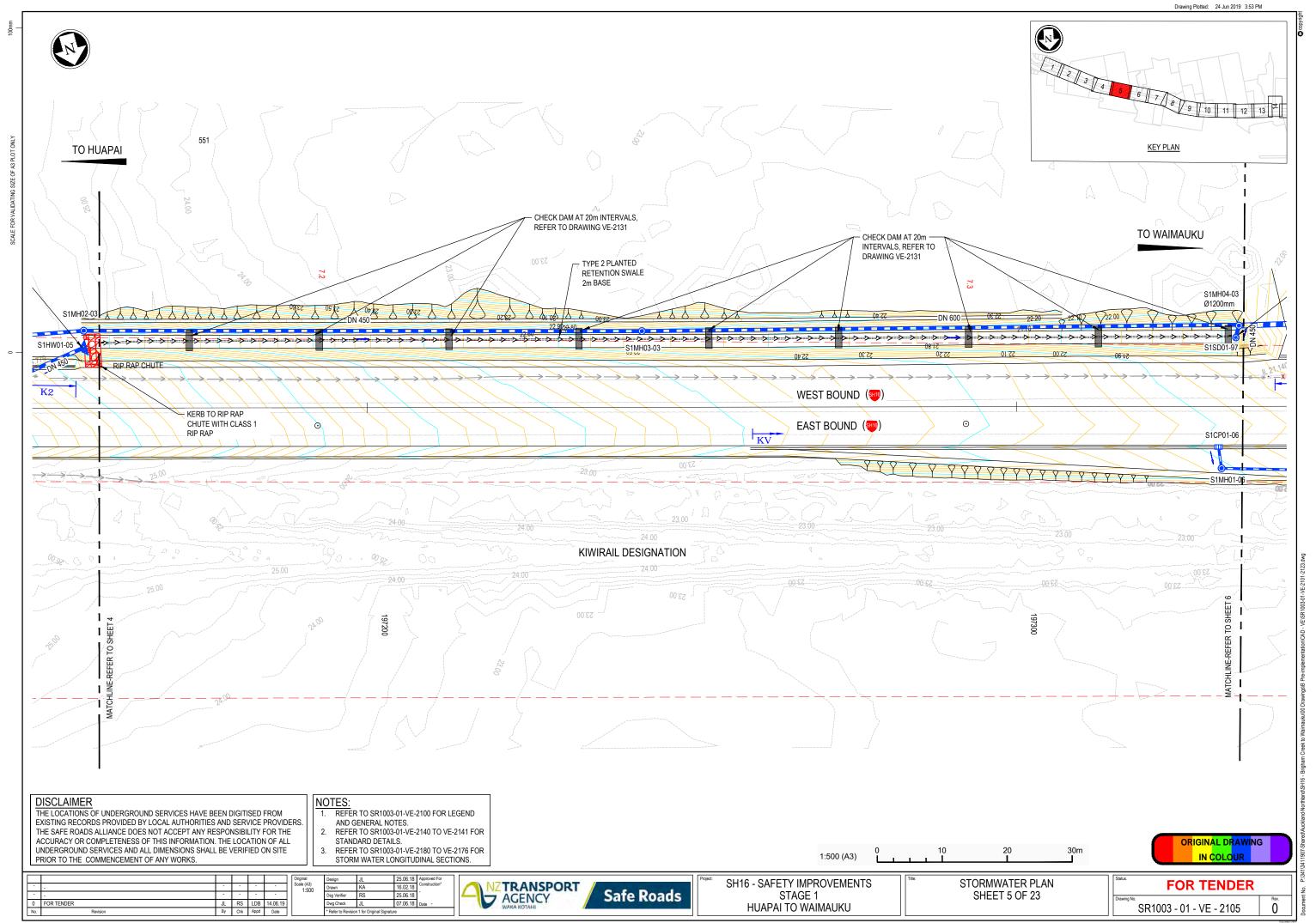
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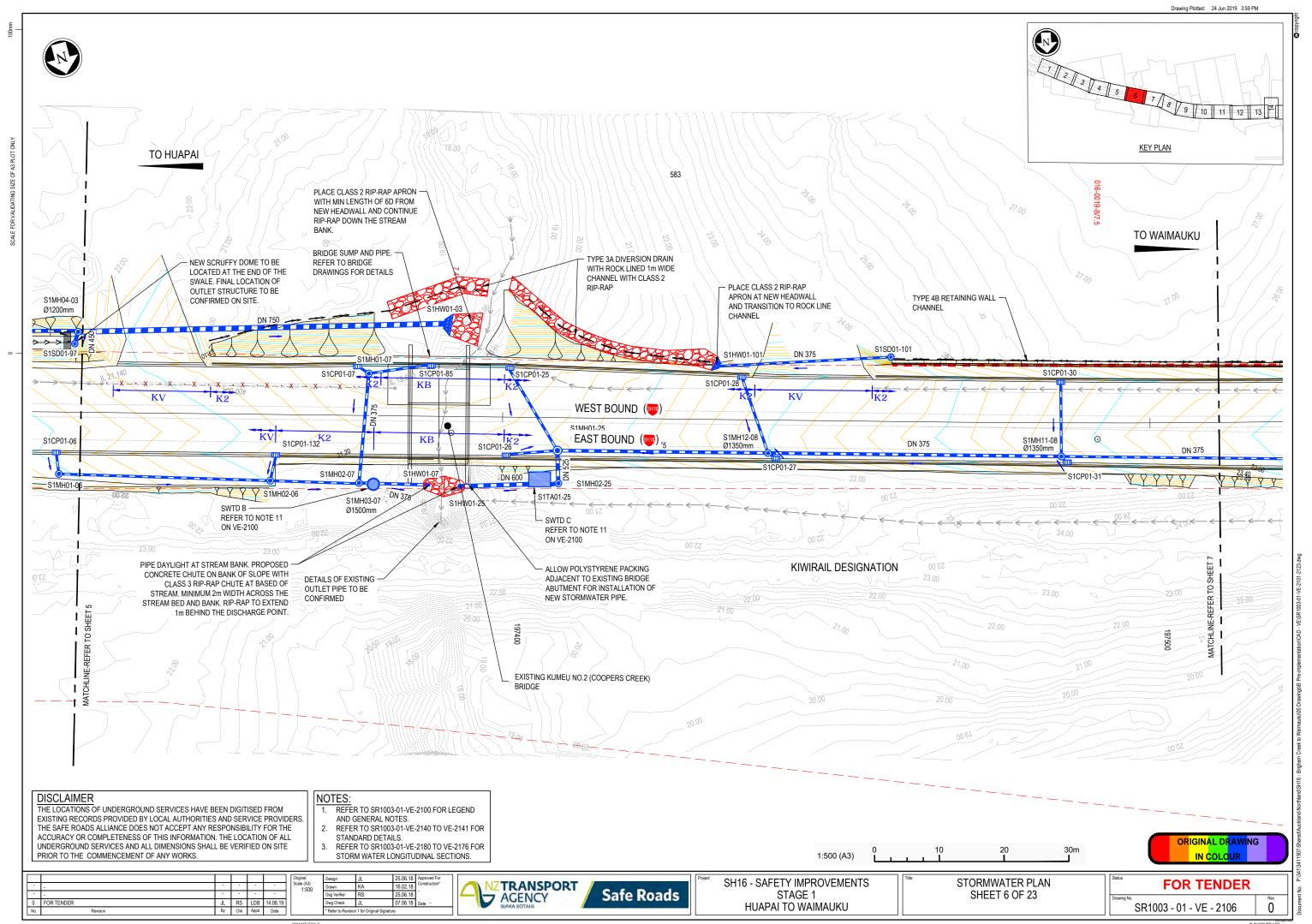
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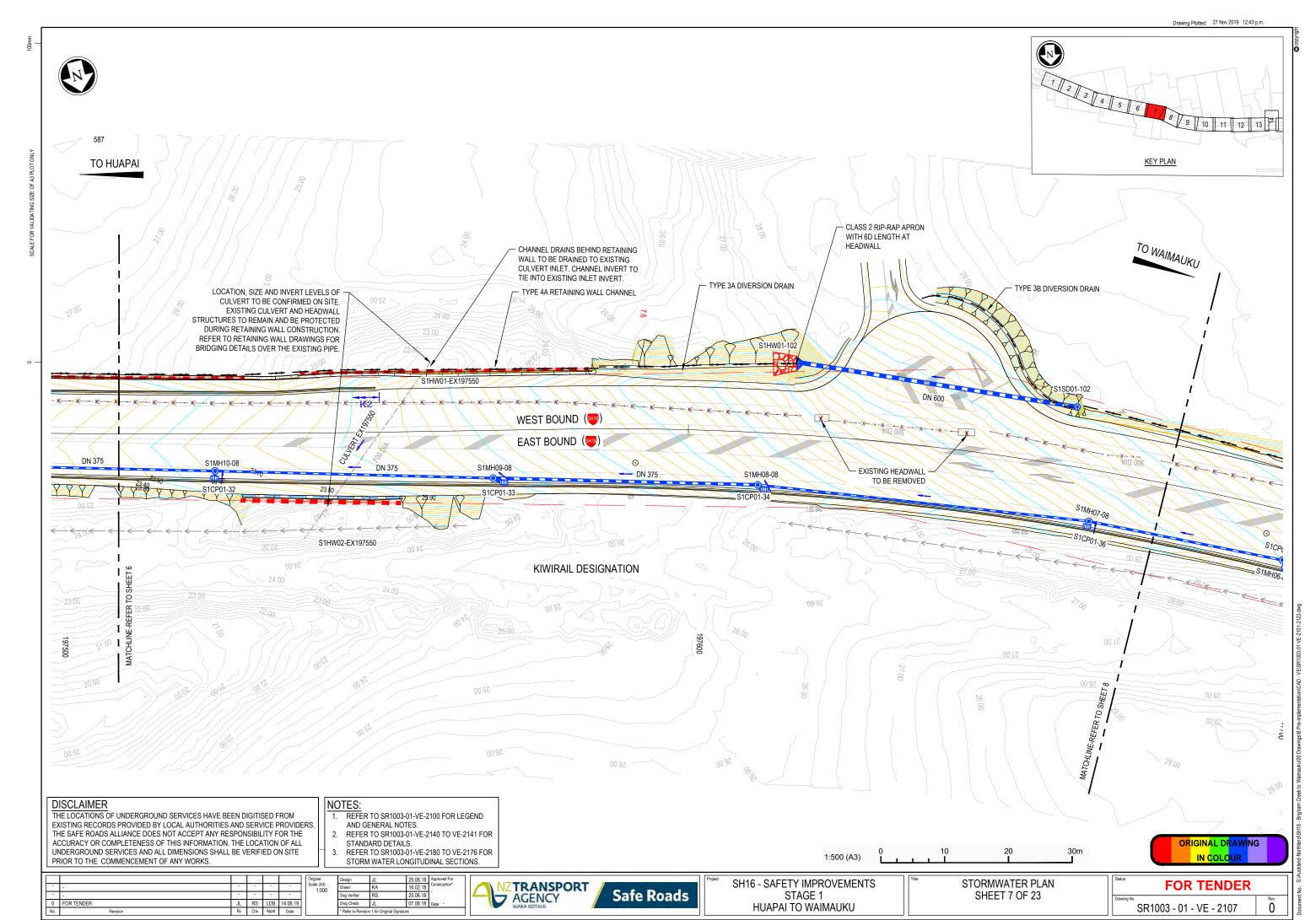




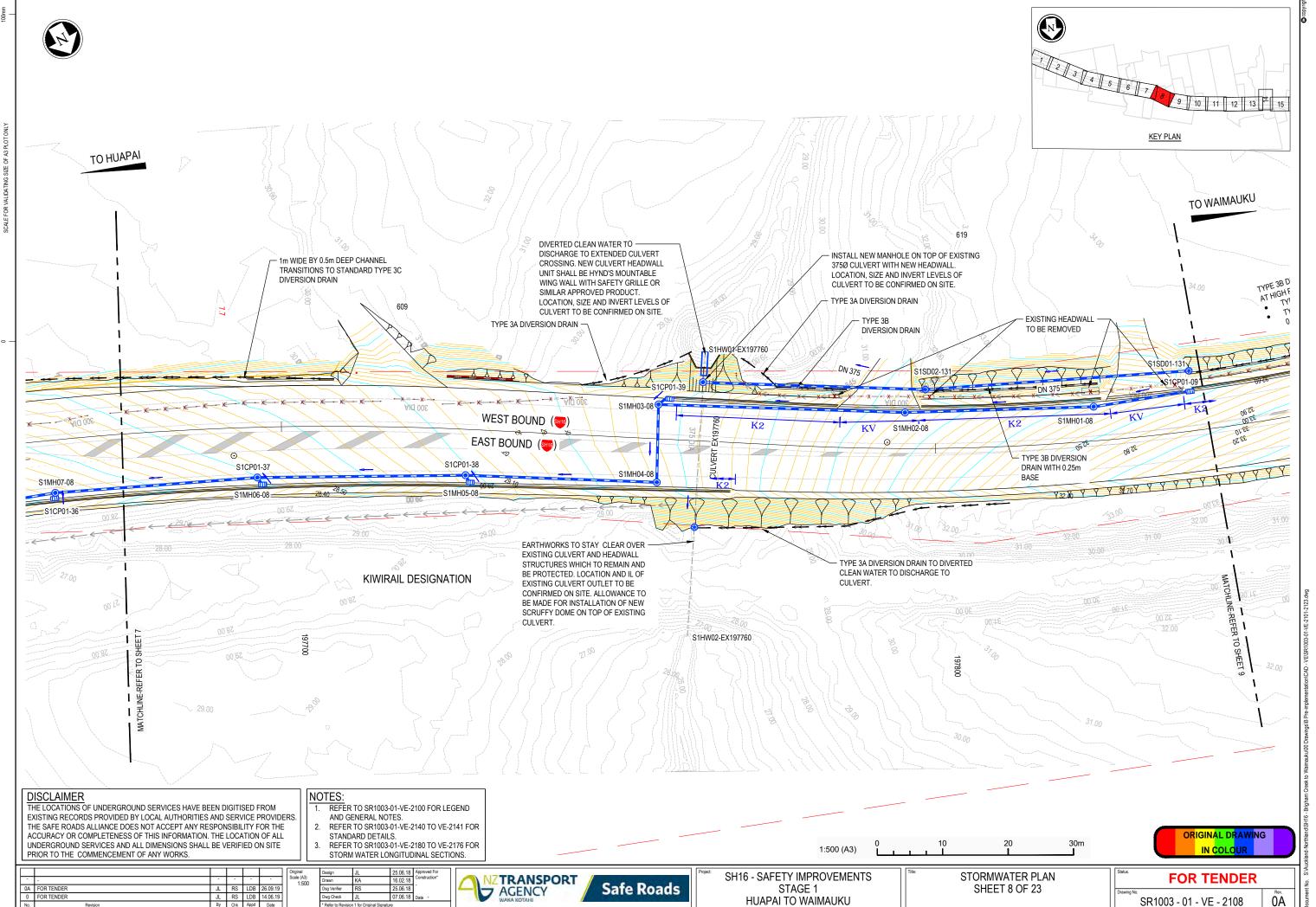


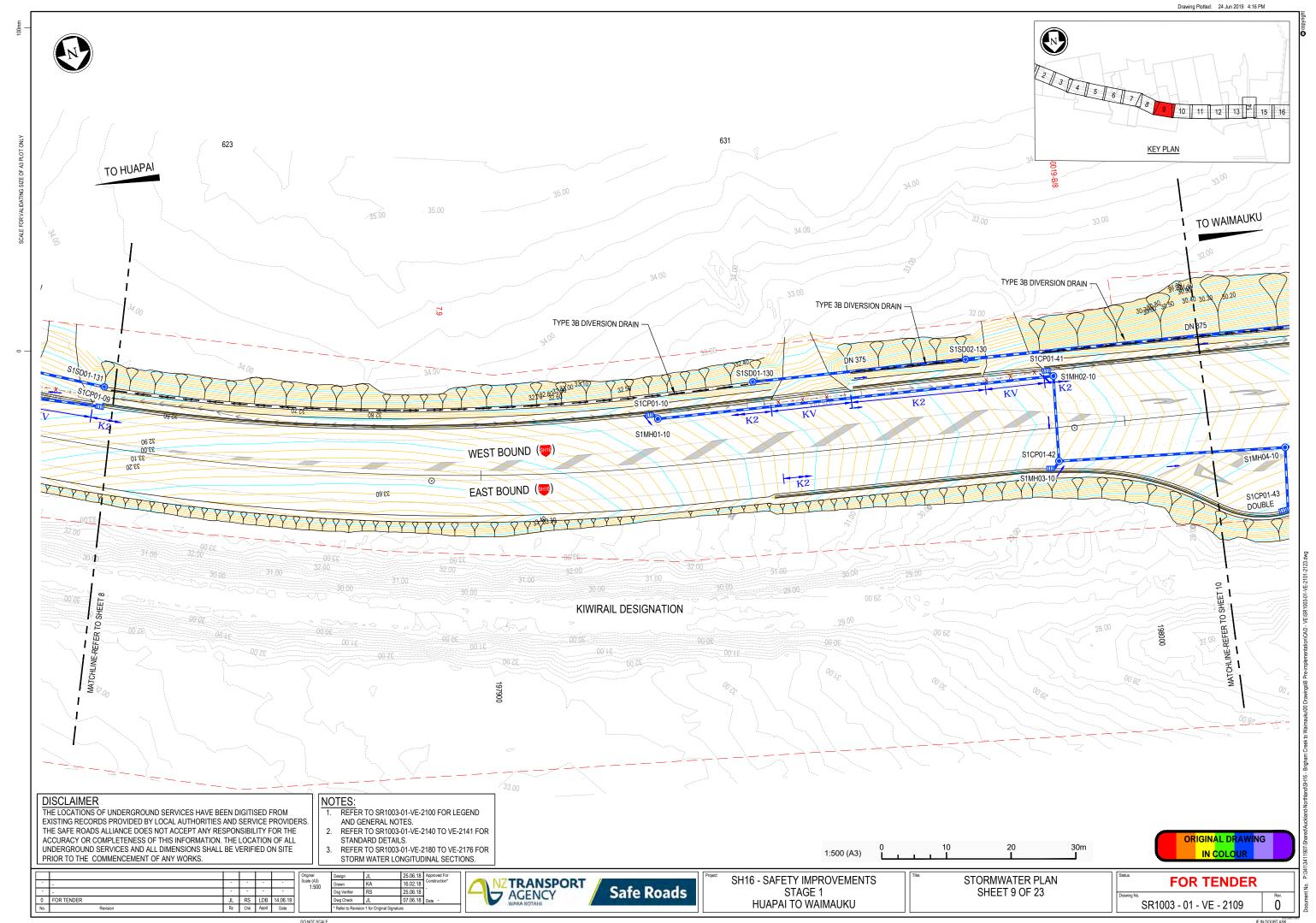


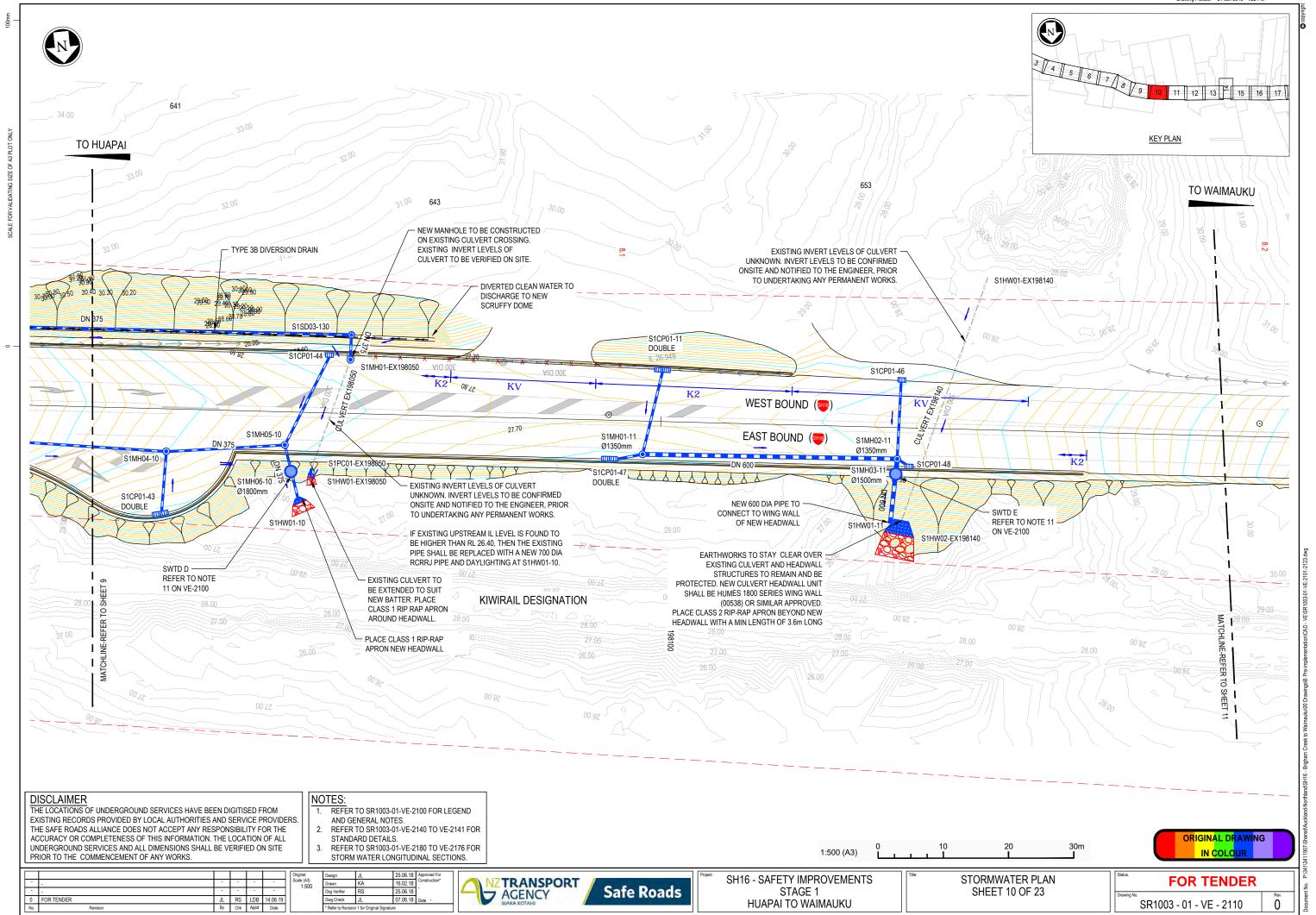


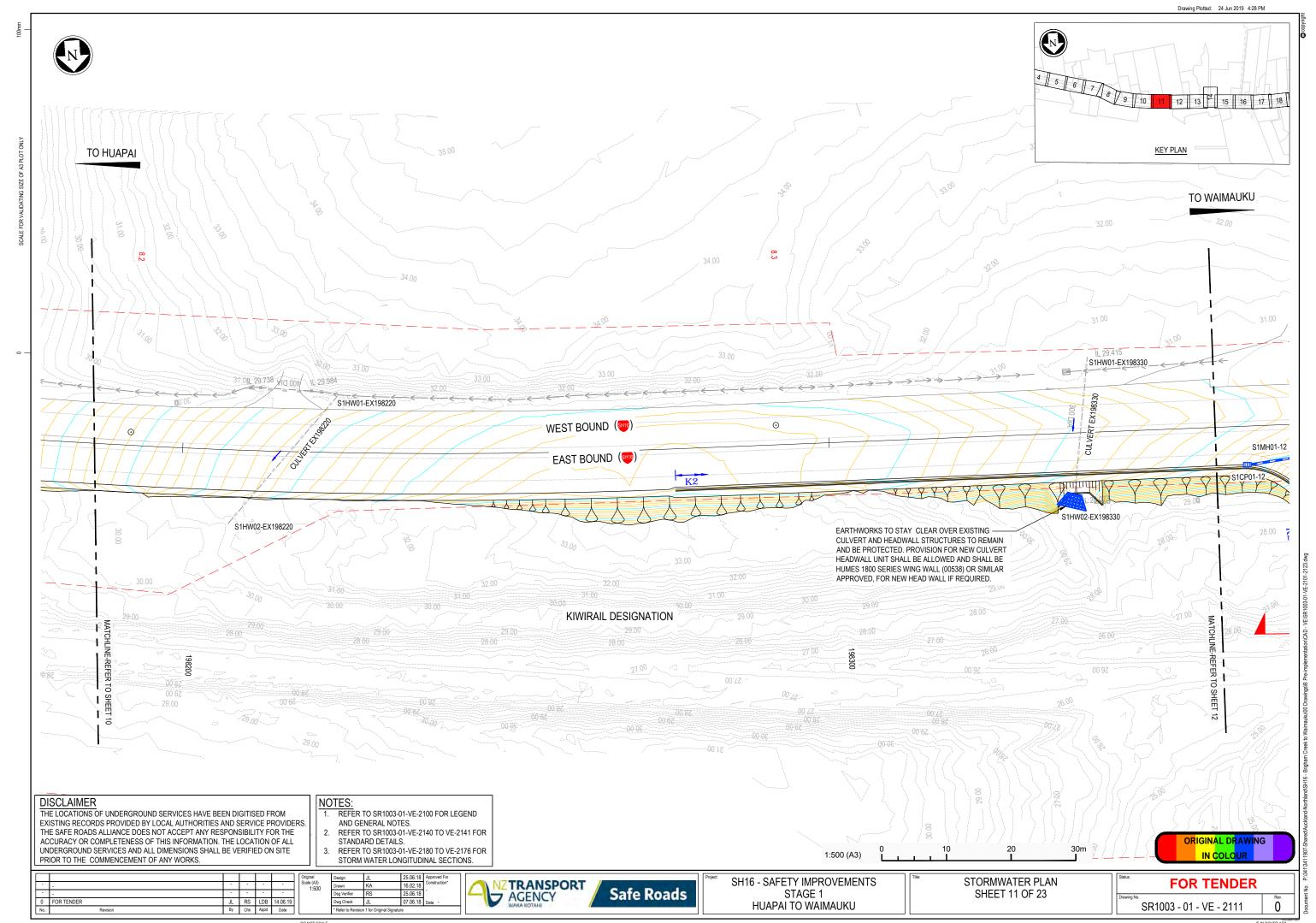


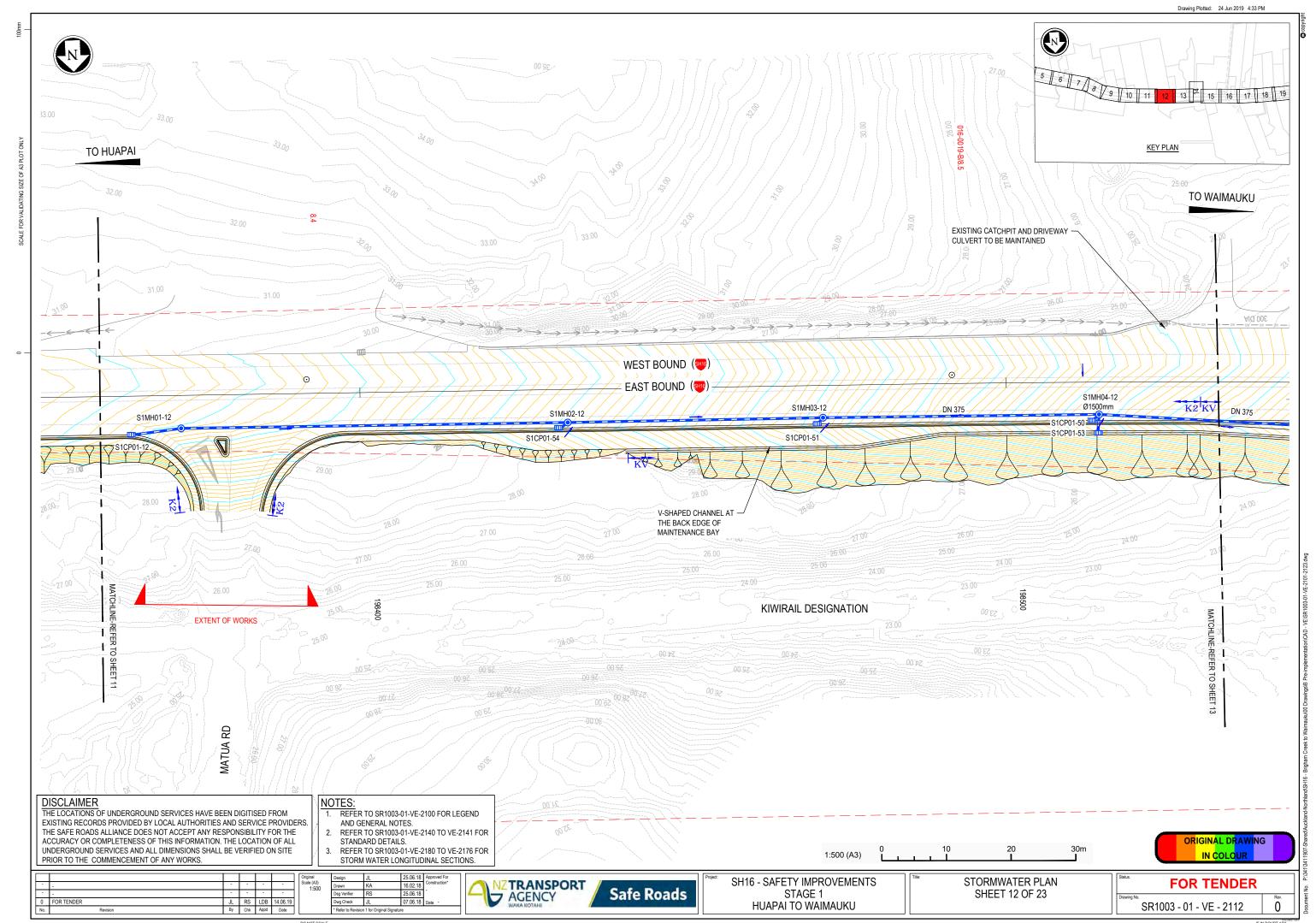


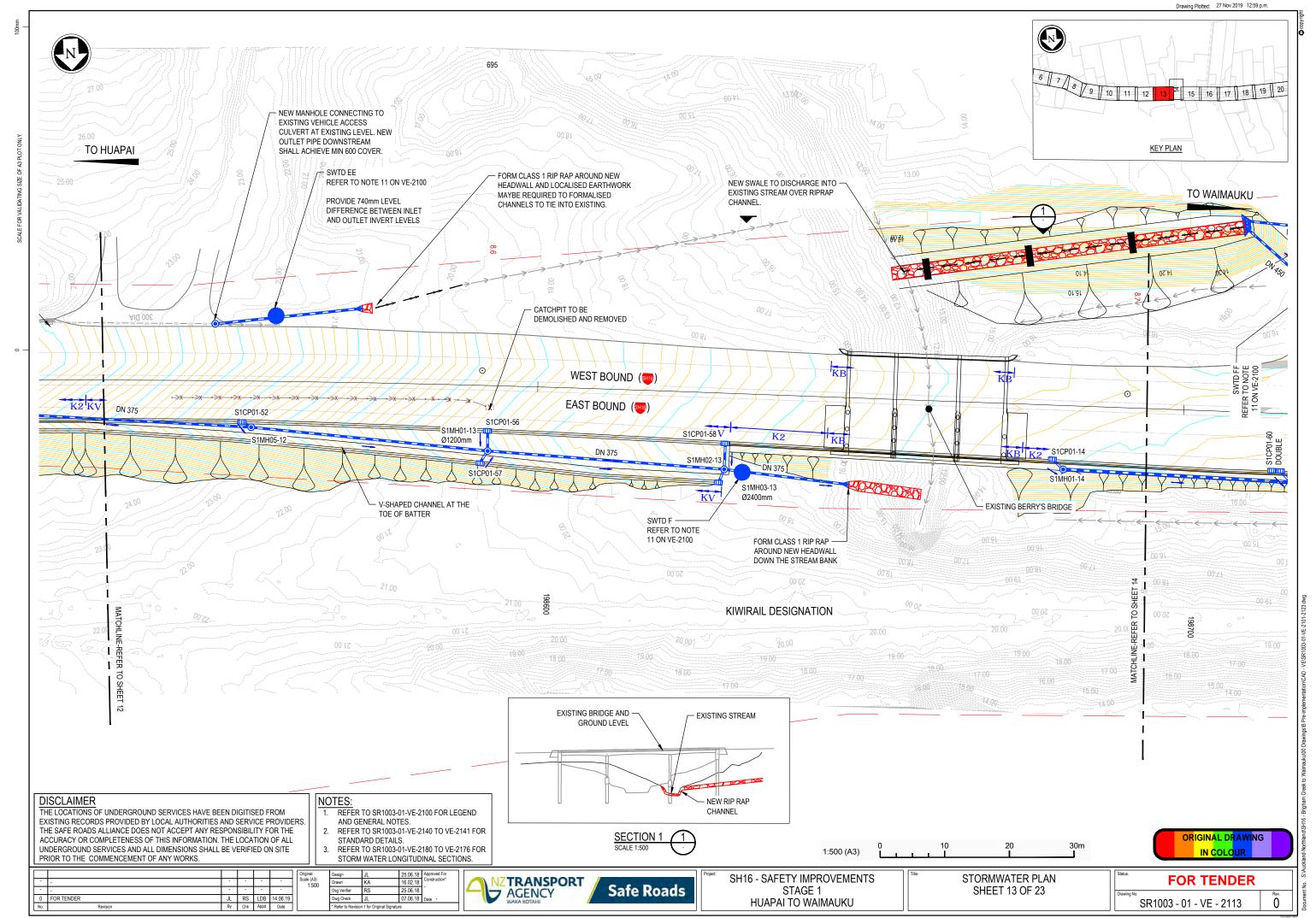












Safe Roads

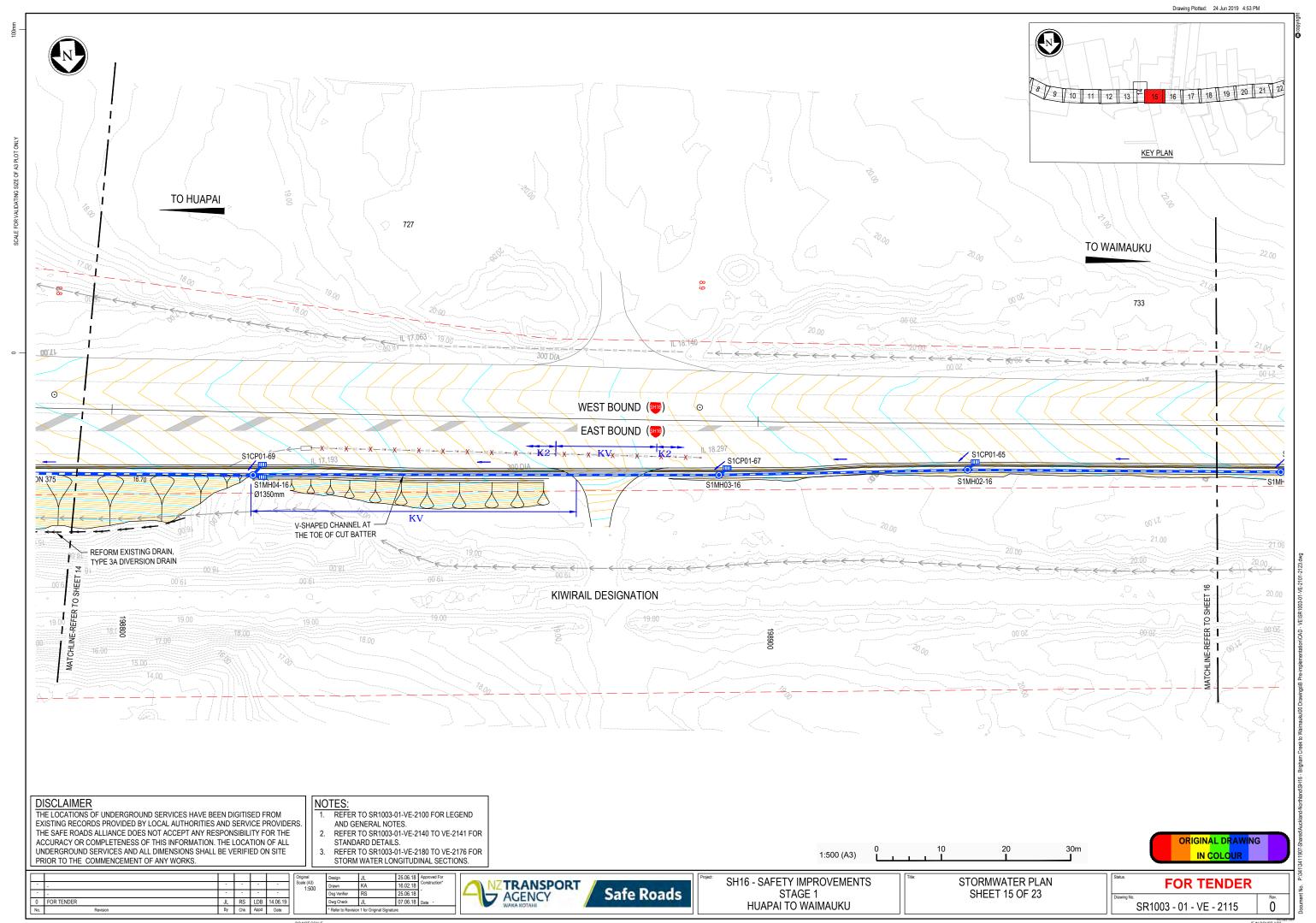
STAGE 1

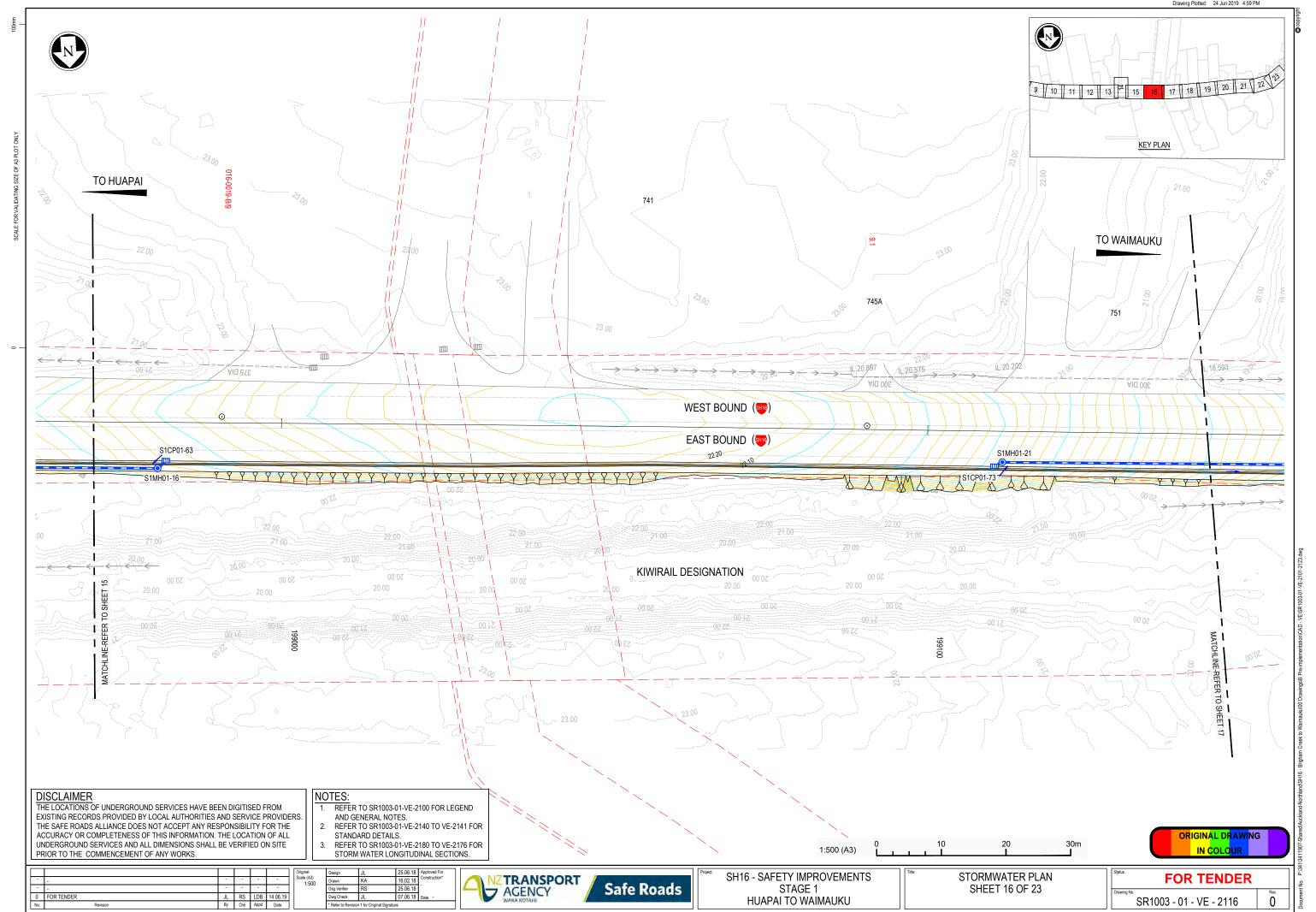
HUAPAI TO WAIMAUKU

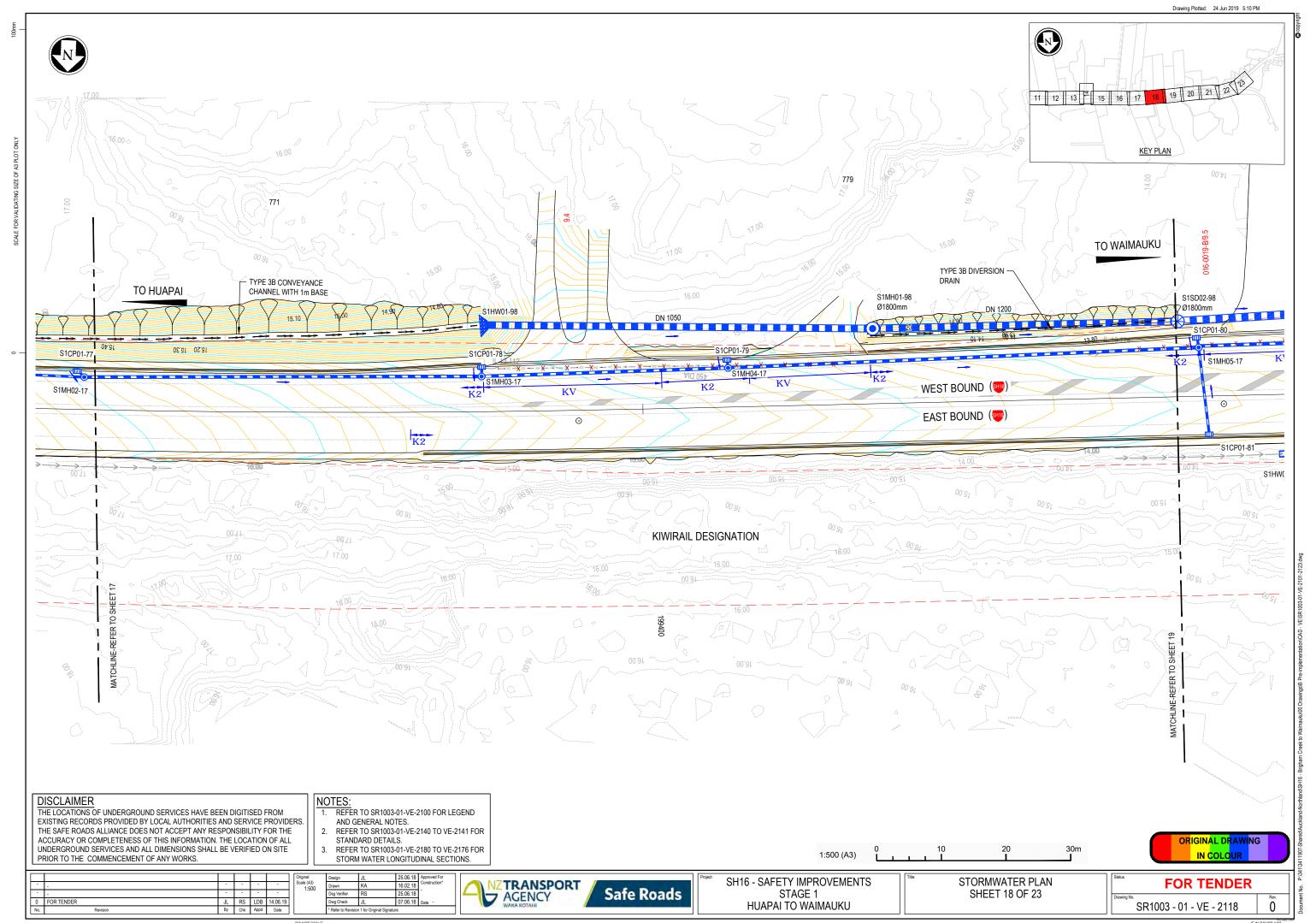
SHEET 14 OF 23

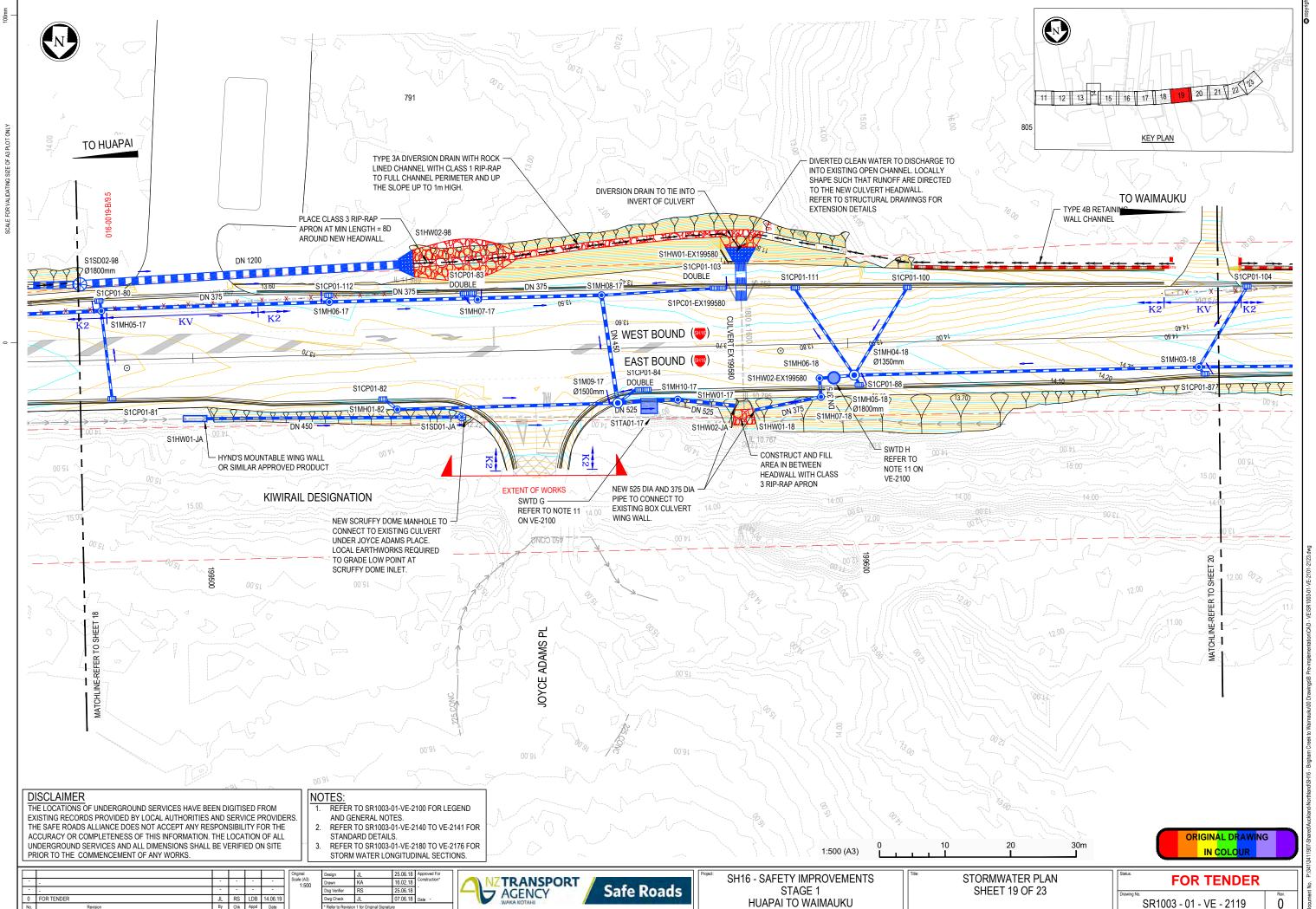
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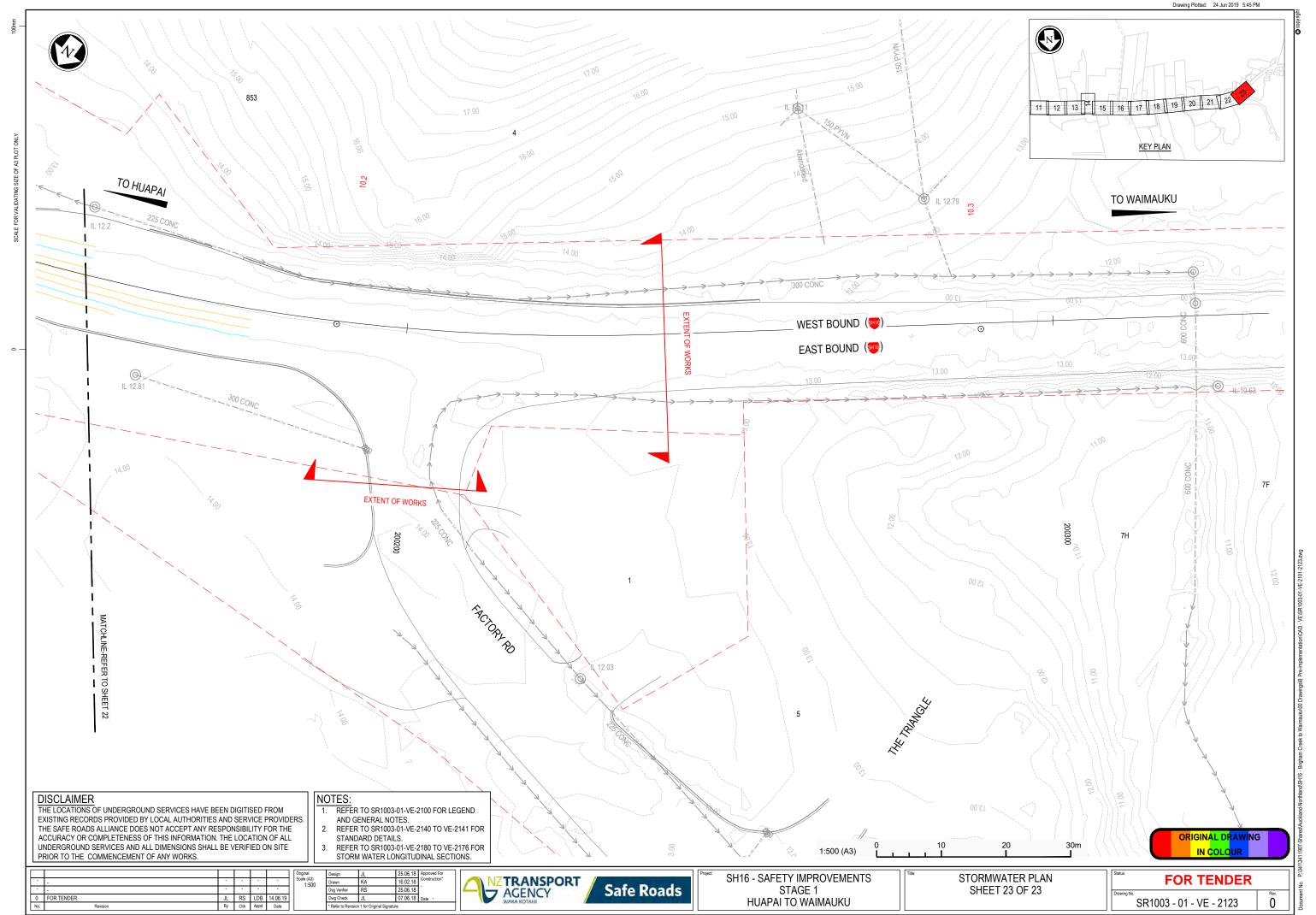
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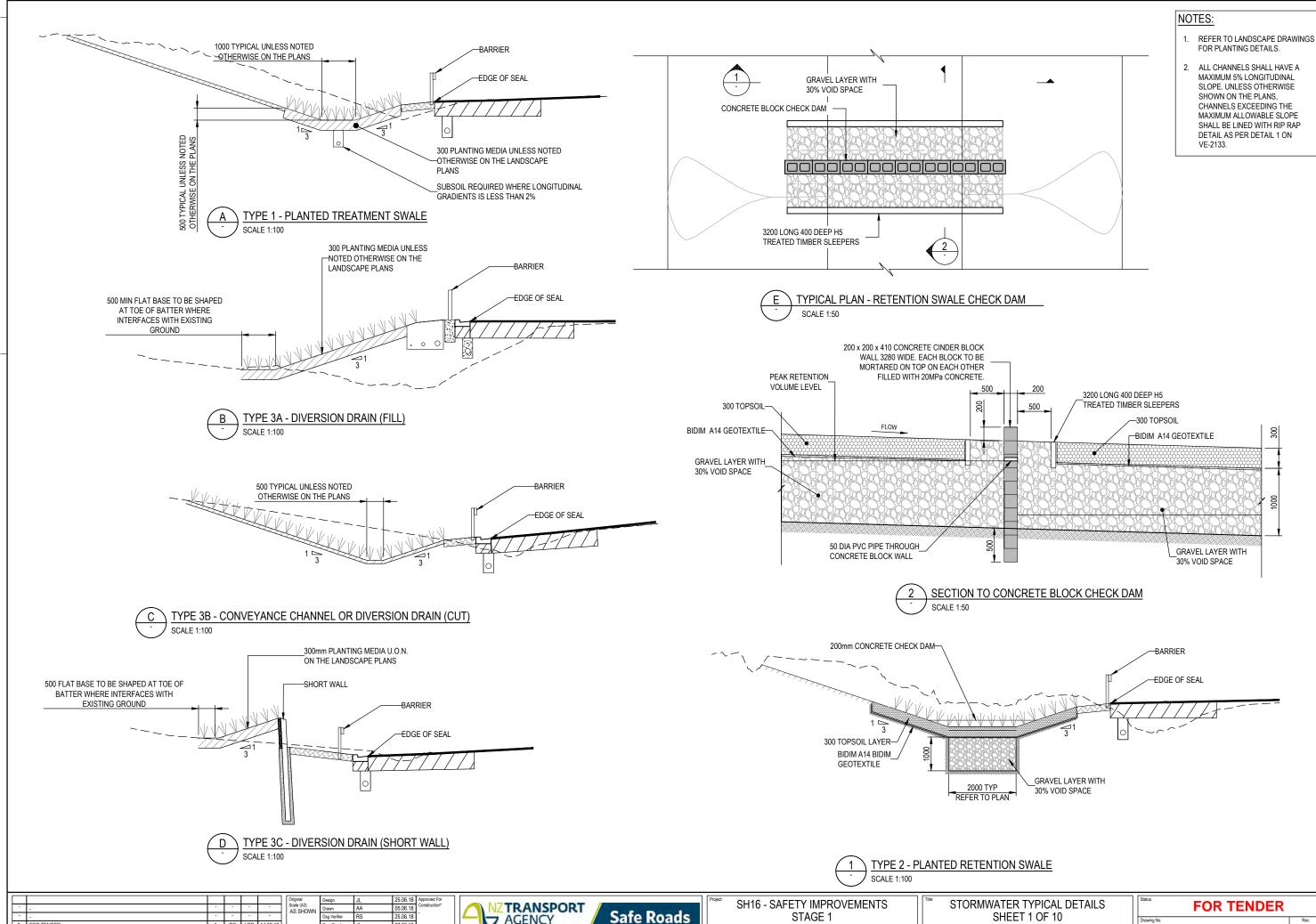




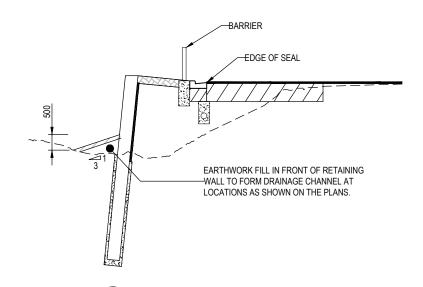


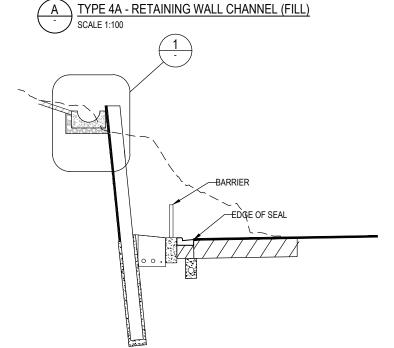


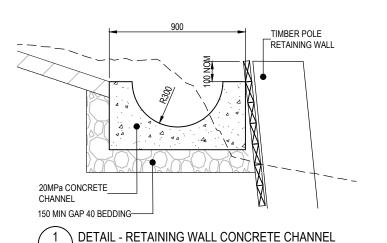




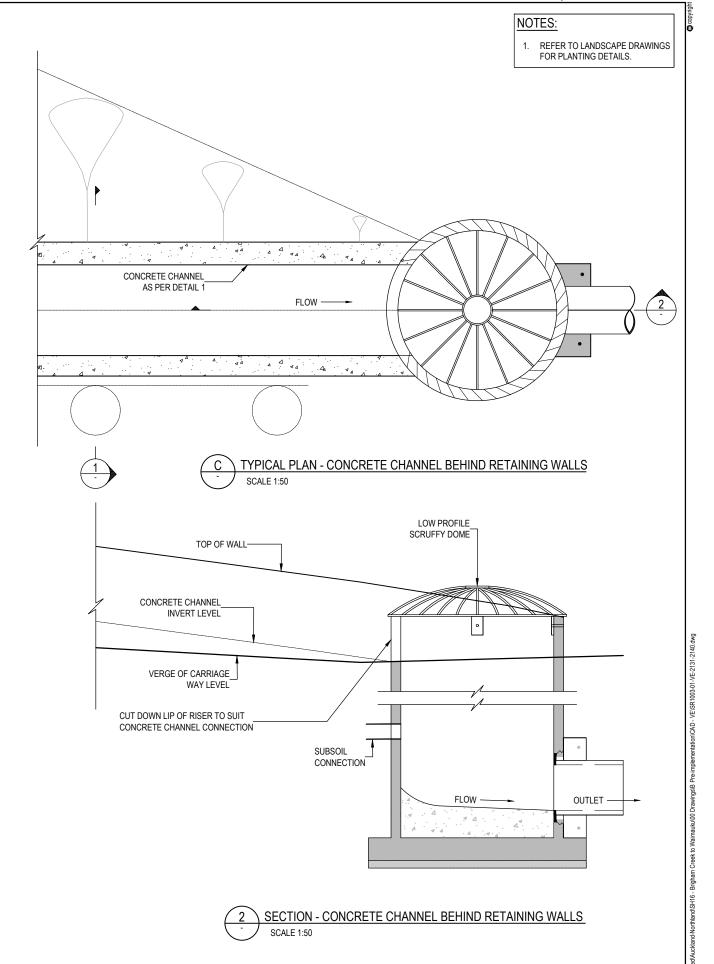
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TYPE 4B - RETAINING WALL CHANNEL (CUT)





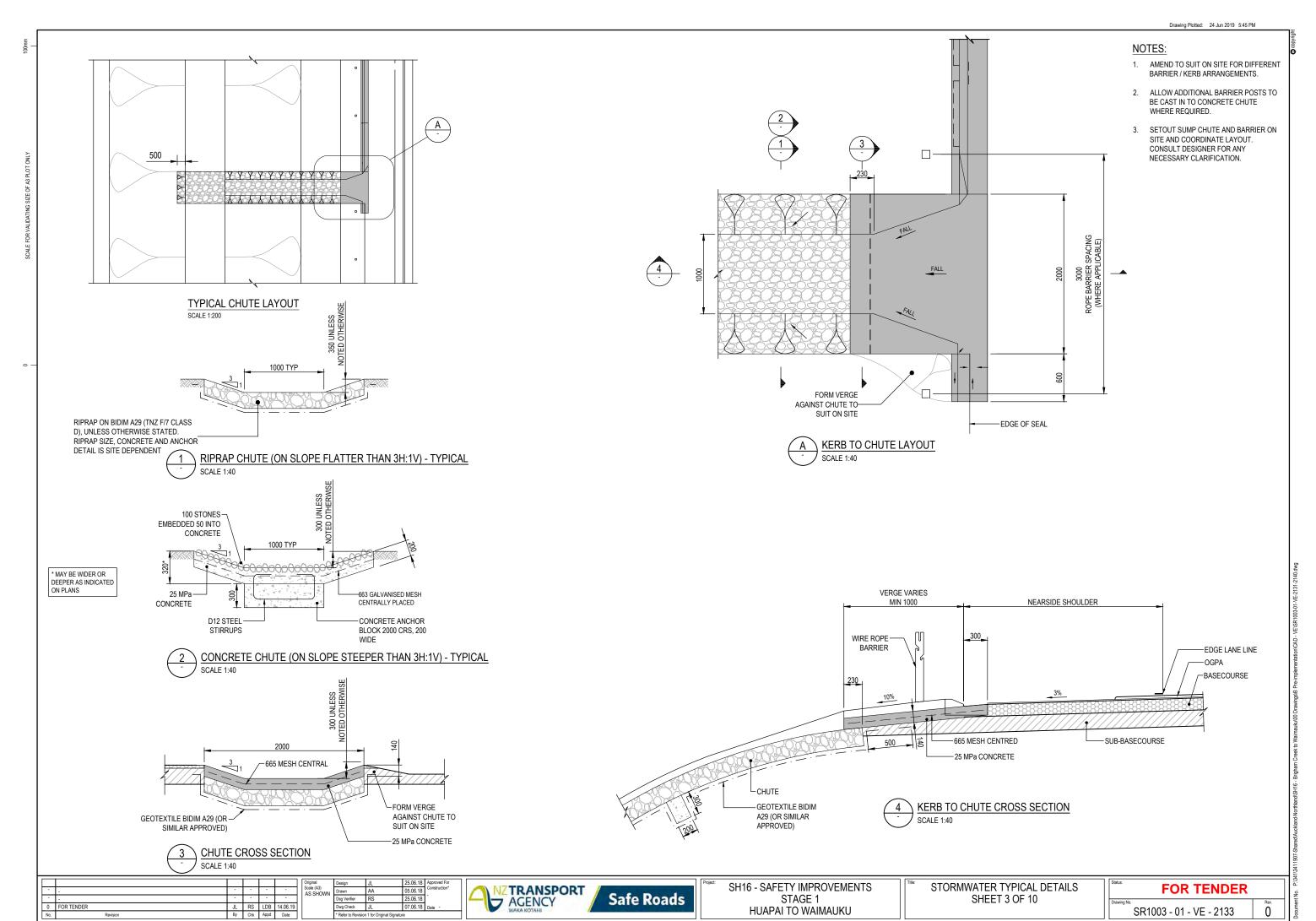
Safe Roads

SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER TYPICAL DETAILS SHEET 2 OF 10 FOR TENDER

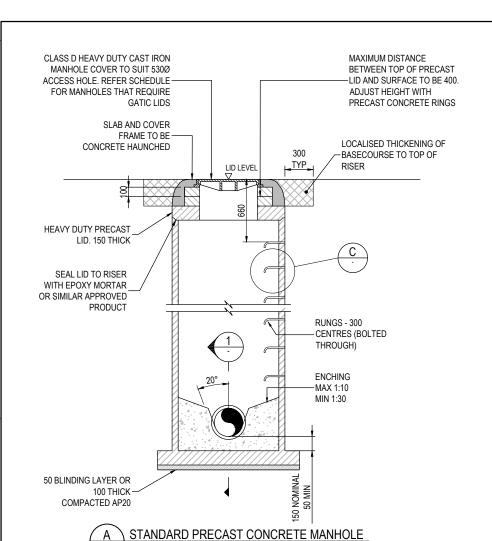
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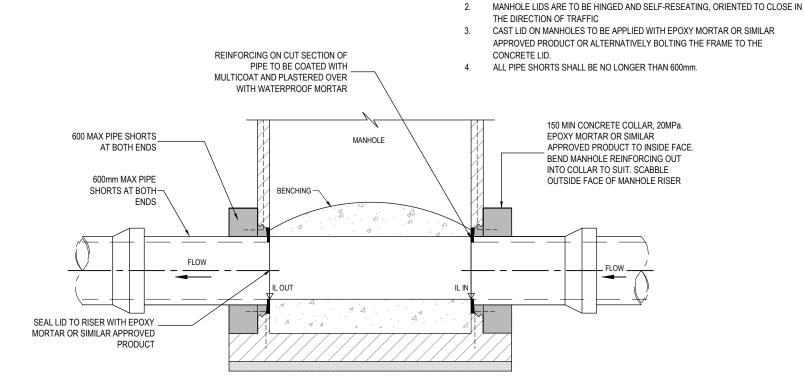
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REFER TO LONG SECTIONS FOR DEPTHS AND INVERT LEVELS. REFER TO 3D



SCALE 1:40

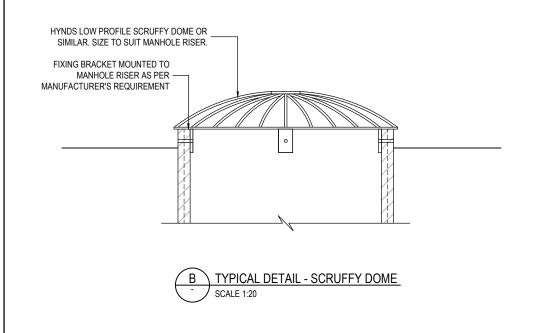


CONCRETE PIPE TO MANHOLE CONNECTION

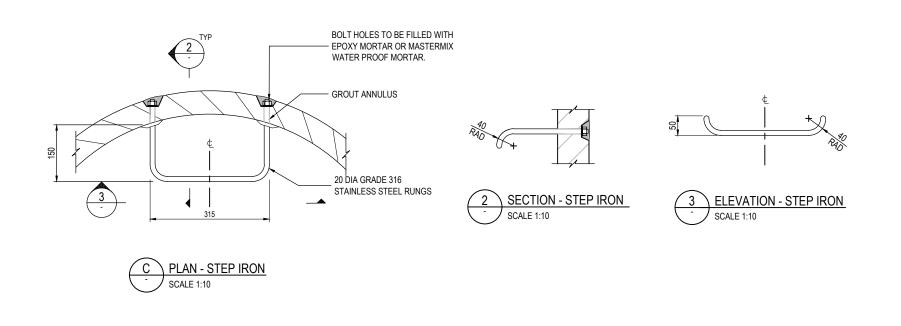
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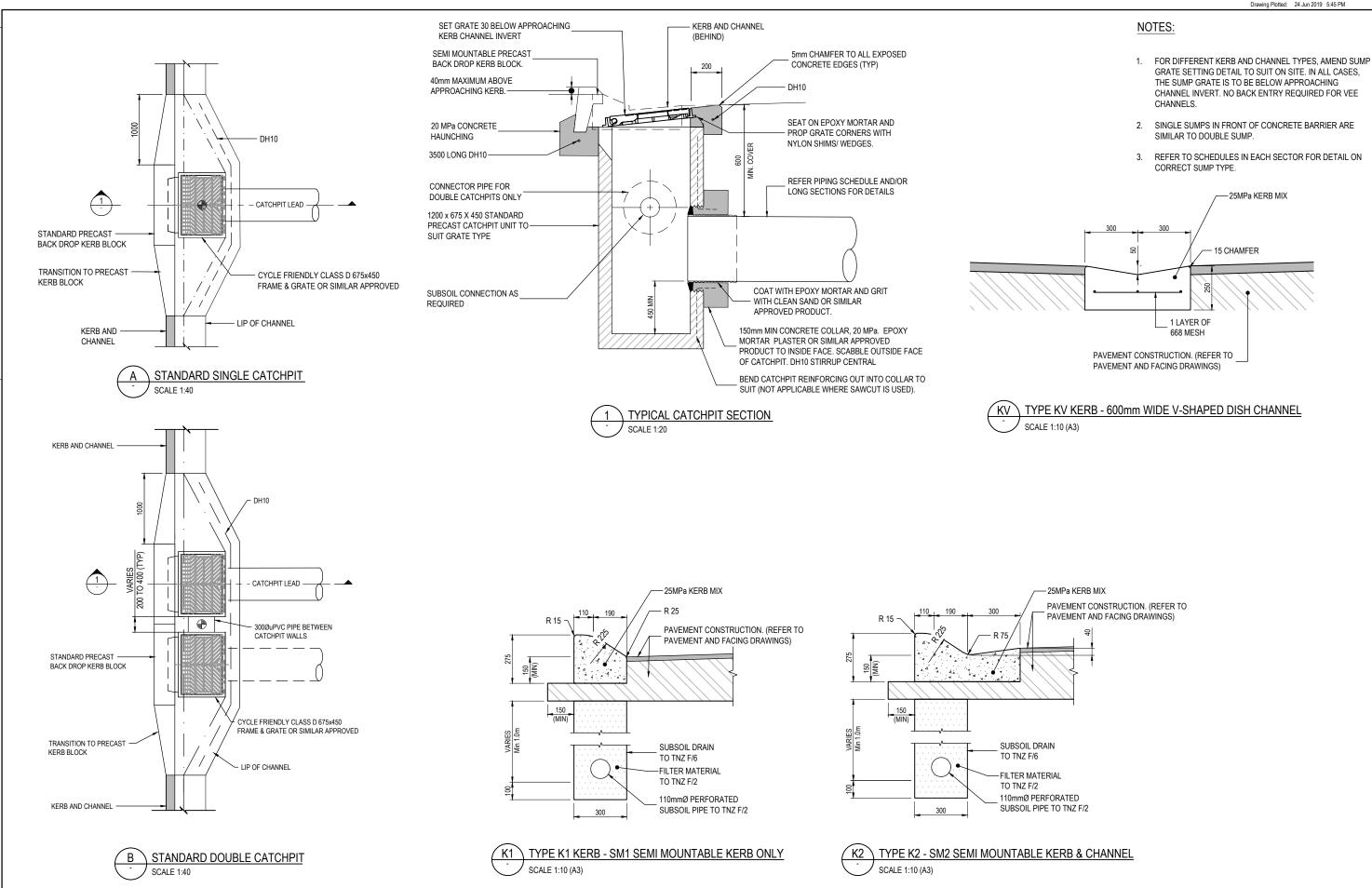
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By Chk Appd Date



SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER TYPICAL DETAILS SHEET 4 OF 10 FOR TENDER
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NZTRANSPORT

→ AGENCY

By Chk Appd Date

Safe Roads

SH16 - SAFETY IMPROVEMENTS

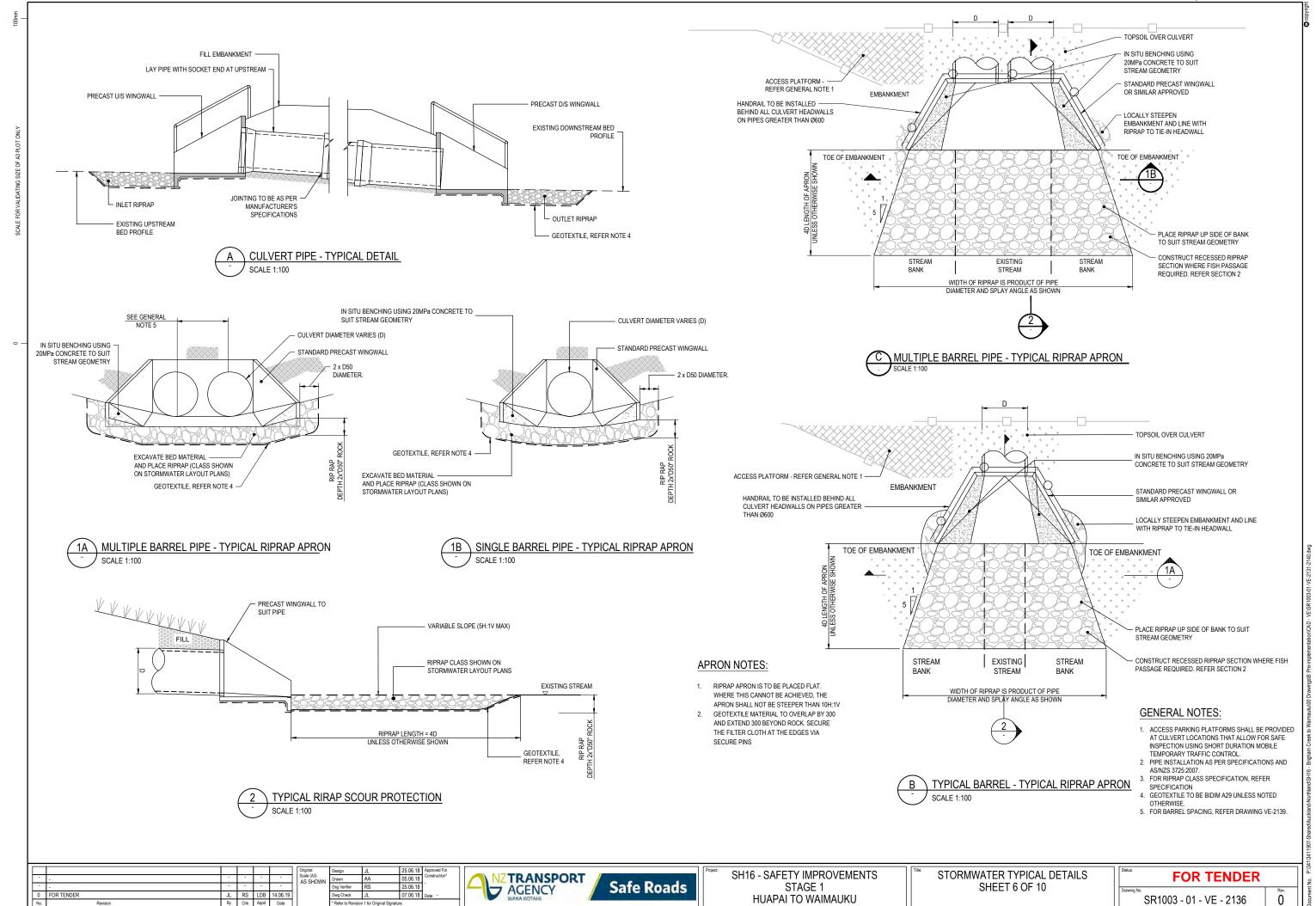
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HUAPAI TO WAIMAUKU

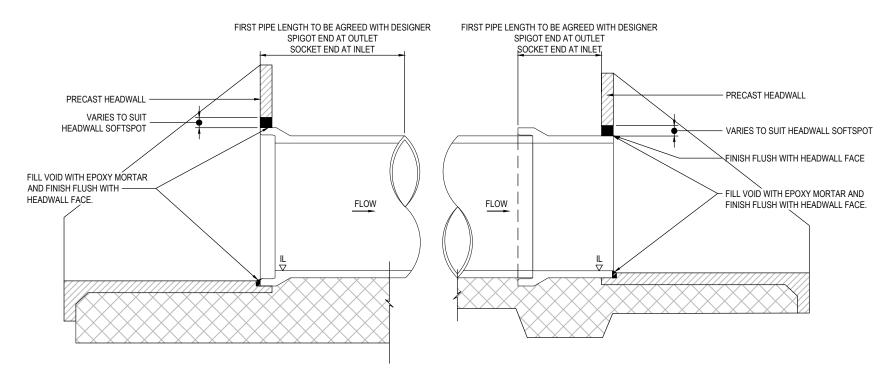
STORMWATER TYPICAL DETAILS

SHEET 5 OF 10

FOR TENDER

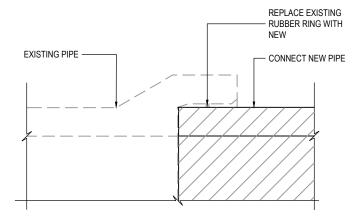


FOR PIPE DIAMETER, INVERT LEVELS, MATERIAL AND CLASS REFER PIT TO SCHEDULE AND / OR LONG SECTION DRAWINGS



CONCRETE CULVERT TO HEADWALL CONNECTION

SCALE 1:40



B TYPICAL DETAIL - CULVERT EXTENSION FROM EXISTING
SCALE 1:10

CULVERT EXTENSION CONSTRUCTION NOTES:

THE CONSTRUCTION OF ANY PROPOSED CULVERT EXTENSION NEED TO BE CARRIED IN THE FOLLOWING:

- SITE CLEARANCE AROUND THE INLET AND OUTLET OF THE EXISTING CULVERT
- INVESTIGATION OF EXISTING CULVERTS AS PER PROJECT SPECIFICATION
- 3. CONFIRM UPSTREAM WORKS AND EXTENT
- REMOVE THE FIRST LENGTH OF EXISTING PIPE
- 5. PROTECT EXISTING COLLAR AND DOWNSTREAM PIPE
- 6. CLEAN EXISTING JOINT
- 7. BEDDING, NEW PIPE INSERTION, PIPE SURROUND AND BACKFILL OF PIPE TRENCH AS PER SPECIFICATION

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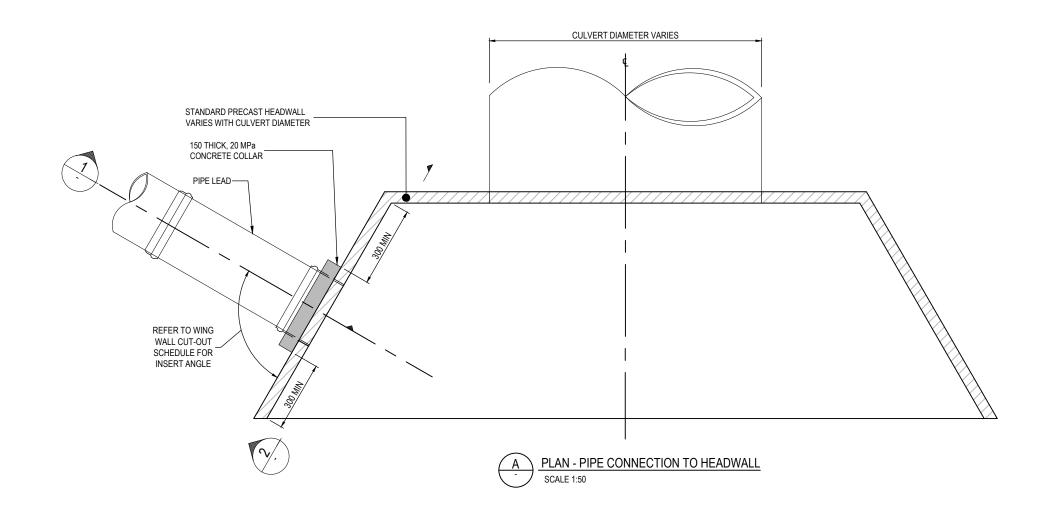


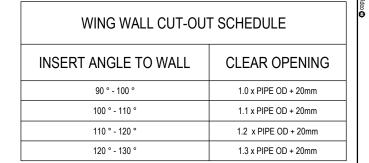


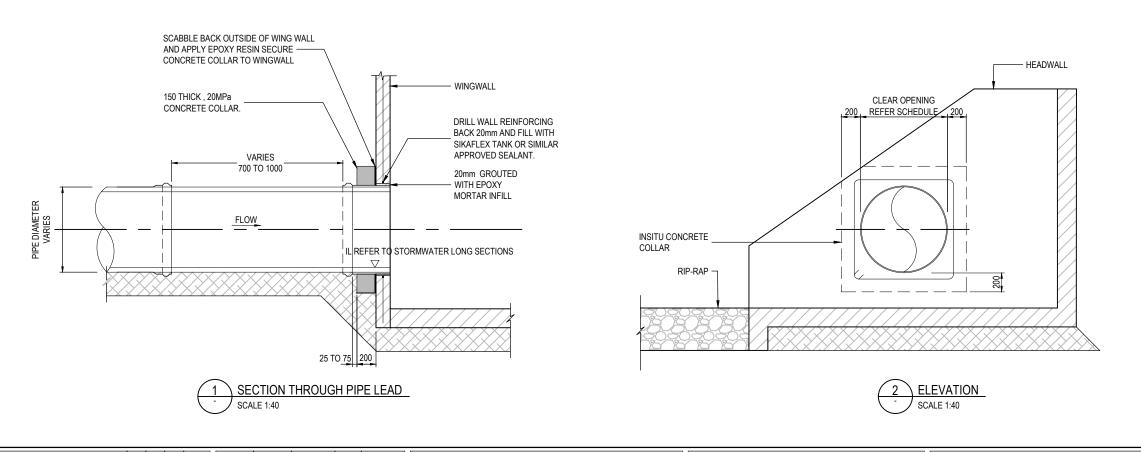
SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER TYPICAL DETAILS SHEET 7 OF 10 FOR TENDER

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Safe Roads

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SH16 - SAFETY IMPROVEMENTS

STAGE 1

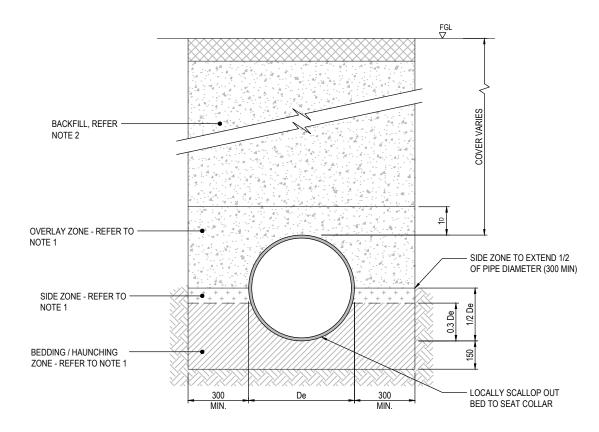
HUAPAI TO WAIMAUKU

STORMWATER TYPICAL DETAILS

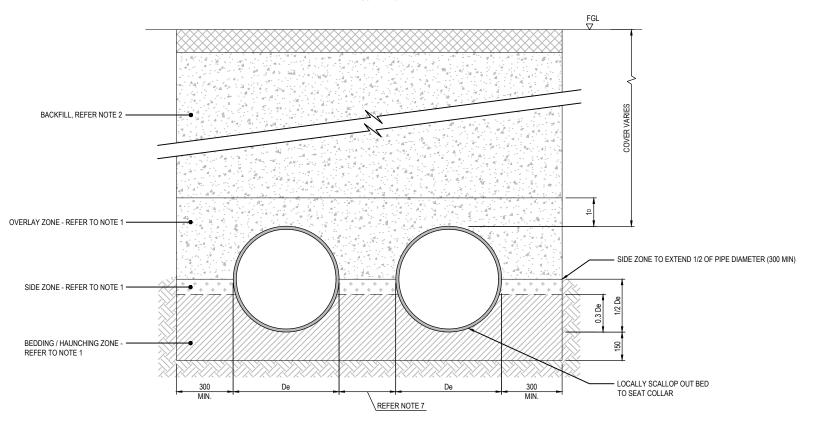
SHEET 8 OF 10

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By Chk Appd Date

FOR TENDER



HS2 TRENCH / BEDDING FOR SINGLE BARREL CONCRETE PIPES SCALE 1:20



HS2 TRENCH / BEDDING FOR MULTIPLE BARREL CONCRETE PIPES

SCALE 1:10

NOTES:

BEDDING / HAUNCHING / SIDE / EMBEDMENT MATERIAL TO BE IN
 ACCORDANCE WITH THE TABLES BELOW. THESE ZONES TO BE COMPACTED
 IN 150mm MAX. LAYERS TO MIN. 90% OF MDD. TESTING SHALL BE IN
 ACCORDANCE WITH TECHNICAL SPECIFICATION C0204. OVERLAY ZONE TO
 BE COMPACTED TO MIN. 90% OF MDD AND SHALL ALSO BE FREE OF STONES
 > 150mm AND CONTAIN LESS THAN 20% OF MATERIAL WITH A SIZE BETWEEN
 75mm AND 150mm

GRADING REQUIREMENTS

BEDDING / H	IAUNCI	HING /	EMBED	MENT	/ ZONE	≣		
SIEVE SIZE (mm) 19 2.36 0.6 0.3 0.15 (
% MASS PASSING	100	100-50	90-20	60-10	25-0	10-0		

	SI	DE ZON	ΙE		
SIEVE SIZE (mm)	75	9.5	2.36	0.6	0.075
% MASS PASSING	100	100-50	100-30	50-15	25-0

- BACKFILL TO BE FREE FROM LUMPS > 150mm AND STONES > 65mm. REFER TO SPECIFICATIONS. REFER TO PAVEMENT PLANS AND DETAILS WHERE UNDER ROAD. REFER TO EARTHWORKS PLANS AND DETAILS ELSEWHERE.
- 3. FORMATION LAYER TO BE TESTED WITH SCALA PENETROMETER EVERY 10m AND SHALL HAVE MINIMUM 2 BLOWS / 100mm.

4. EMBEDMENT MINIMUM VALUES

D	MINIMUM	VALUES
De	1c	1 _D
>300 ≤450	200	150
>450 ≤900	300	150
>900 ≤1500	350	200
>1500 ≤4000	0.25De	300

- FOR EMBANKMENT INSTALLATION, THE SAME EMBEDMENT REQUIREMENTS SHALL BE MET. THE POSITIVE PROJECTION SHALL BE LESS THAN 0.5 TIMES THE PIPE DIAMETER.
- 6. FOR PIPE CLASS AND MATERIAL REFER STORMWATER LONGITUDINAL SECTION DRAWINGS.
- 7. BARREL SPACING SHOWN IN TABLE BELOW IS BASED ON AS/NZS 3725:2007. THESE VALUES ARE MINIMUMS TO BE ADOPTED WHERE PIPE MANUFACTURER HAS NOT SPECIFIED OTHERWISE. FOR CROSS CULVERTS ALL BARREL SPACING WILL BE GOVERNED BY THE HEADWALL PENETRATION SPACING.

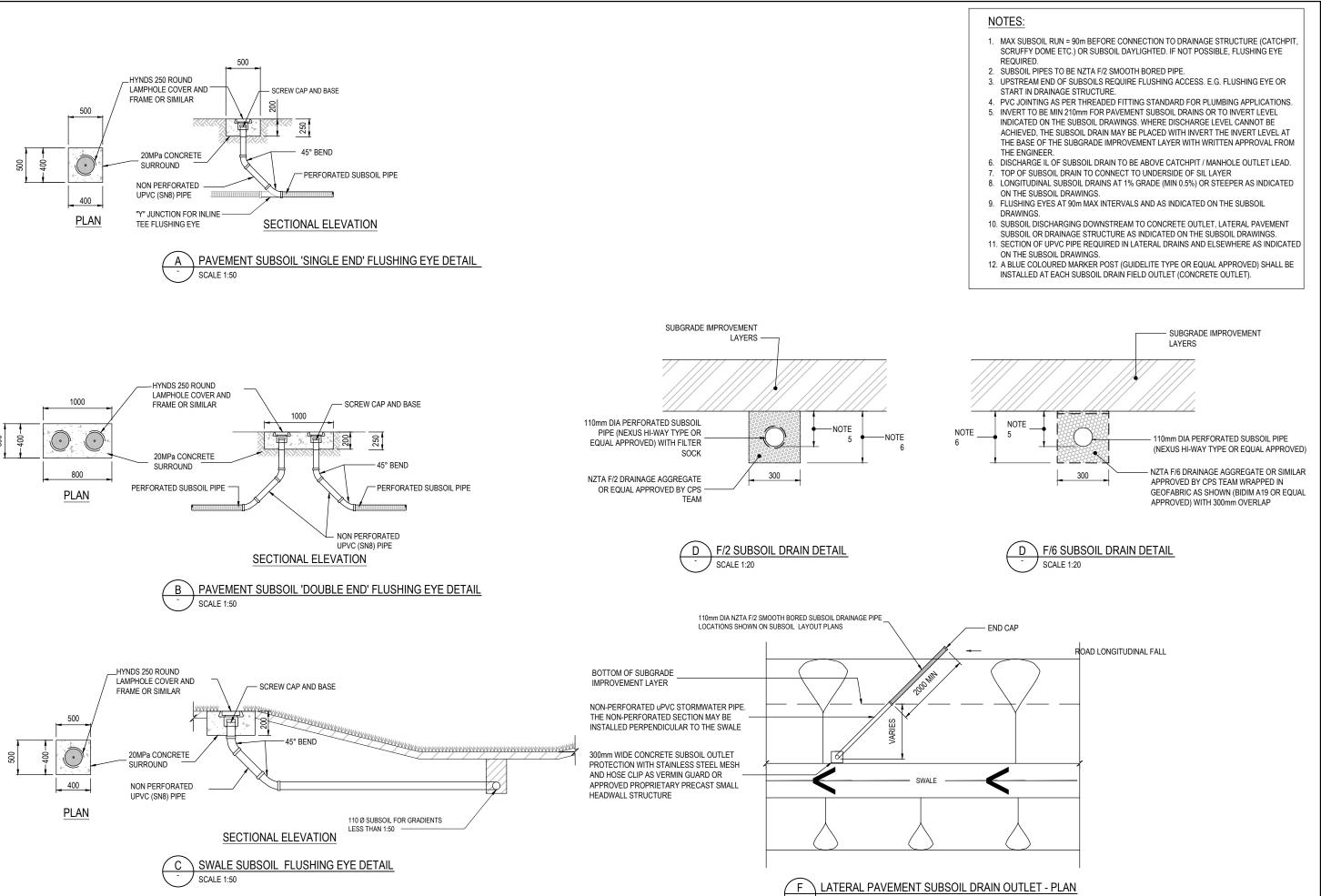
BARRE	L SPACING
PIPE INTERNAL DIAMETER (mm)	SEPARATION
<= 600mm	150mm
600 TO 1200mm	200mm
>= 1200mm	D/6 WHERE D IS OUTSIDE DIAMETER OF THE BARREL (NOT THE SOCKET)

8. MINIMUM COVER OF 600mm SHALL BE PROVIDED TO ALL NEW STORMWATER PIPES UNDER THE ROAD CARRIAGEWAY. WHERE MINIMUM COVER CAN NOT BE ACHIEVED, ALTERNATIVE BEDDING TYPE, BEDDING MATERIAL OR PIPE CLASS MAY NEED TO BE ADOPTED UPON THE INSTRUCTION OF THE ENGINEER.

									Design	JL		Approved For
ĺ	-	<u> </u>		-	-	-	1	Scale (A3) AS SHOWN	Drawn	AA	05.06.18	Construction*
ĺ	-	=	,	-	-	-			Dsg Verifier	RS	25.06.18	-
ĺ	0	FOR TENDER	JL	RS	LDB	14.06.19			Dwg Check	JL	07.06.18	Date -
1	No	Revision	Bv	Chk	Appd	Date	1		* Refer to Revision	1 for Original Signatur	re .	



SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER TYPICAL DETAILS SHEET 9 OF 10 FOR TENDER



VZTRANSPORT

→ AGENCY

Safe Roads

SCALE 1:100

STORMWATER TYPICAL DETAILS

SHEET 10 OF 10

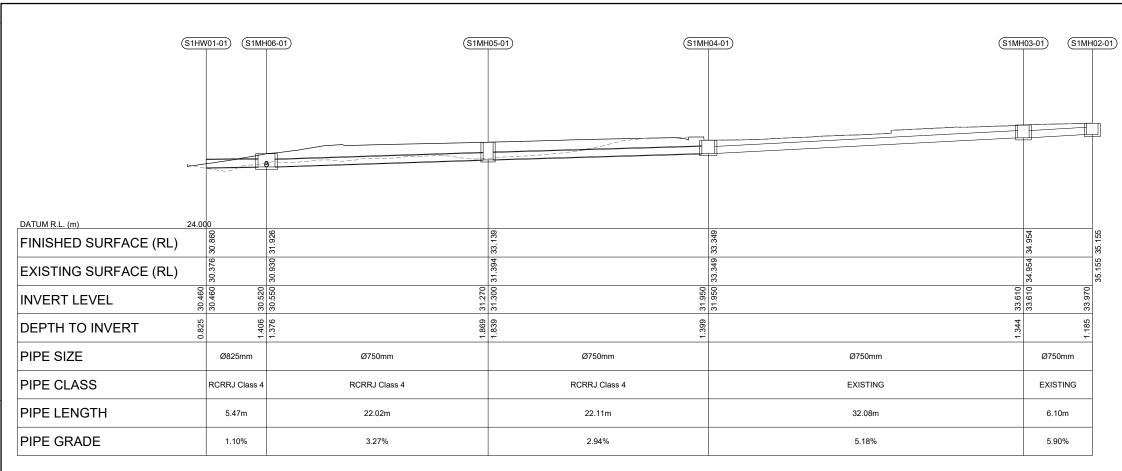
SH16 - SAFETY IMPROVEMENTS

STAGE 1

HUAPAI TO WAIMAUKU

By Chk Appd Date

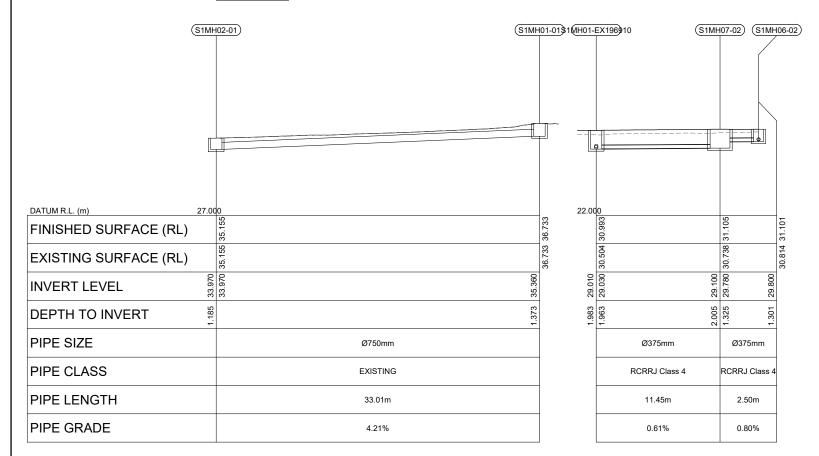
FOR TENDER



BEDDING NOTE:

 ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.

SW LINE 01



SW LINE 01 SW LINE 02

						Original	Design	JL	25.06.18	Approved For	
						Scale (A3) 1:400 (H)	Drawn	KA	05.06.18	Construction*	
-	-	-	-	-	-	1:200 (V)	Dsg Verifier	RS	25.06.18	i -	
0	FOR TENDER	JL	RS	LDB	14.06.19		Dwg Check	JL	07.06.18	Date -	
No.	Revision	By	Chk	Appd	Date		* Refer to Revision	* Refer to Revision 1 for Original Signature			



SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU

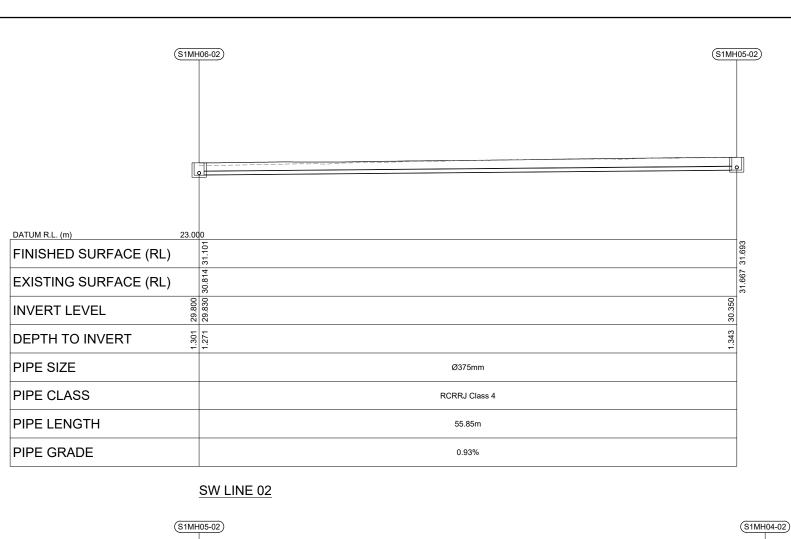
STORMWATER LONGITUDINAL SECTIONS SHEET 1 OF 30 FOR TENDER

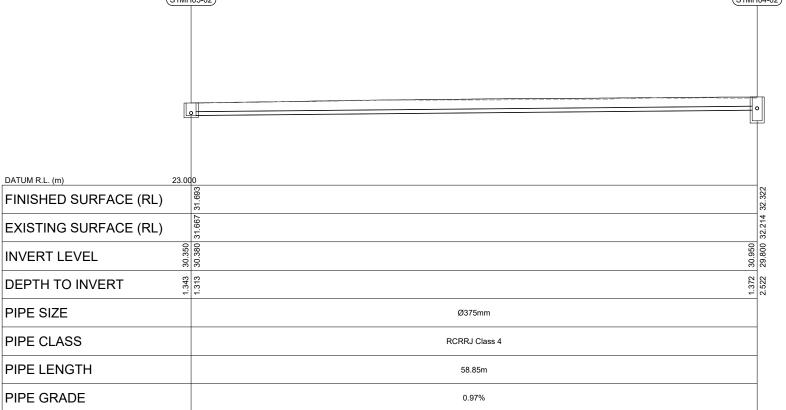
sering No.

SR1003 - 01 - VE - 2151

ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.

BEDDING NOTE:





SW LINE 02

						_						_
							Original	Design	JL		Approved For	П
						1	Scale (A3) 1:400 (H)	Drawn	KA	05.06.18	Construction*	П
-	-	-	-	-	-			Dsg Verifier	RS	25.06.18	-	
0	FOR TENDER	JL	RS	LDB	14.06.19		, ,	Dwg Check	JL	07.06.18	Date -	П
No.	Revision	By	Chk	Appd	Date	1		* Refer to Revision 1 for Original Signature				

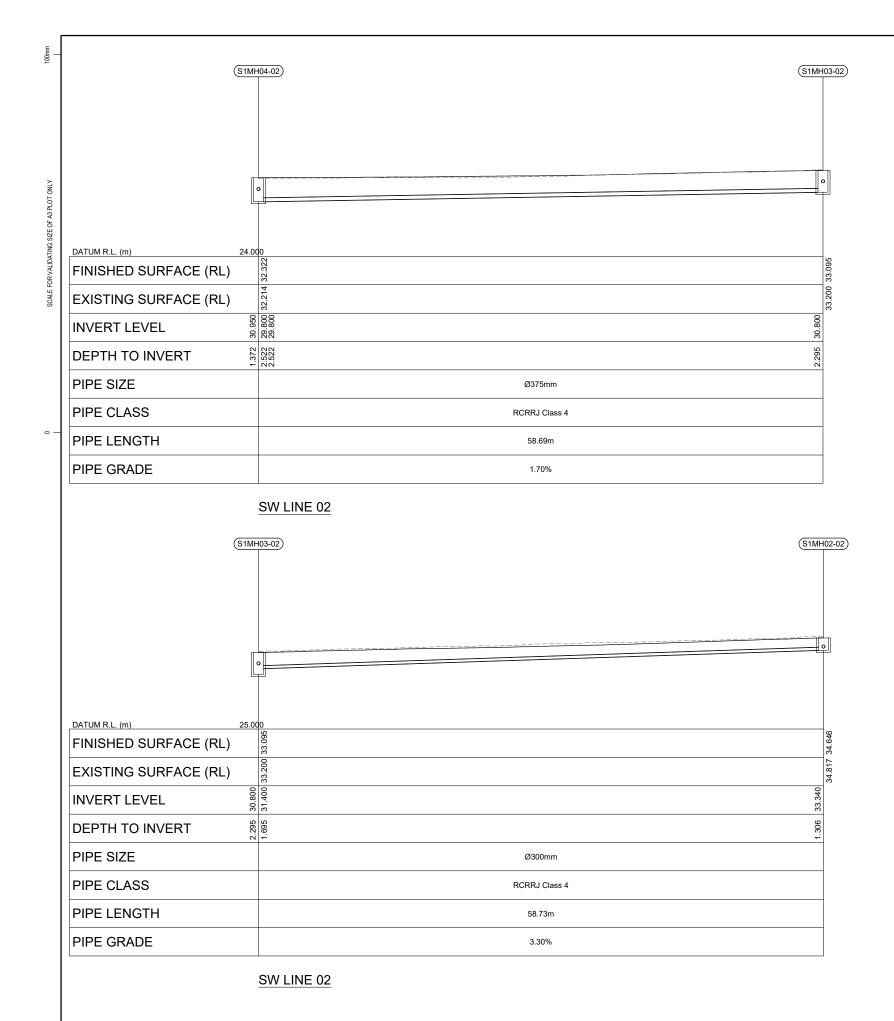


Safe Roads

SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER LONGITUDINAL SECTIONS SHEET 2 OF 30 FOR TENDER

rawing No.

SR1003 - 01 - VE - 2152



BEDDING NOTE:

 ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.

| Criginal Scale (R-CT) | Crig

| JL | 25.06.18 | Approved For | Construction* | KA | 05.06.18 | Construction* | KB | 25.06.18 | Date - | Construction* | Cons

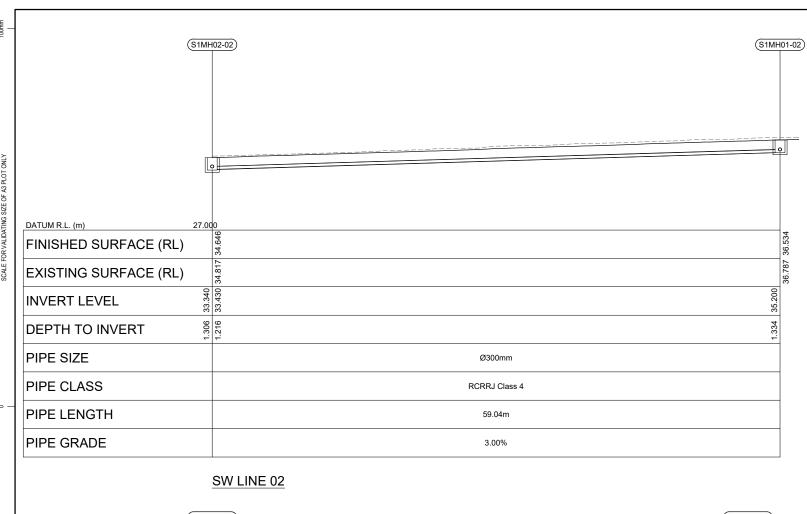


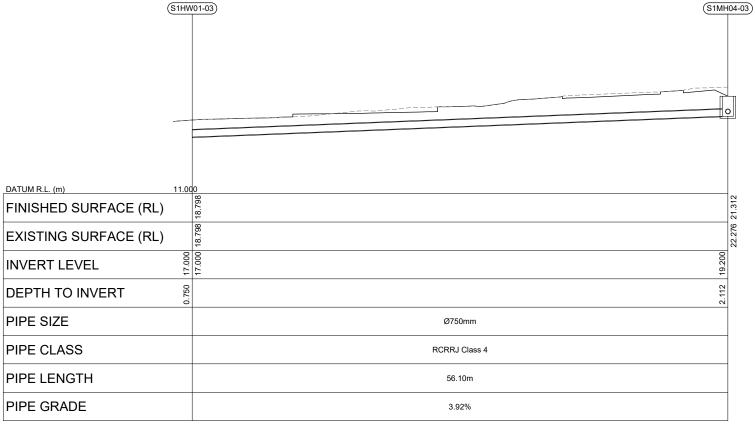


SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER LONGITUDINAL SECTIONS SHEET 3 OF 30

FOR TENDER

SR1003 - 01 - VE - 2153





SW LINE 03

						Original	Design	JL
						Scale (A3) 1:400 (H)	Drawn	KA
-	=	,	-	-	-	1:200 (V)	Dsg Verifier	RS
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No.	Revision	By	Chk	Appd	Date		* Refer to Revision	n 1 for Oric

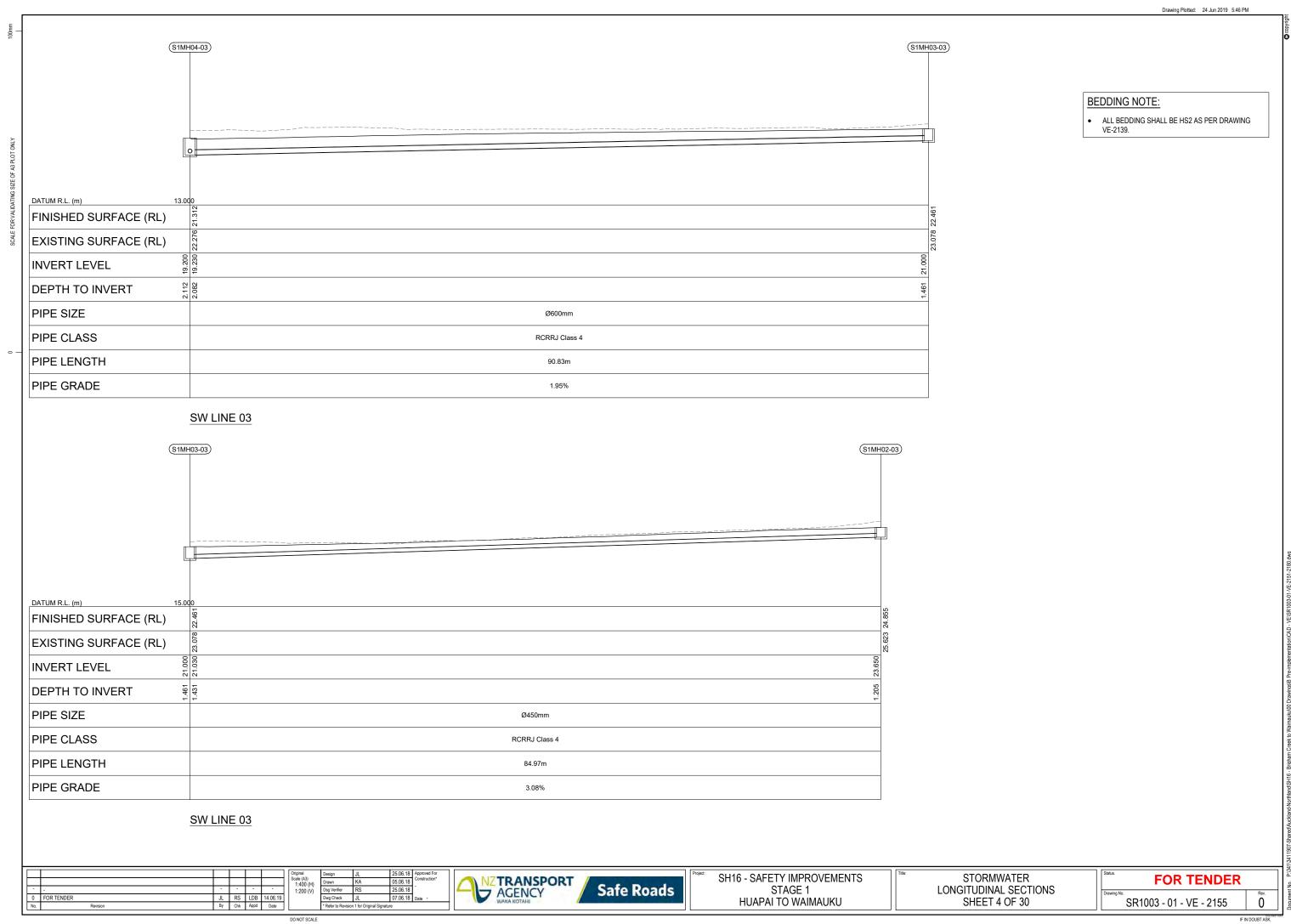


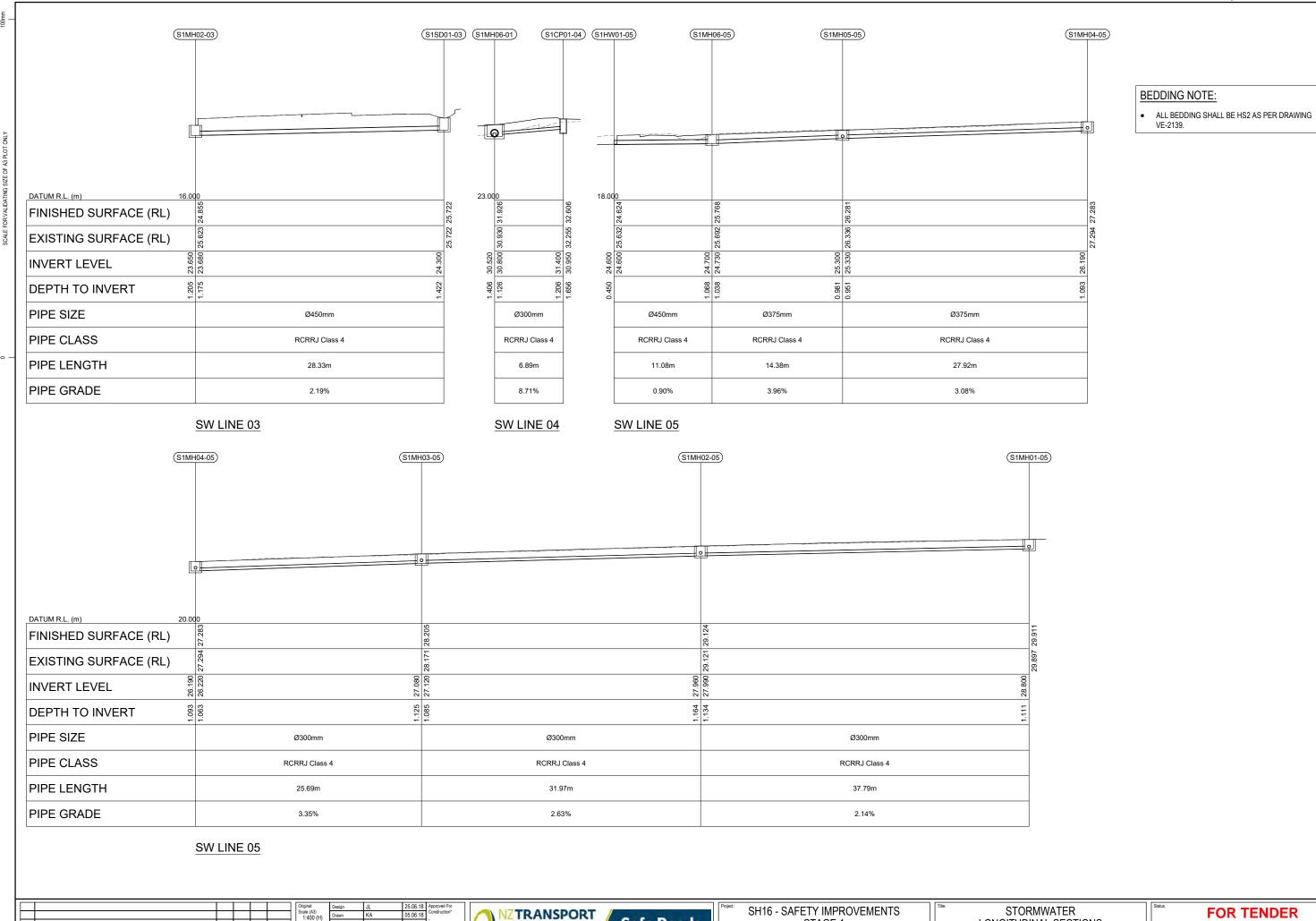


SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER LONGITUDINAL SECTIONS SHEET 4 OF 30 FOR TENDER

 ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.

BEDDING NOTE:





Safe Roads

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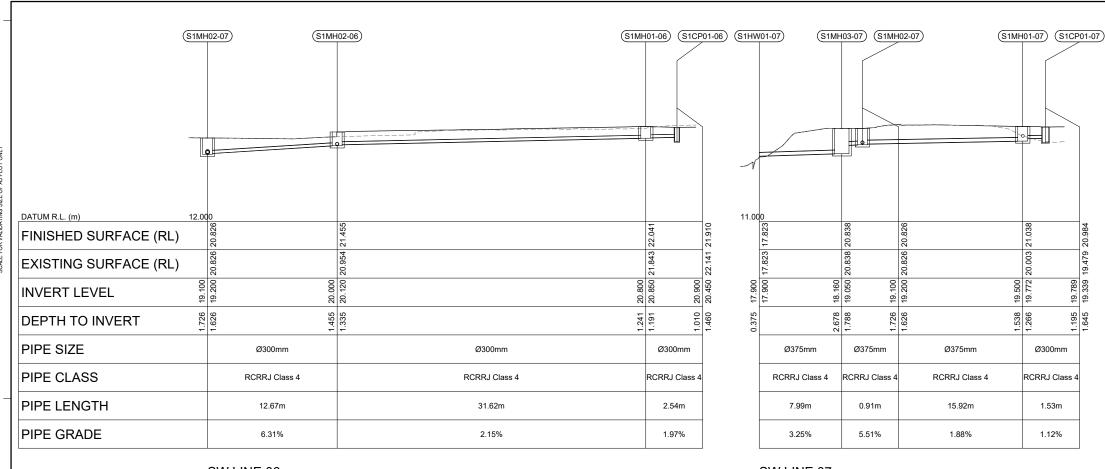
STAGE 1

HUAPAI TO WAIMAUKU

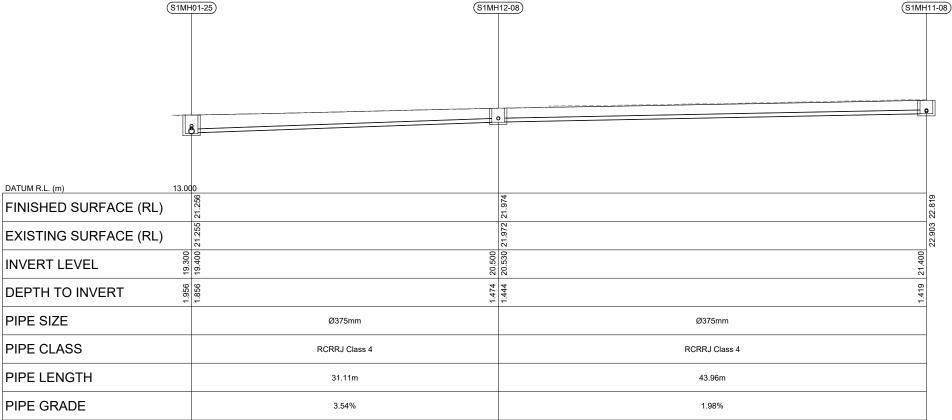
SR1003 - 01 - VE - 2156

LONGITUDINAL SECTIONS

SHEET 6 OF 30



SW LINE 06 SW LINE 07



BEDDING NOTE:

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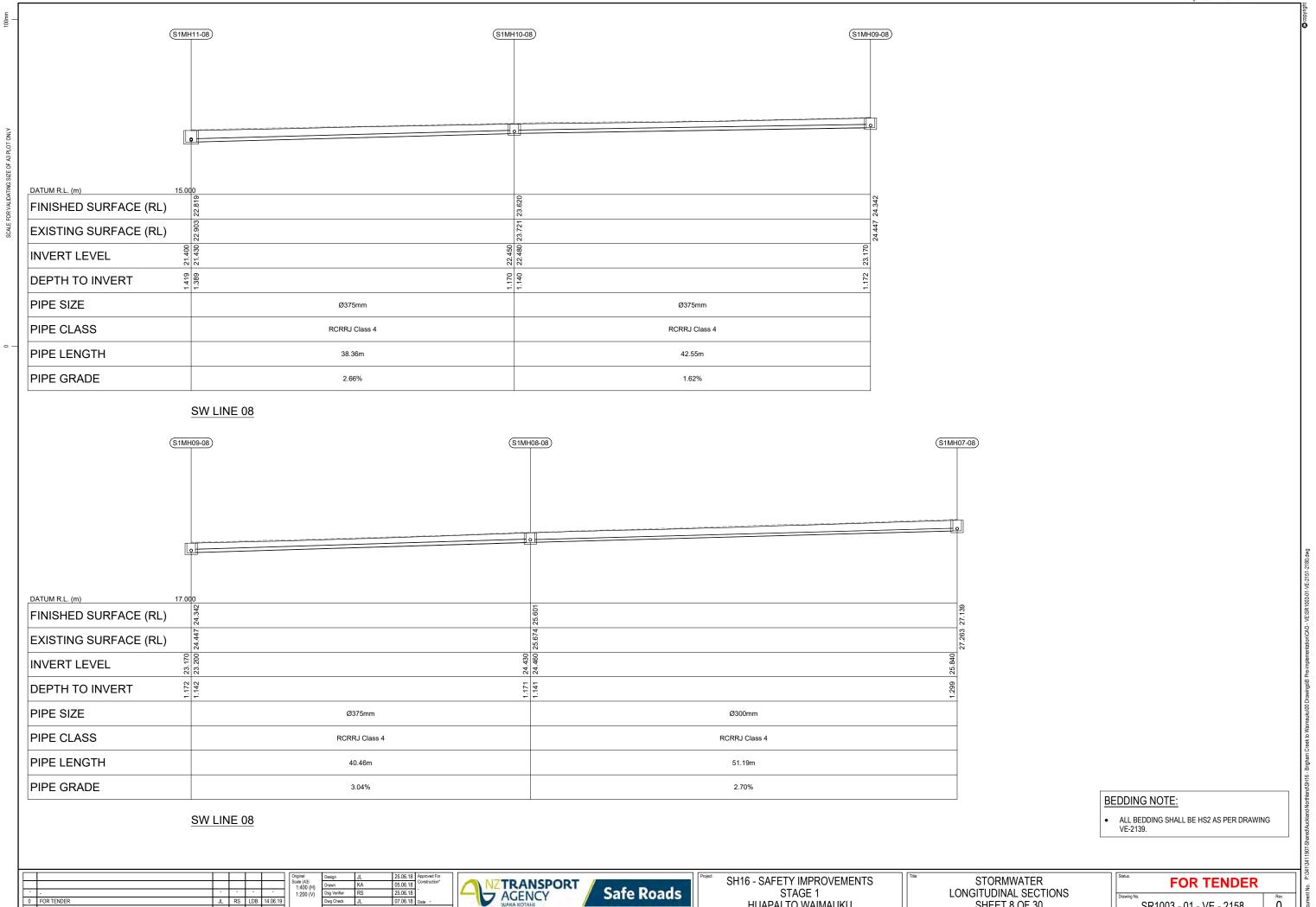
ı							Original	Design	Ŀ
ı							Scale (A3) 1:400 (H)	Drawn	ŀ
ı	-	-	-	-	-	-	1:200 (V)	Dsg Verifier	F
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1	Ma	Devision	Rv	Chl	Annd	Data		* Defer to Devision	4

SW LINE 08

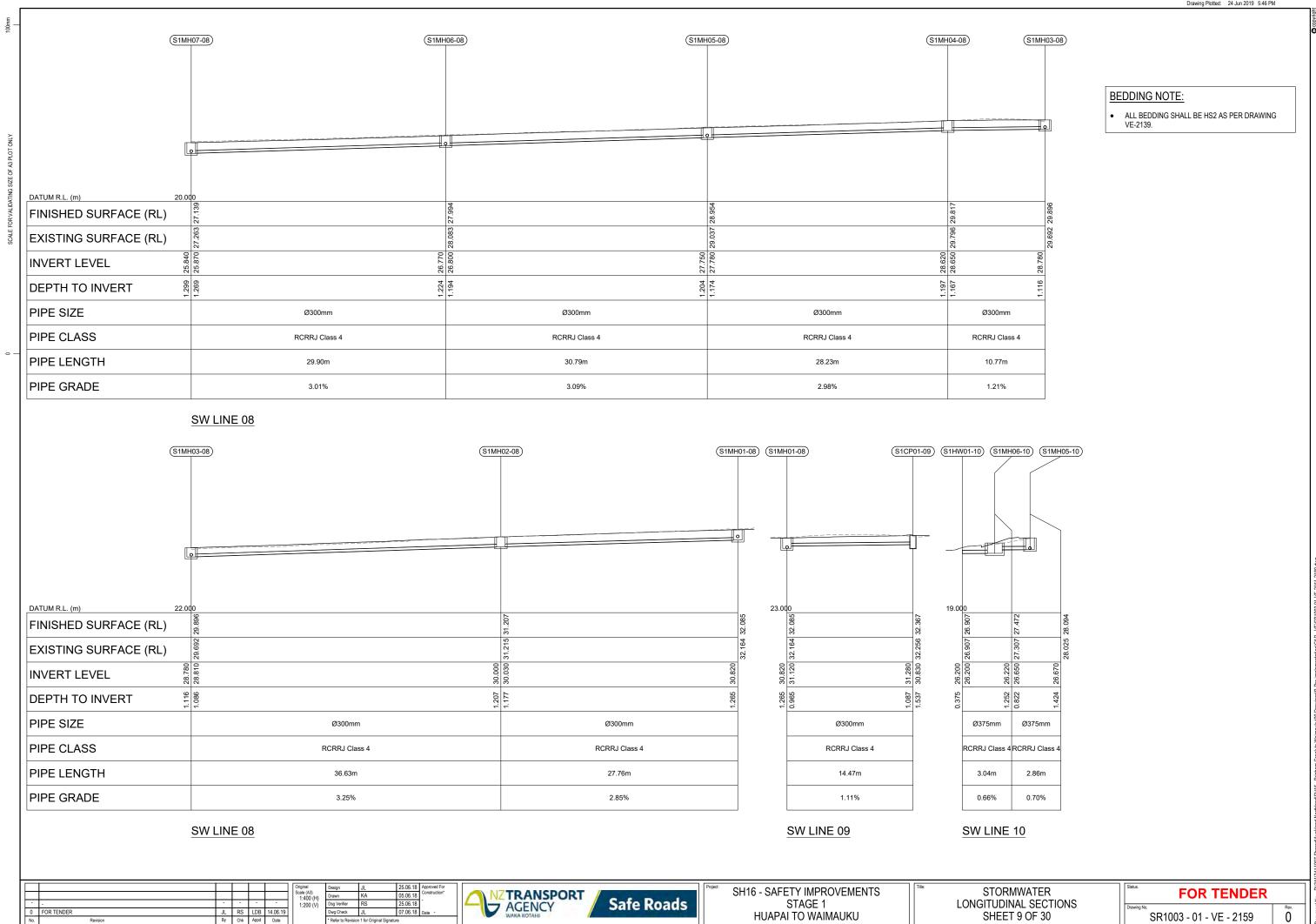


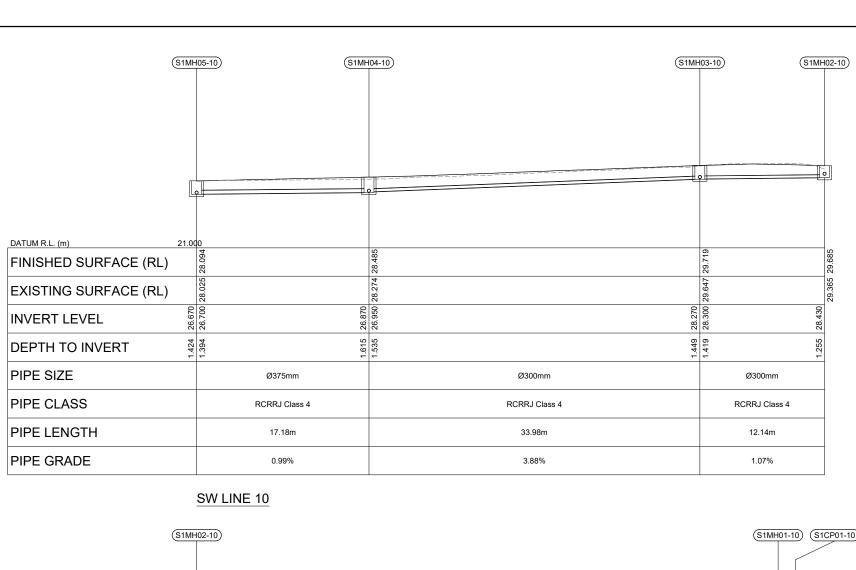


SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER LONGITUDINAL SECTIONS SHEET 7 OF 30 FOR TENDER



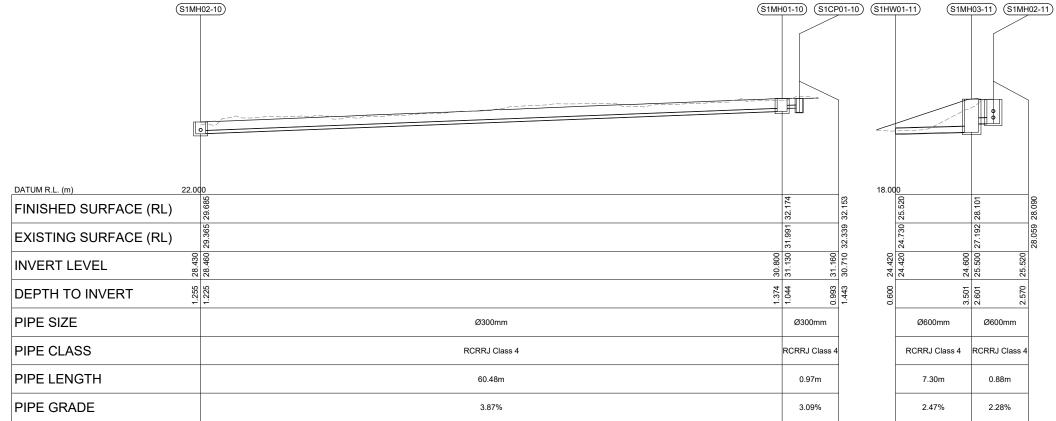
SHEET 8 OF 30





BEDDING NOTE:

 ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.



SW LINE 10 SW LINE 11

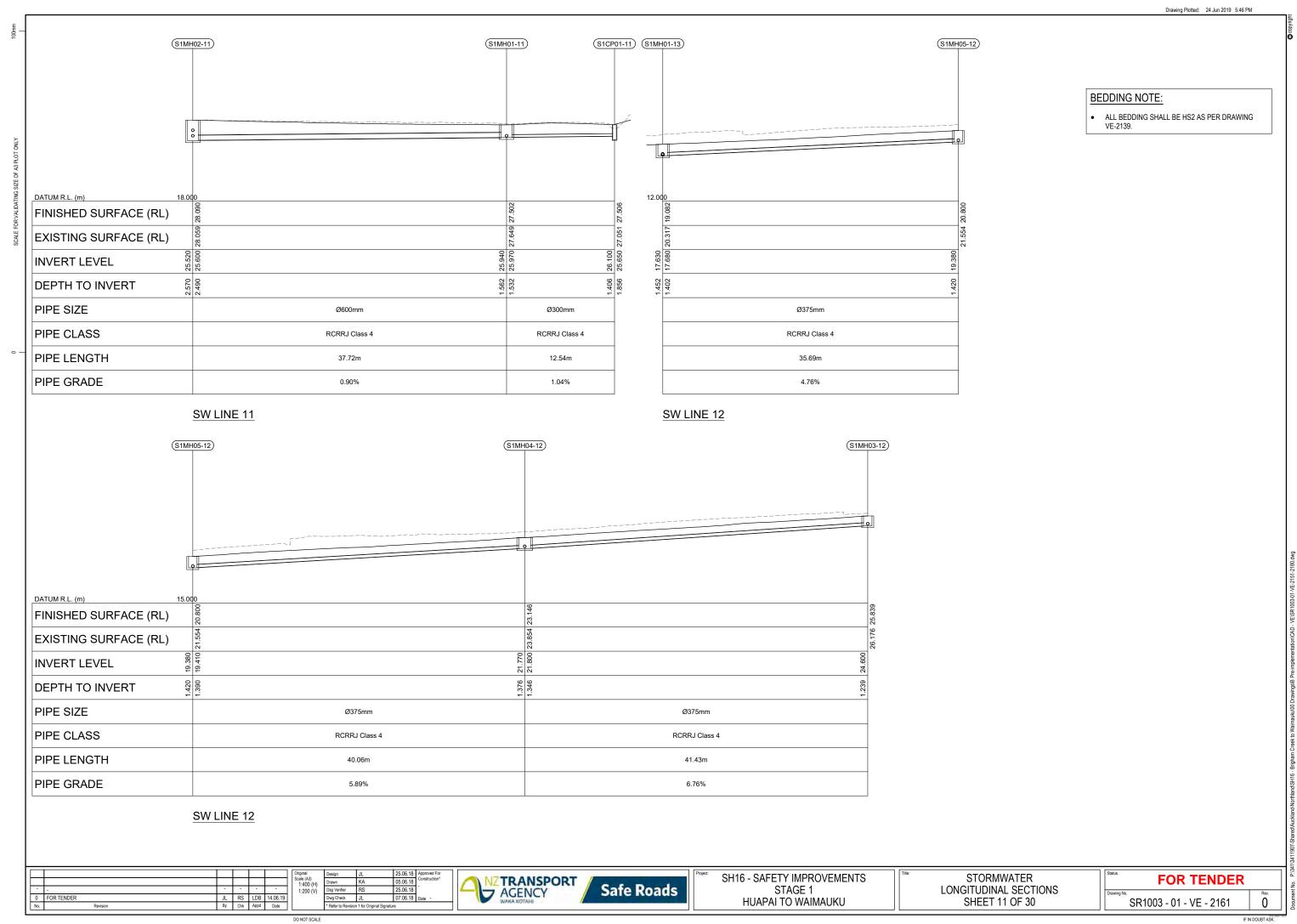
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-	-	,	-	-	-		Dsg Verifier	RS	25.06.18	-		
0	FOR TENDER	JL	RS	LDB	14.06.19	` '	Dwg Check	JL	07.06.18	Date -		
No.	Revision	By	Chk	Appd	Date		* Refer to Revision 1 for Original Signature					

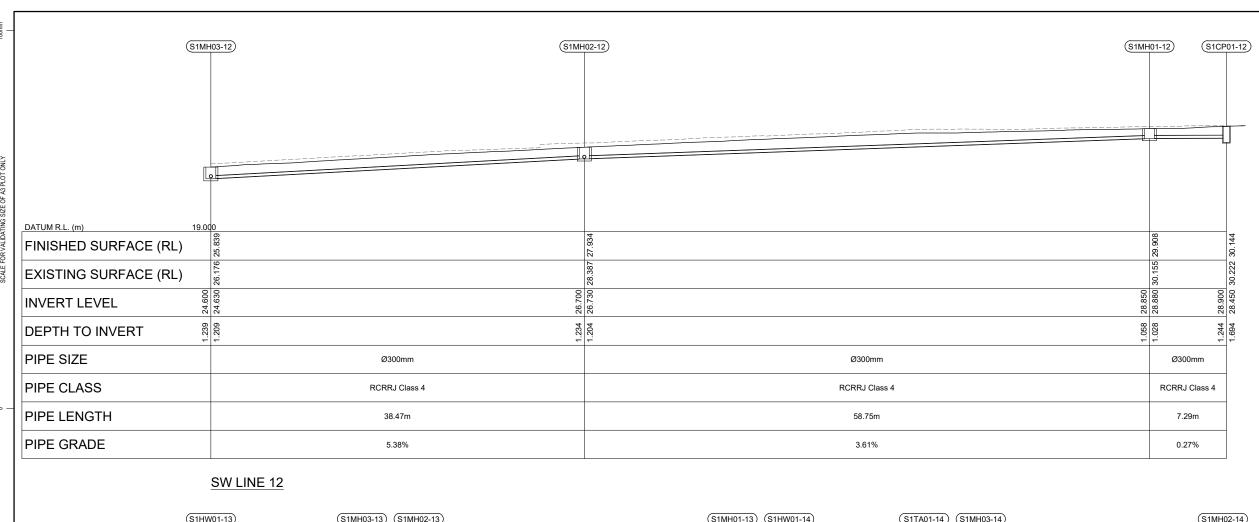


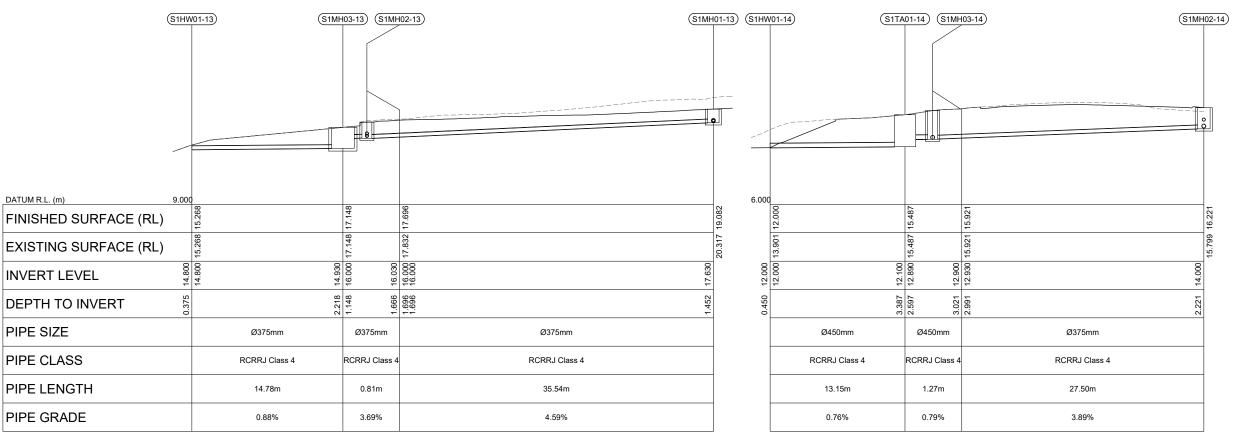
SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU

STORMWATER LONGITUDINAL SECTIONS SHEET 10 OF 30 FOR TENDER

SR1003 - 01 - VE - 2160







SW LINE 13

BEDDING NOTE:

 ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.

ı								Original
ı							3	Scale (A3) 1:400 (
ı	-	-	,	-	-	-		1:200 (
ı	0	FOR TENDER	JL	RS	LDB	14.06.19		•
ı	No	Revision	Rv	Chk	Annd	Date		

L	25.06.18	Approved For	П	
A.	05.06.18	Construction*	Н	1
IS .	25.06.18	-	П	L
L	07.06.18	Date -	П	
for Original Signature				

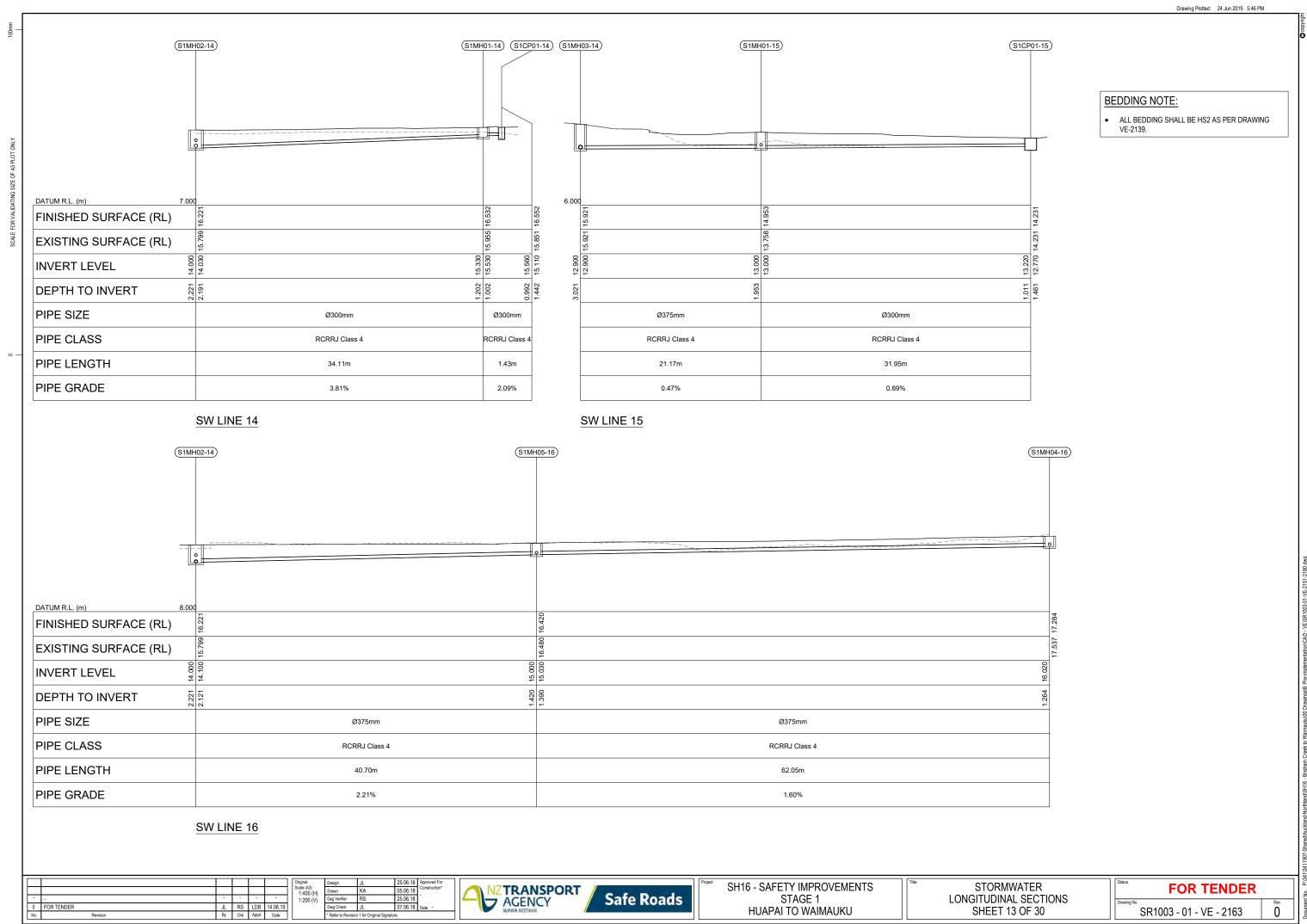


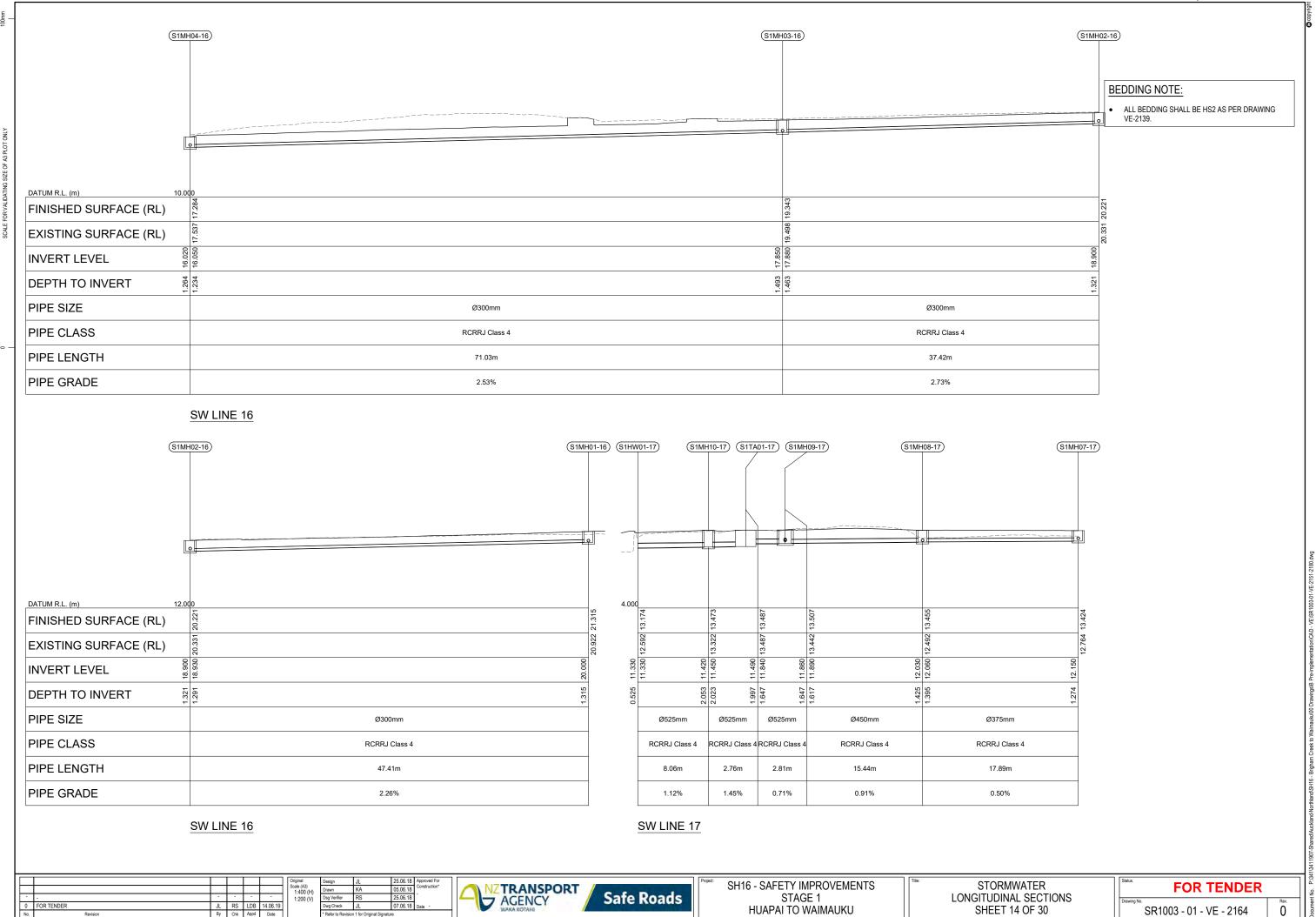


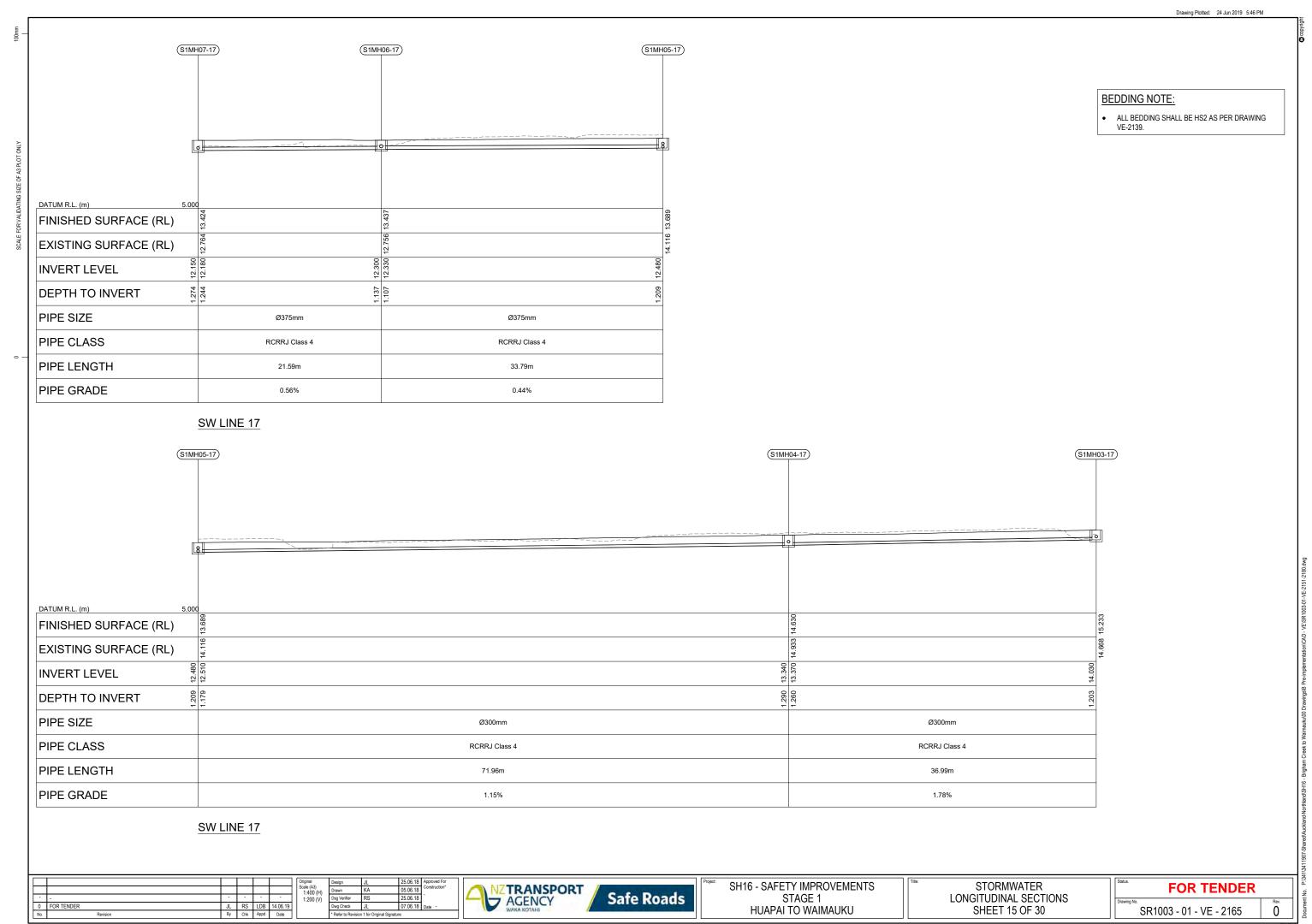
SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU

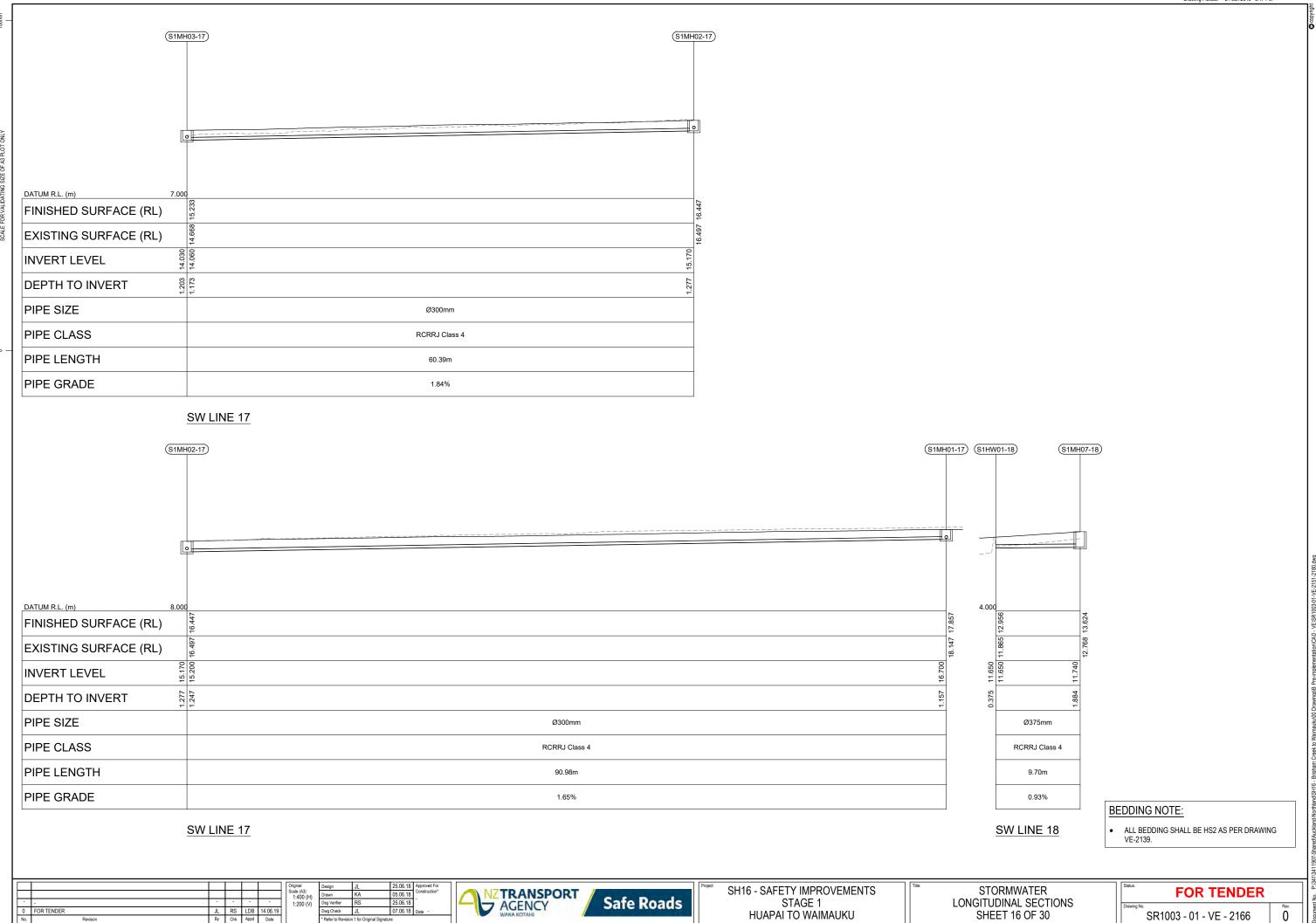
STORMWATER LONGITUDINAL SECTIONS SHEET 12 OF 30 FOR TENDER

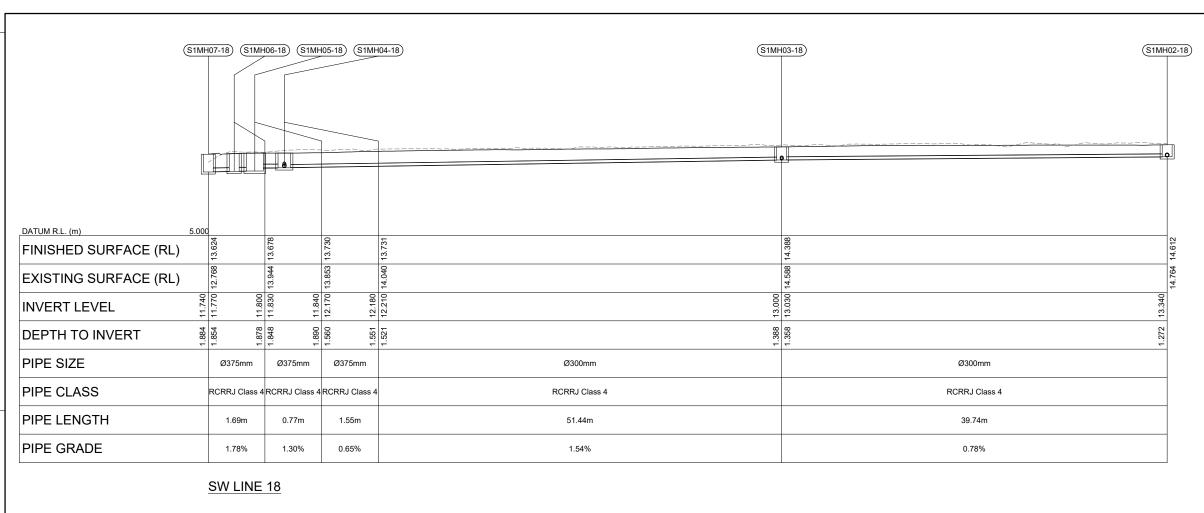
SR1003 - 01 - VE - 2162

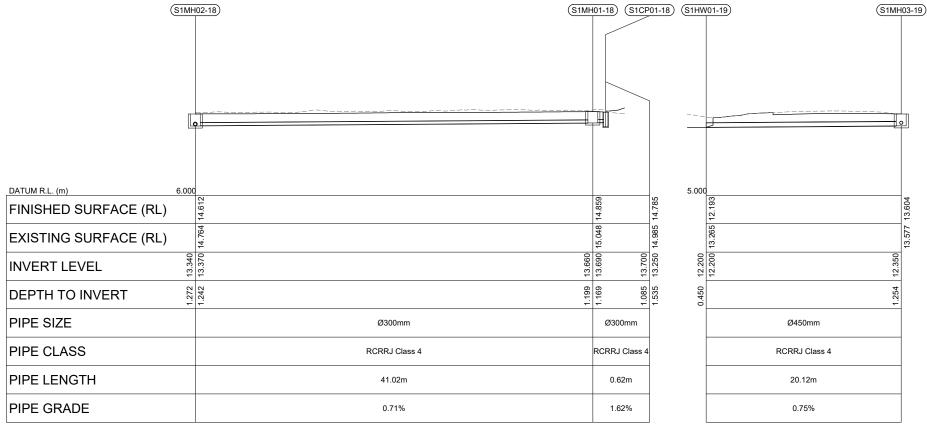












SW LINE 18

BEDDING NOTE:

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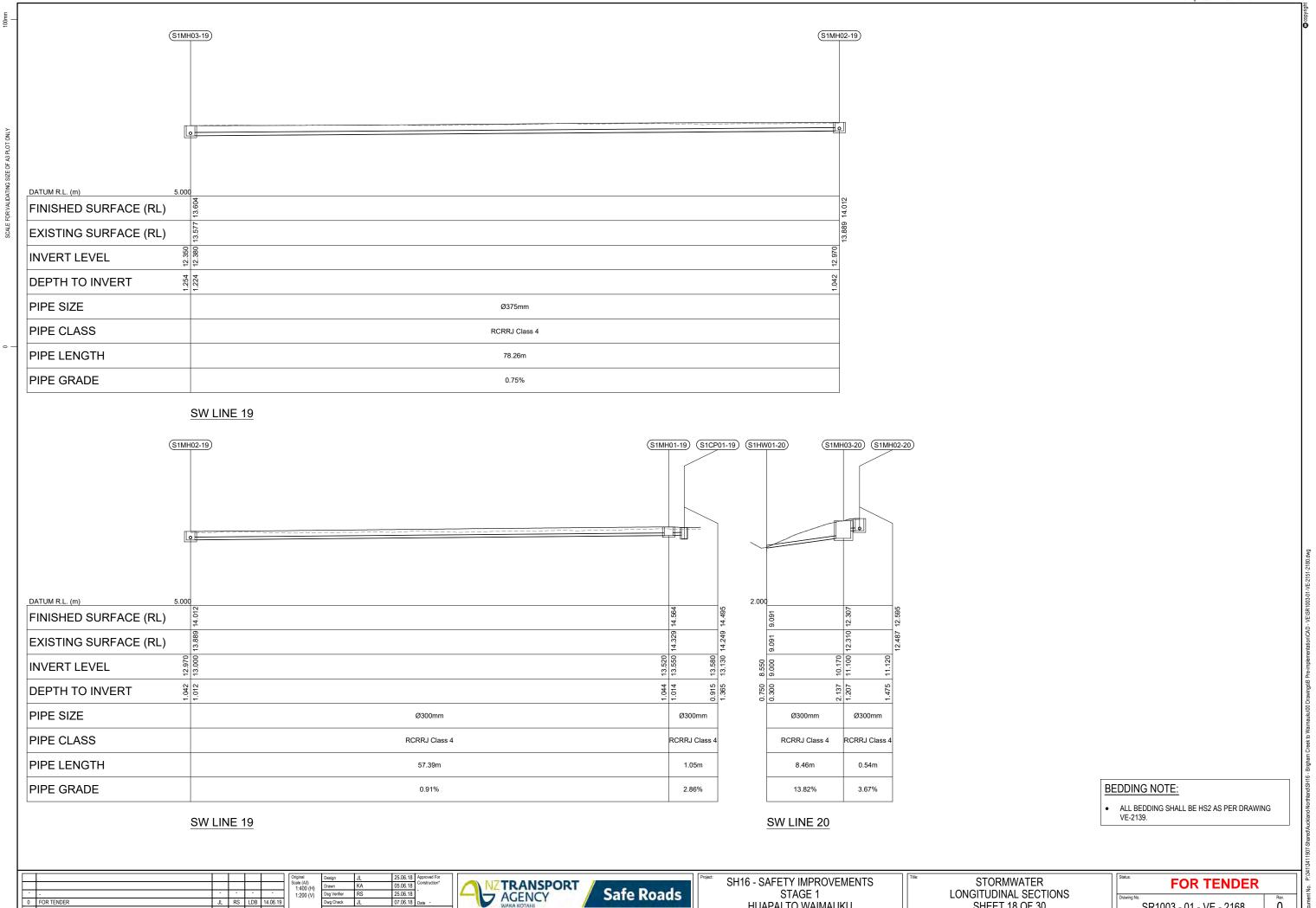
						Original Scale (A3)
						1:400 (H
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0	FOR TENDER	JL	RS	LDB	14.06.19	,
No.	Revision	Ву	Chk	Appd	Date	

NZ TRANSPORT AGENCY
Wild Rolling



SW LINE 19

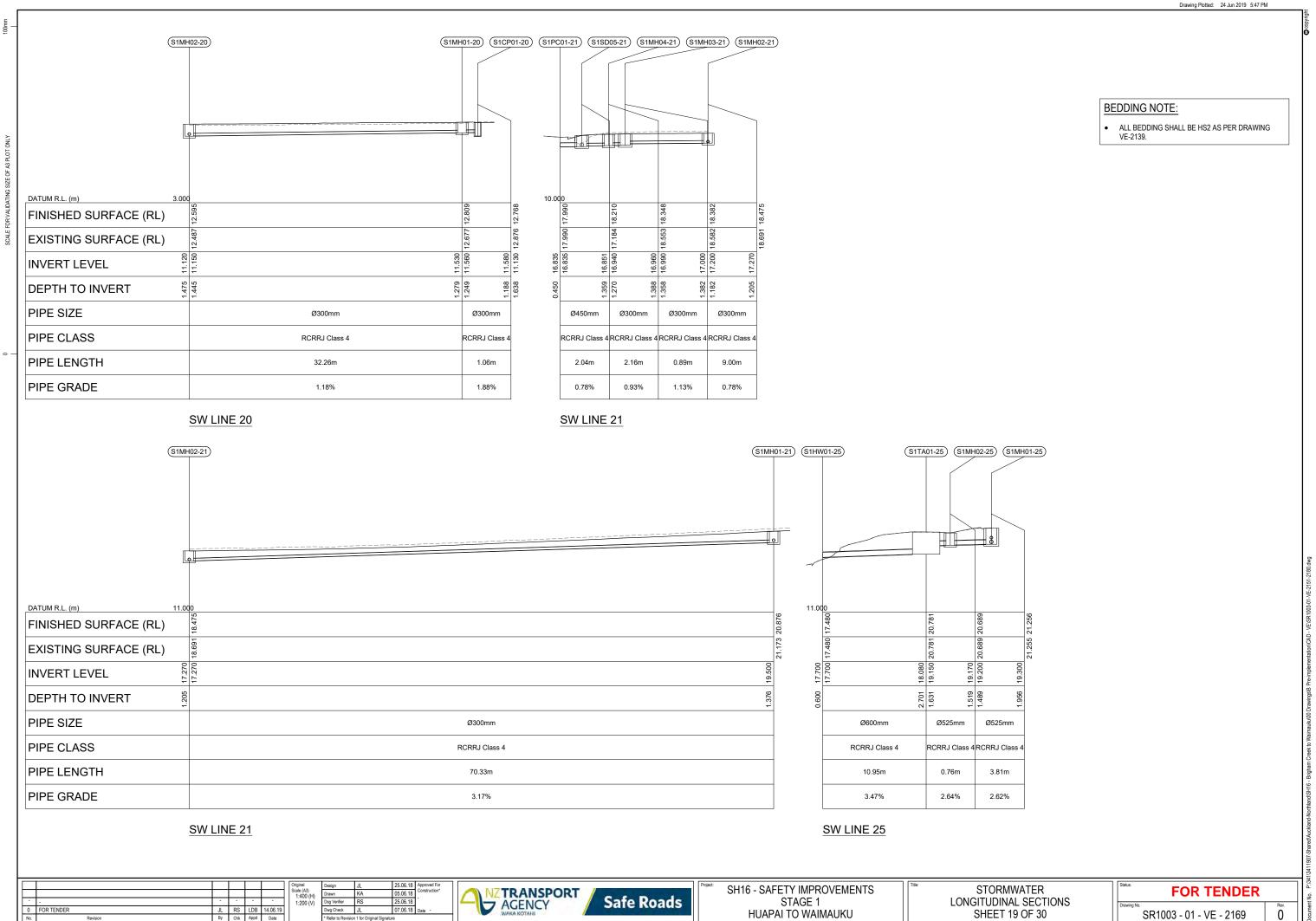
SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER LONGITUDINAL SECTIONS SHEET 17 OF 30 FOR TENDER

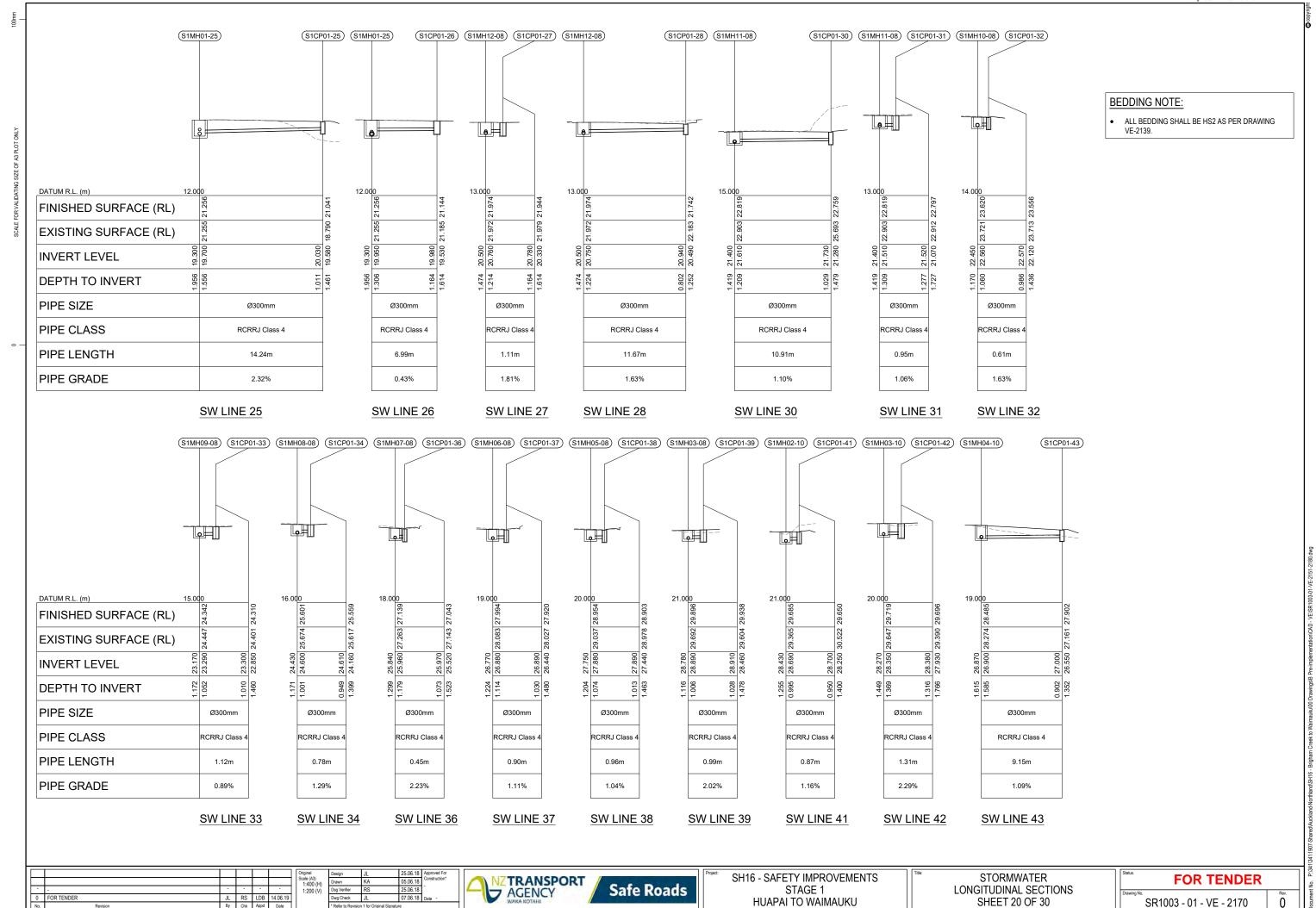


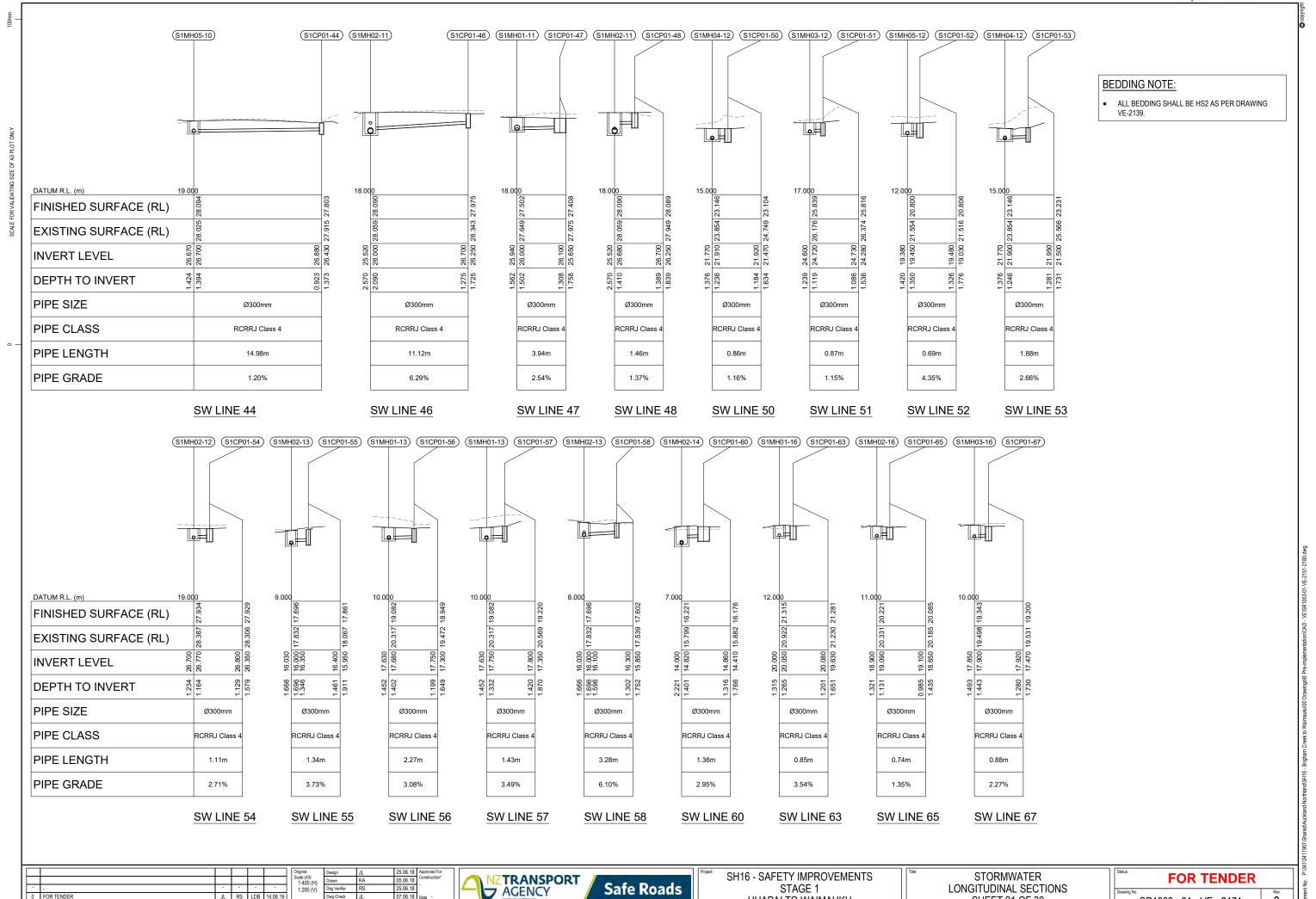
SR1003 - 01 - VE - 2168

SHEET 18 OF 30

HUAPAI TO WAIMAUKU



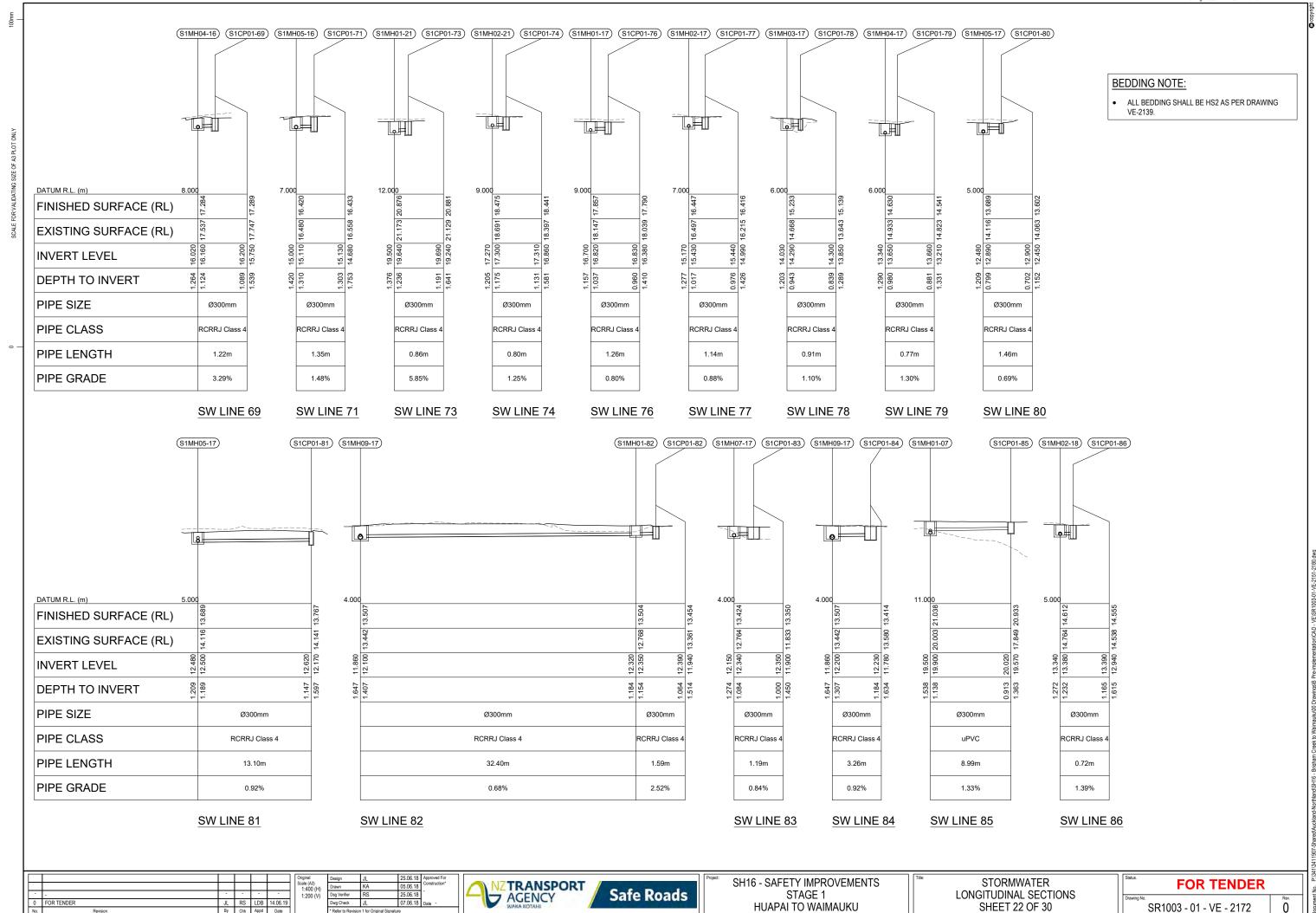




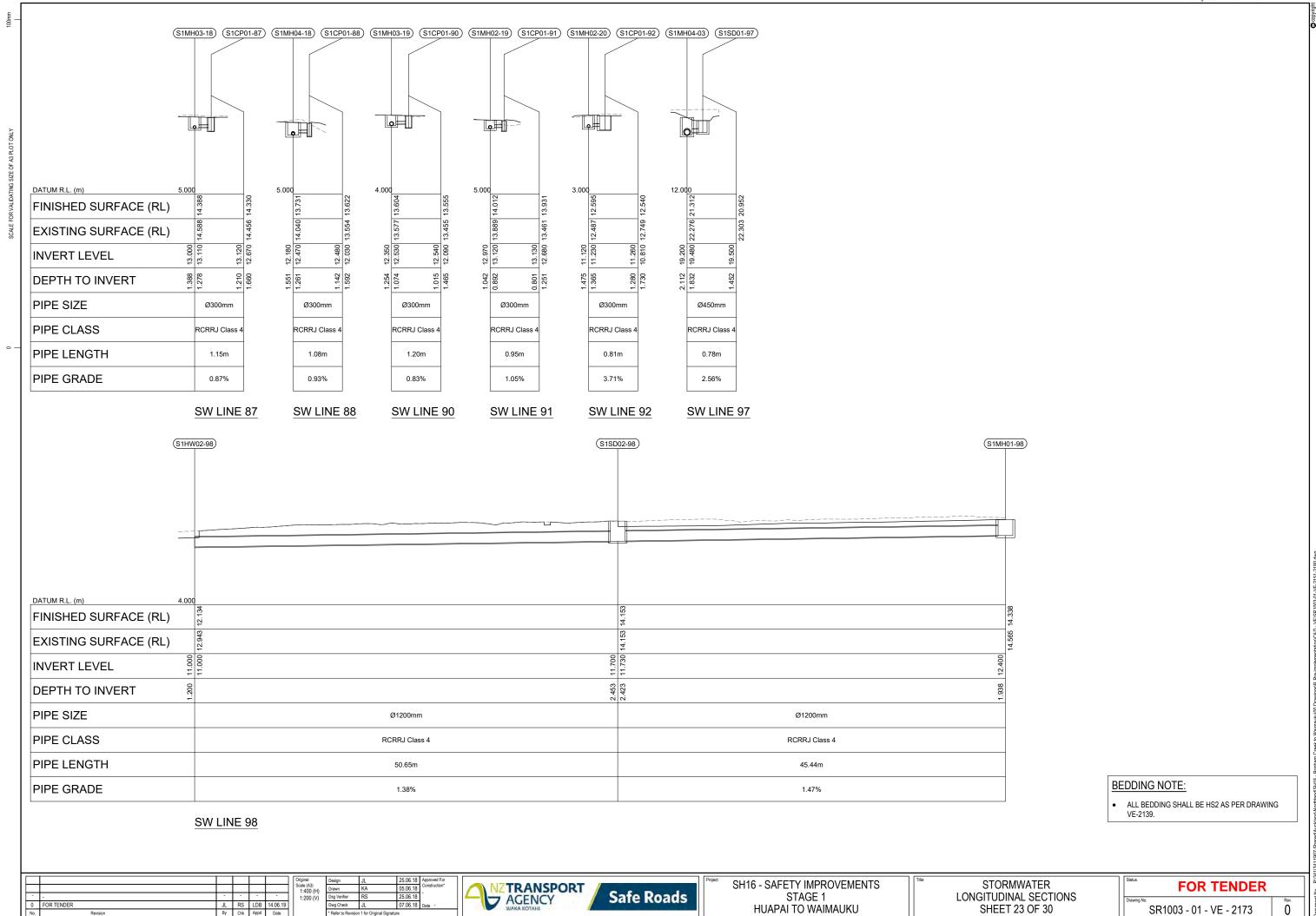
HUAPAI TO WAIMAUKU

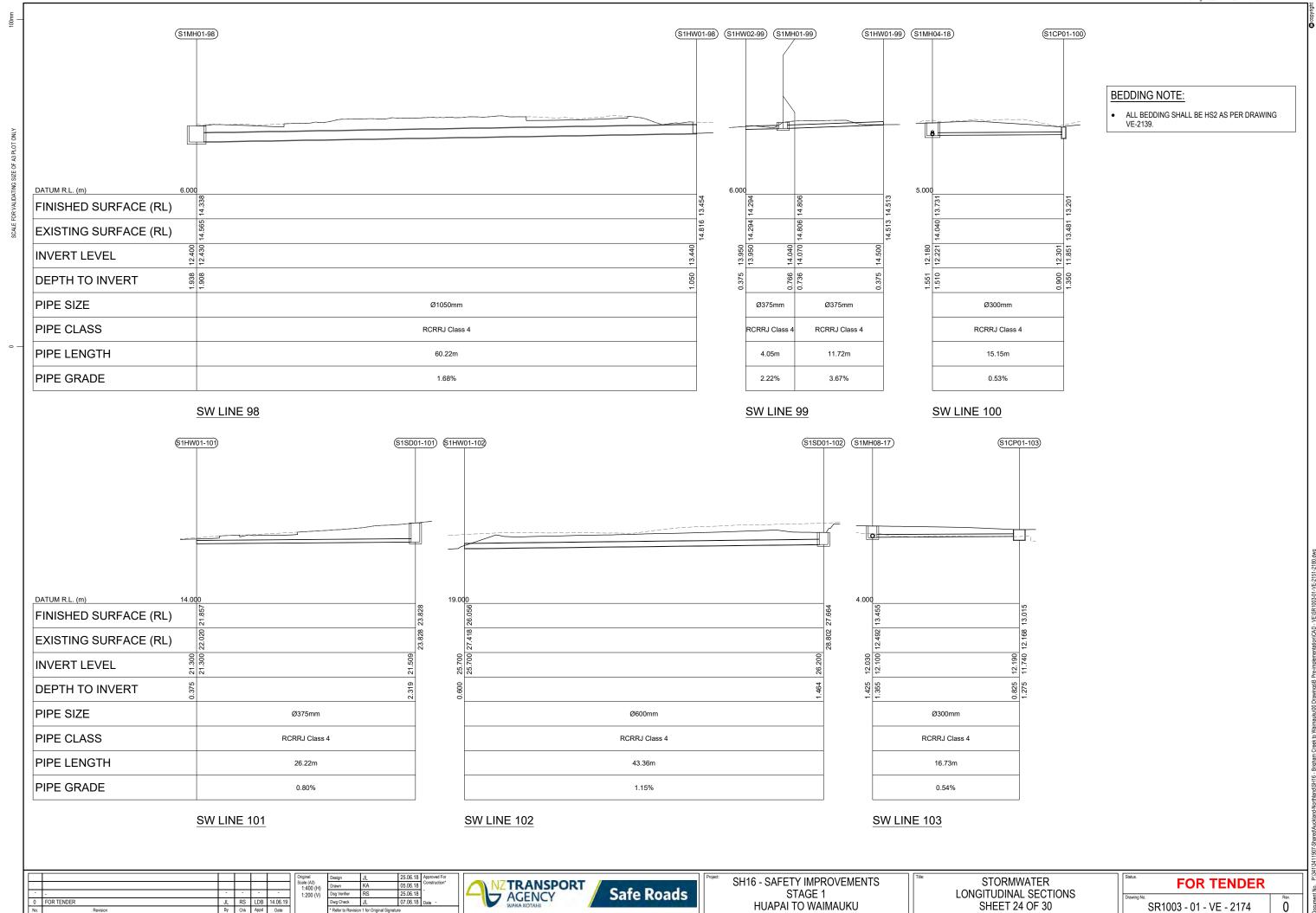
SHEET 21 OF 30

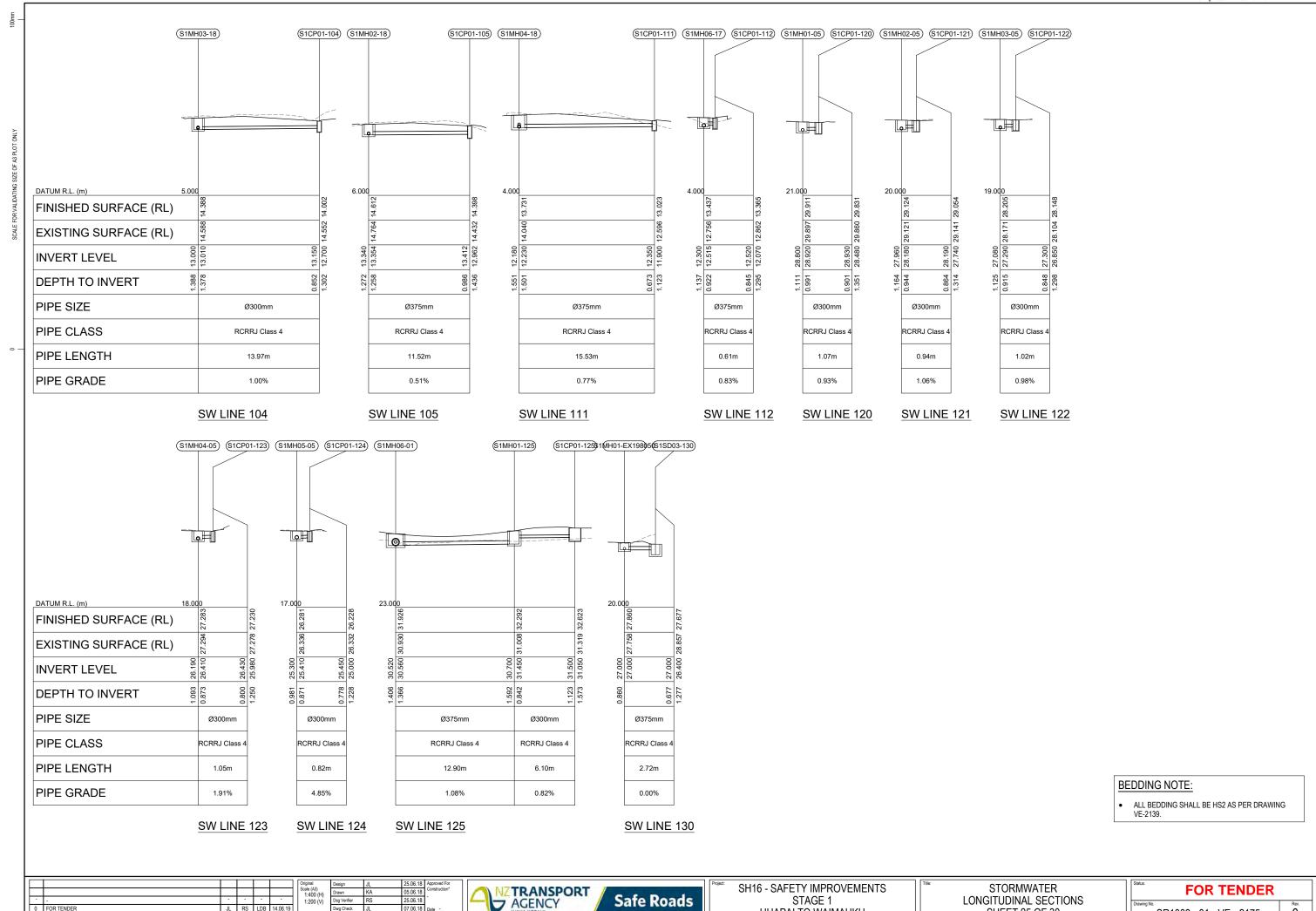
SR1003 - 01 - VE - 2171



By Chk Appd Date



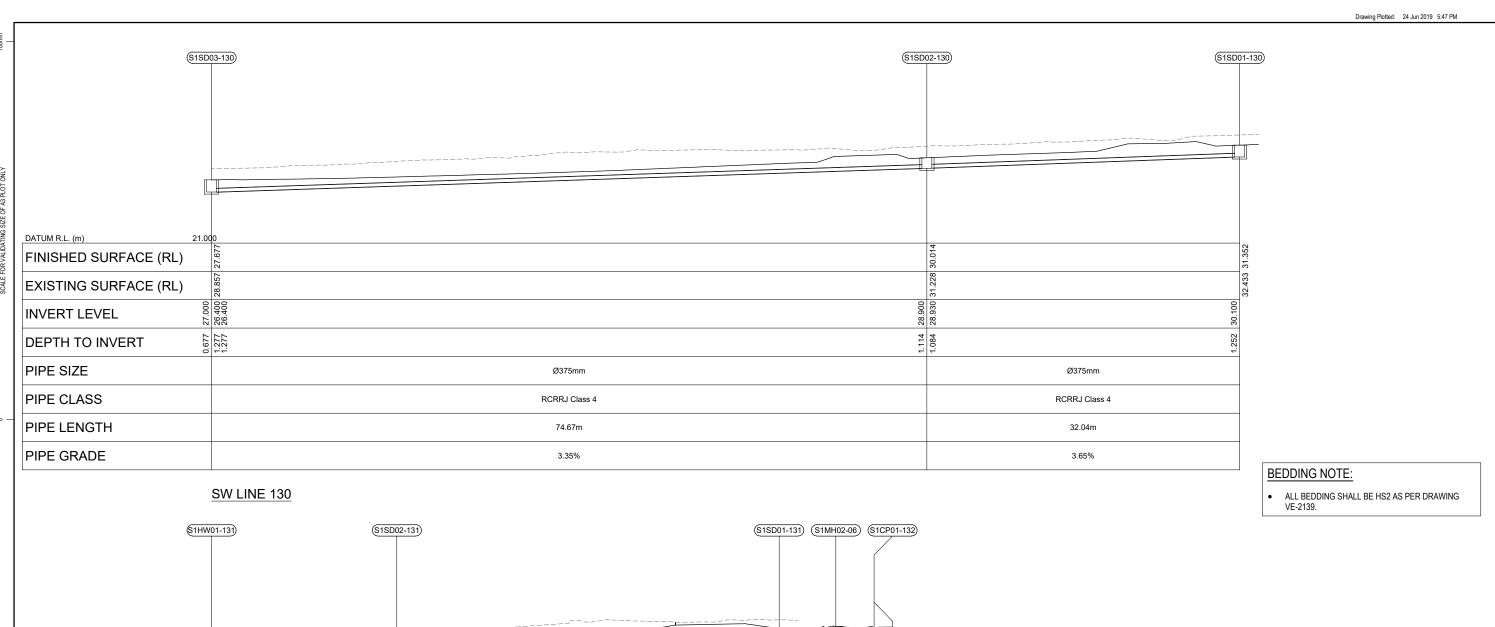


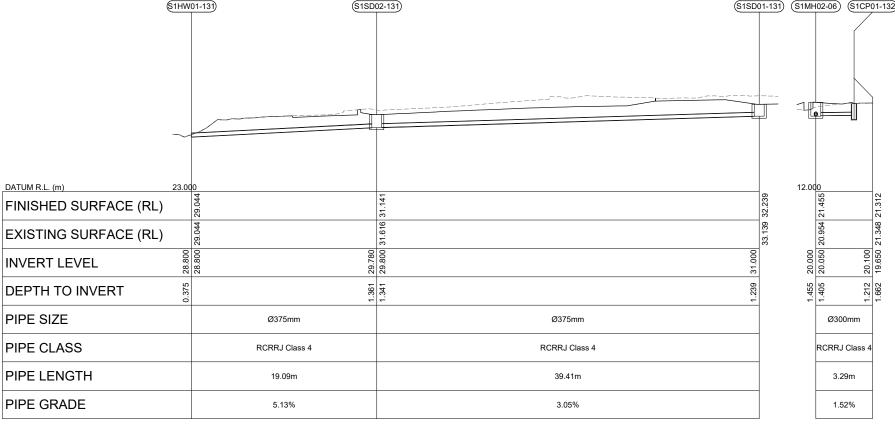


HUAPAI TO WAIMAUKU

SHEET 25 OF 30

SR1003 - 01 - VE - 2175





SW LINE 132

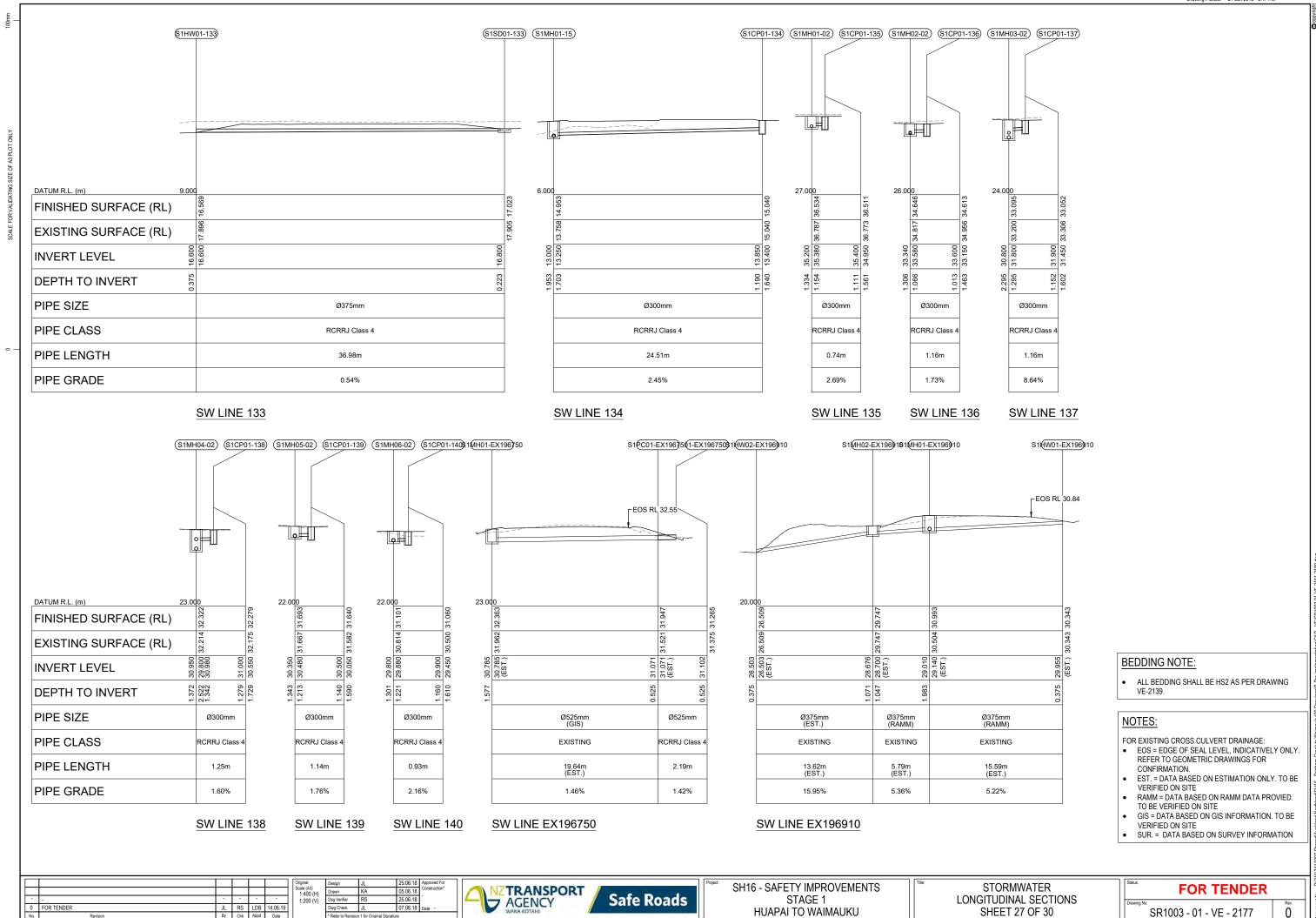
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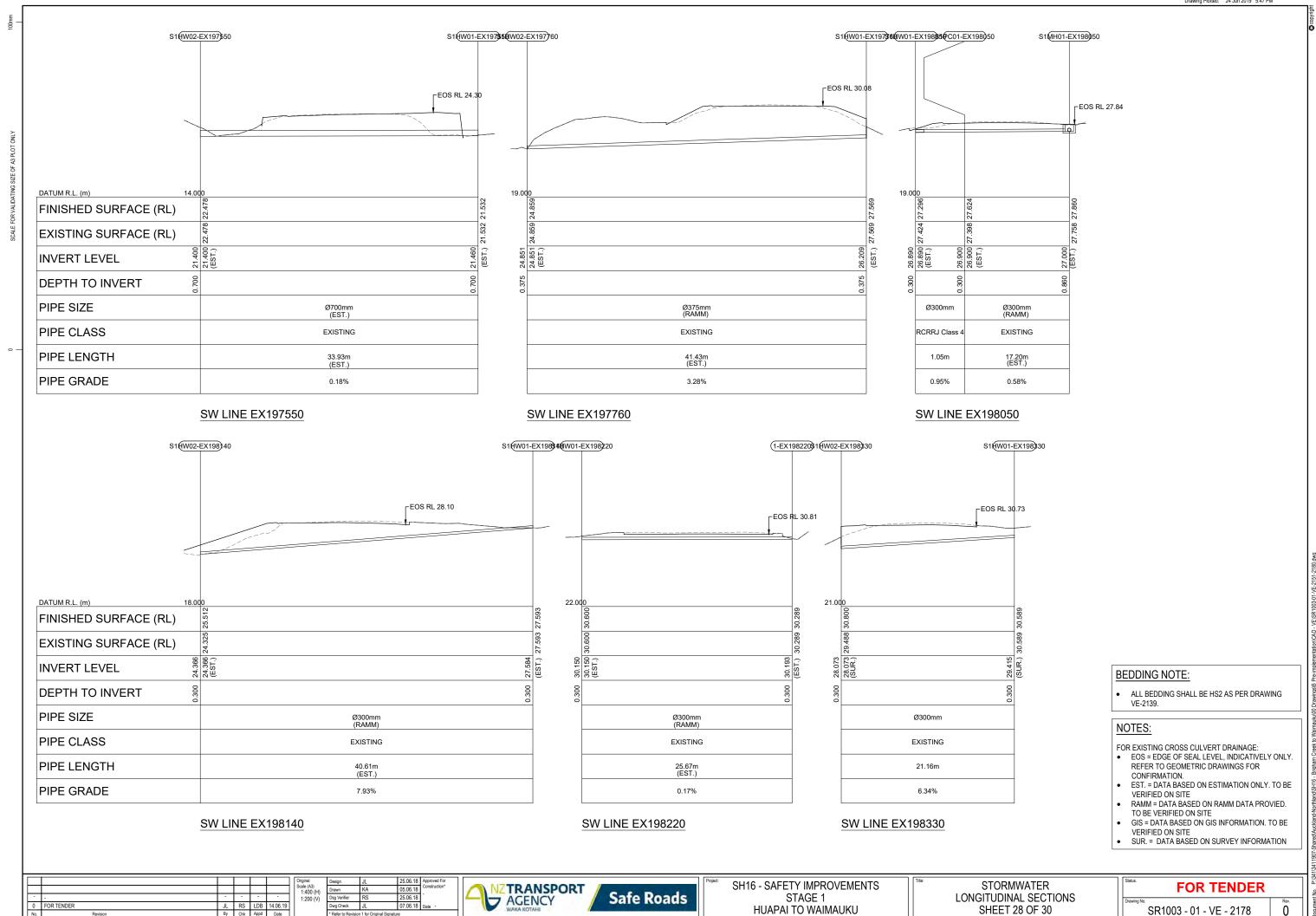


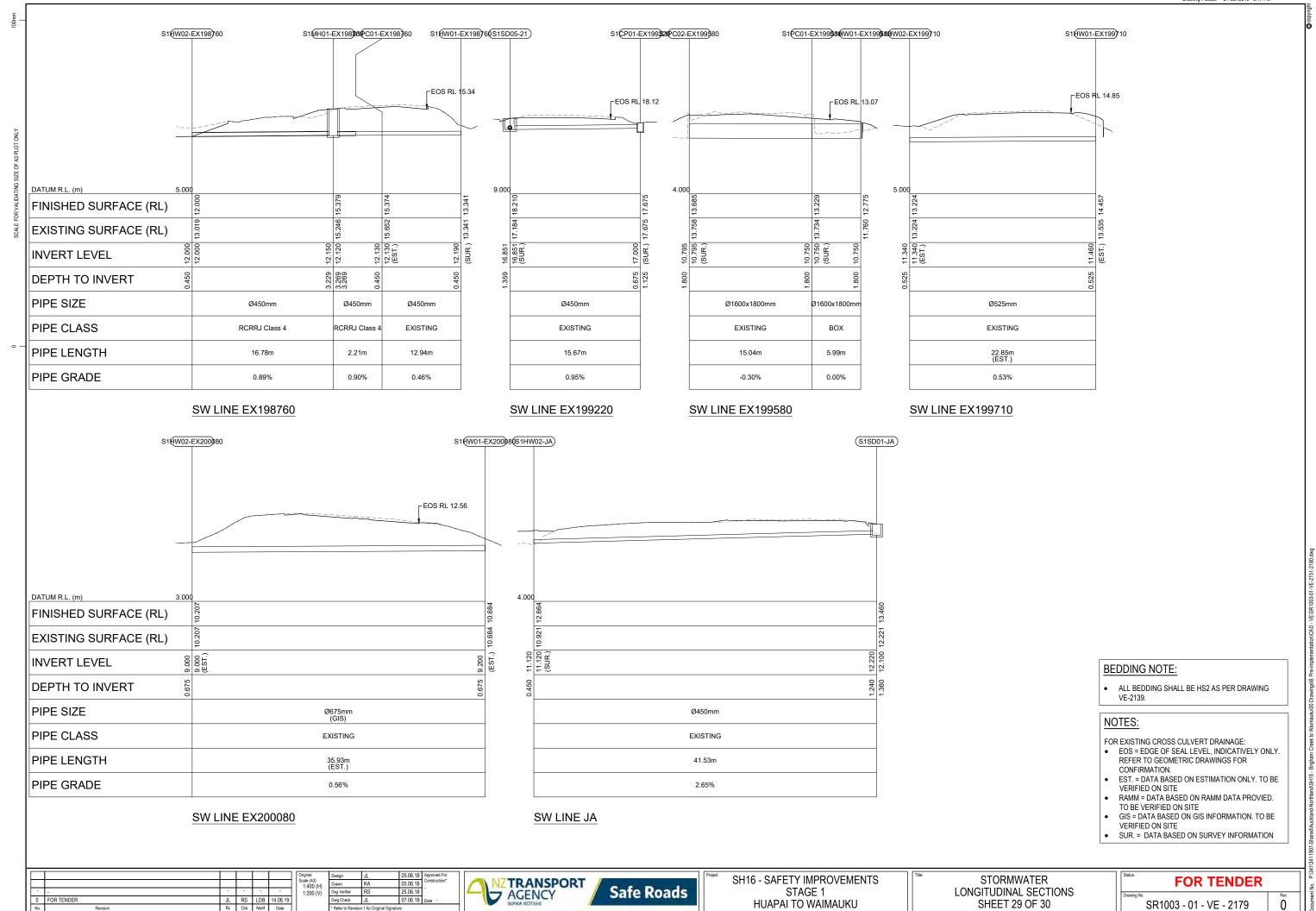


SH16 - SAFETY IMPROVEMENTS STAGE 1 HUAPAI TO WAIMAUKU STORMWATER LONGITUDINAL SECTIONS SHEET 26 OF 30 FOR TENDER

SR1003 - 01 - VE - 2176

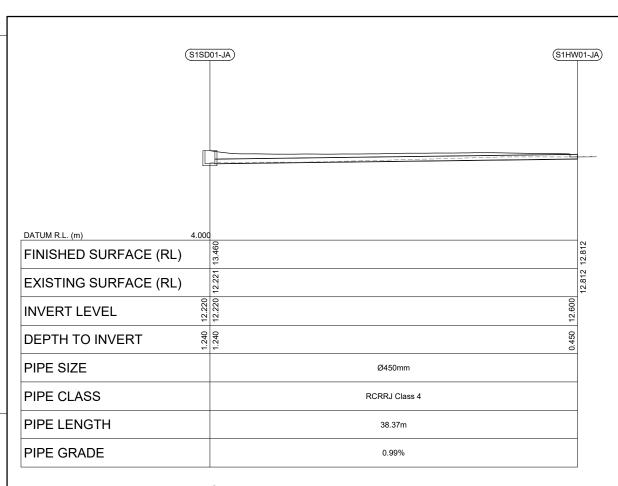






ALL BEDDING SHALL BE HS2 AS PER DRAWING VE-2139.

BEDDING NOTE:



SW LINE JA

NZ TRANSPORT AGENCY WAKA KOTAHI

SH16 - SAFETY IMPROVEMENTS

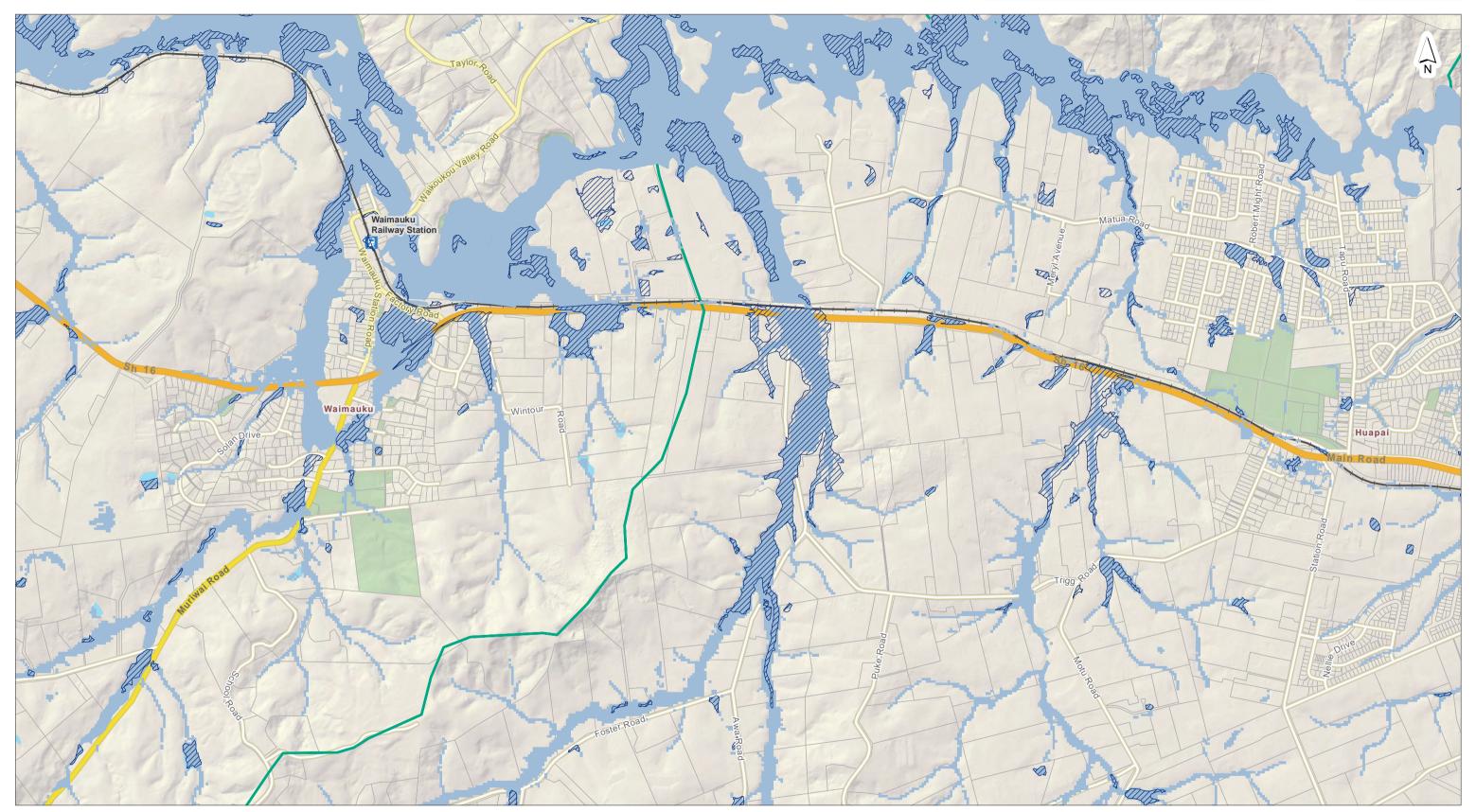
STORMWATER LONGITUDINAL SECTIONS SHEET 30 OF 30

FOR TENDER SR1003 - 01 - VE - 2180

Safe Roads STAGE 1 HUAPAI TO WAIMAUKU

APPENDIX B 1% AEP FLOOD MAP

Auckland Council Map



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Stage 1 Flood map





Flood Prone Areas

Flood Prone Areas

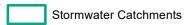
Flood Sensitive Area



Flood Plains

Flood Plains

Stormwater Catchments



Place Name (25,000)

Place Name (25,000)

Hydrographic Place Name (25,000)

Hydrographic Place Name (25,000)

Rail Stations (25,000)

Rail Stations (25,000)

Railway (25,000)

----- Railway (25,000)

Auckland Council Boundary

Auckland Council Boundary

Roads (15,000)

CLASSIFICATION

Motorway

Major Road

Arterial Road

Medium Road

Minor Road

Parcels

Parcels

Lakes



Base Region (CRS)



Region Cache Public Open Space Extent



DISCLAIMER:

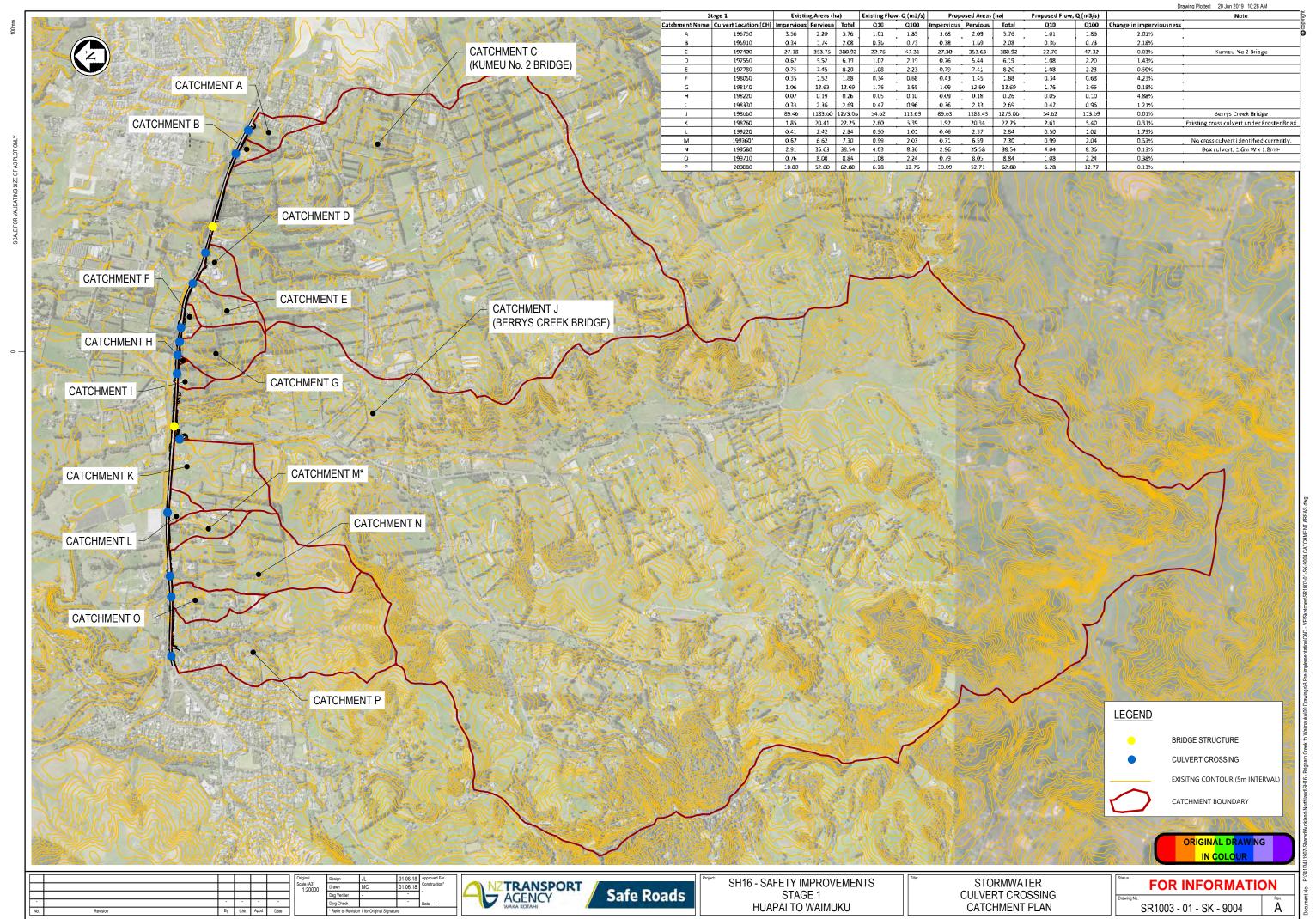
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Legend



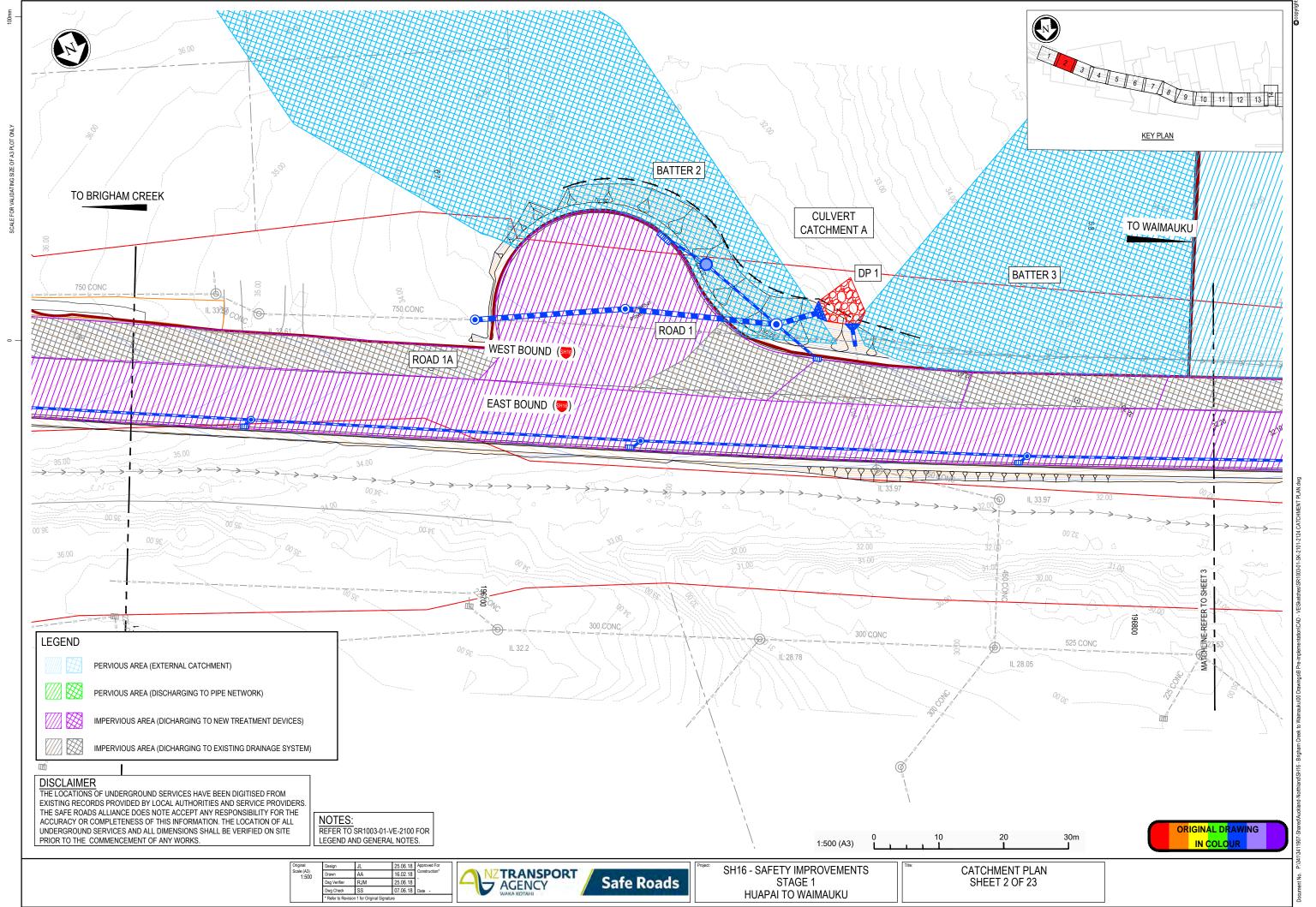
APPENDIX C

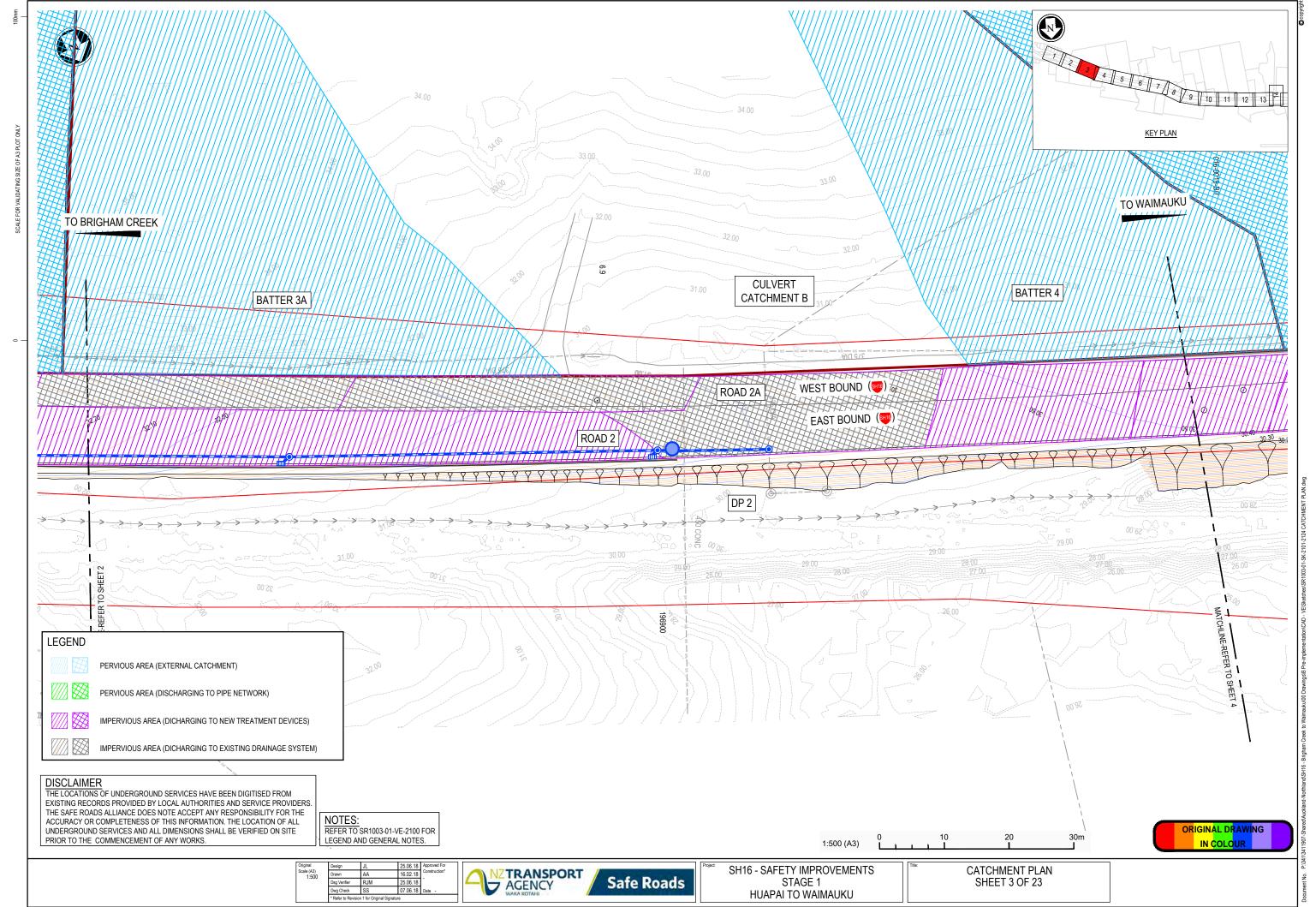
CULVERT CATCHMENT SUMMARY

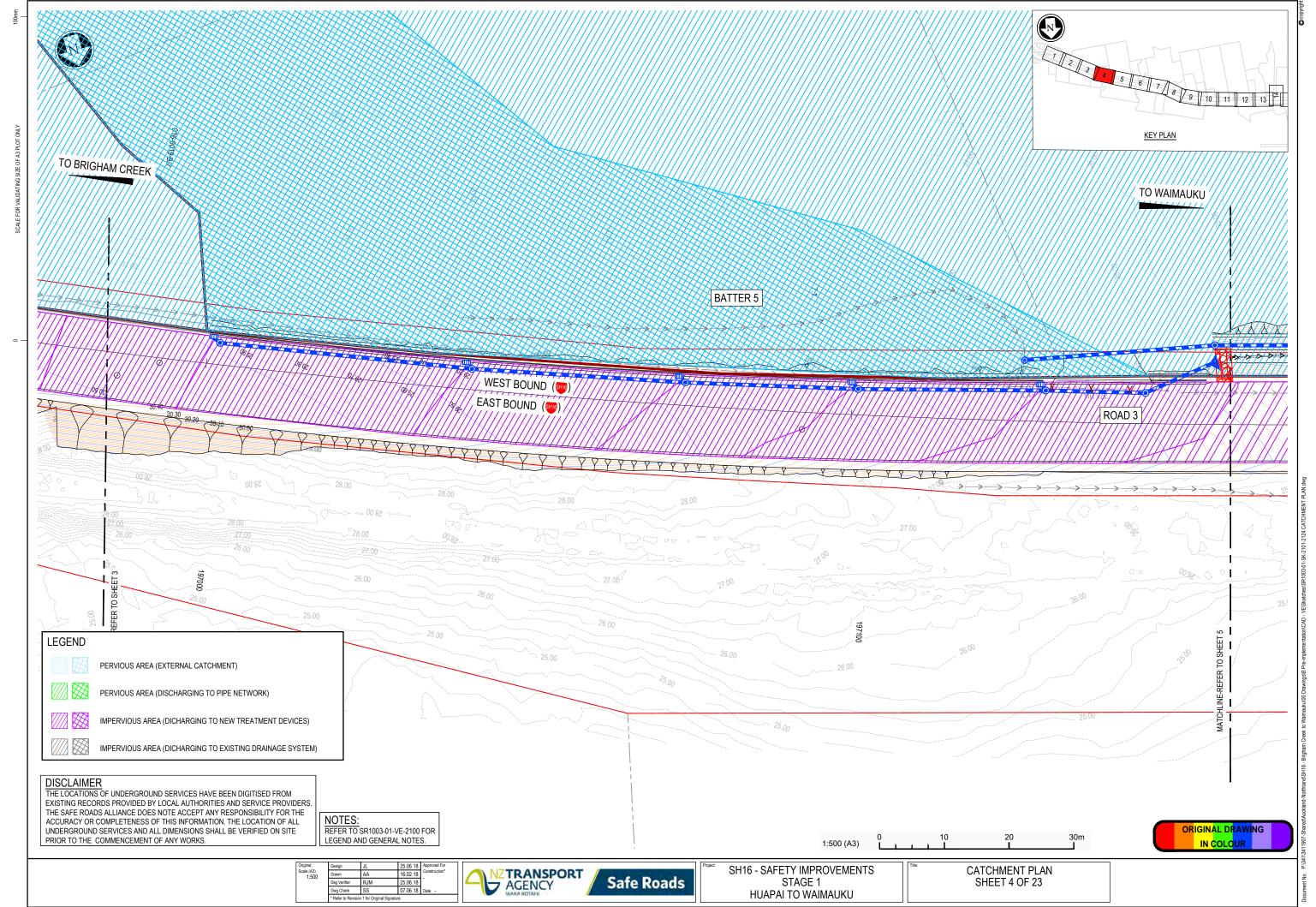


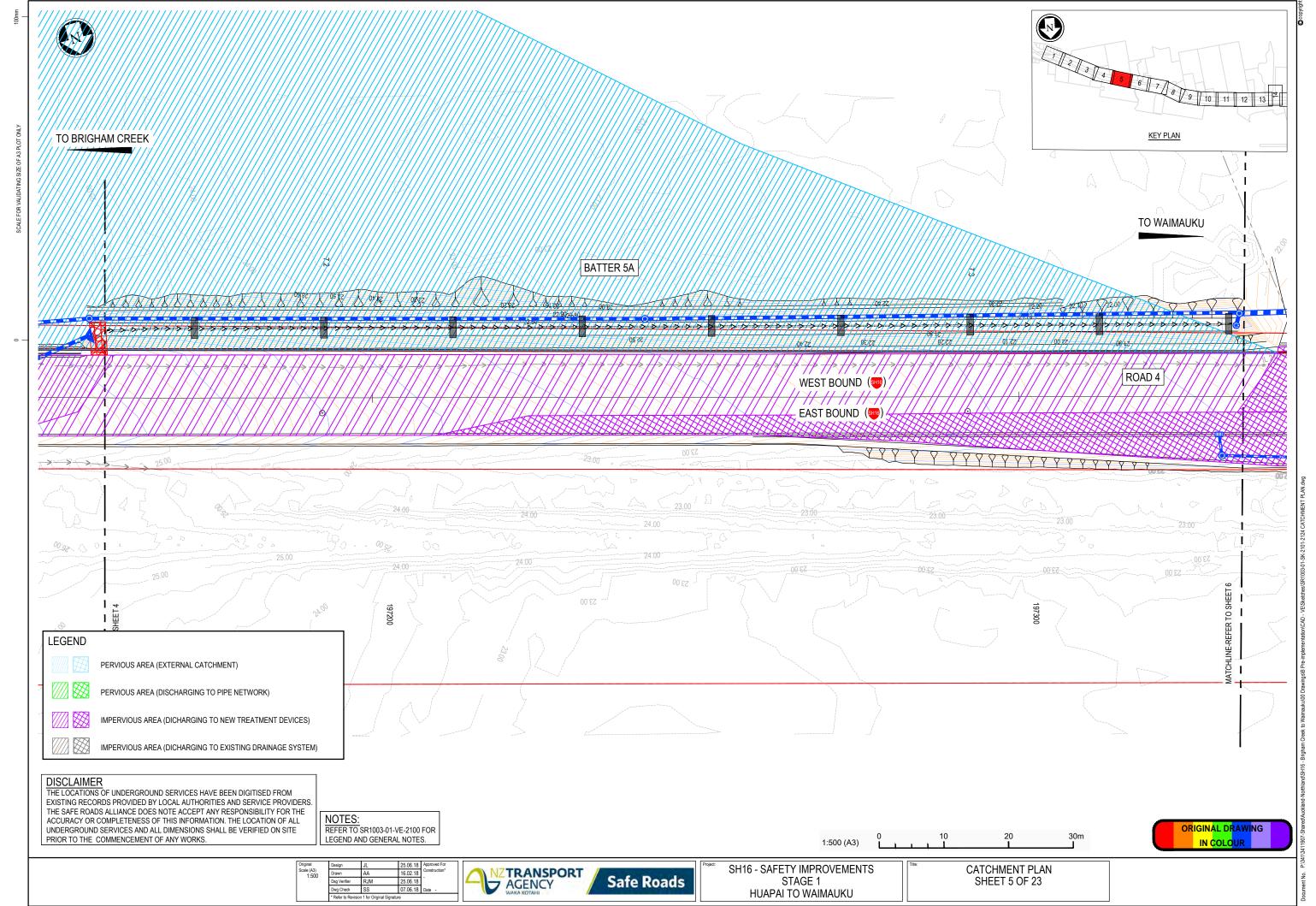
APPENDIX D

DISCHARGE POINT CATCHMENT PLAN



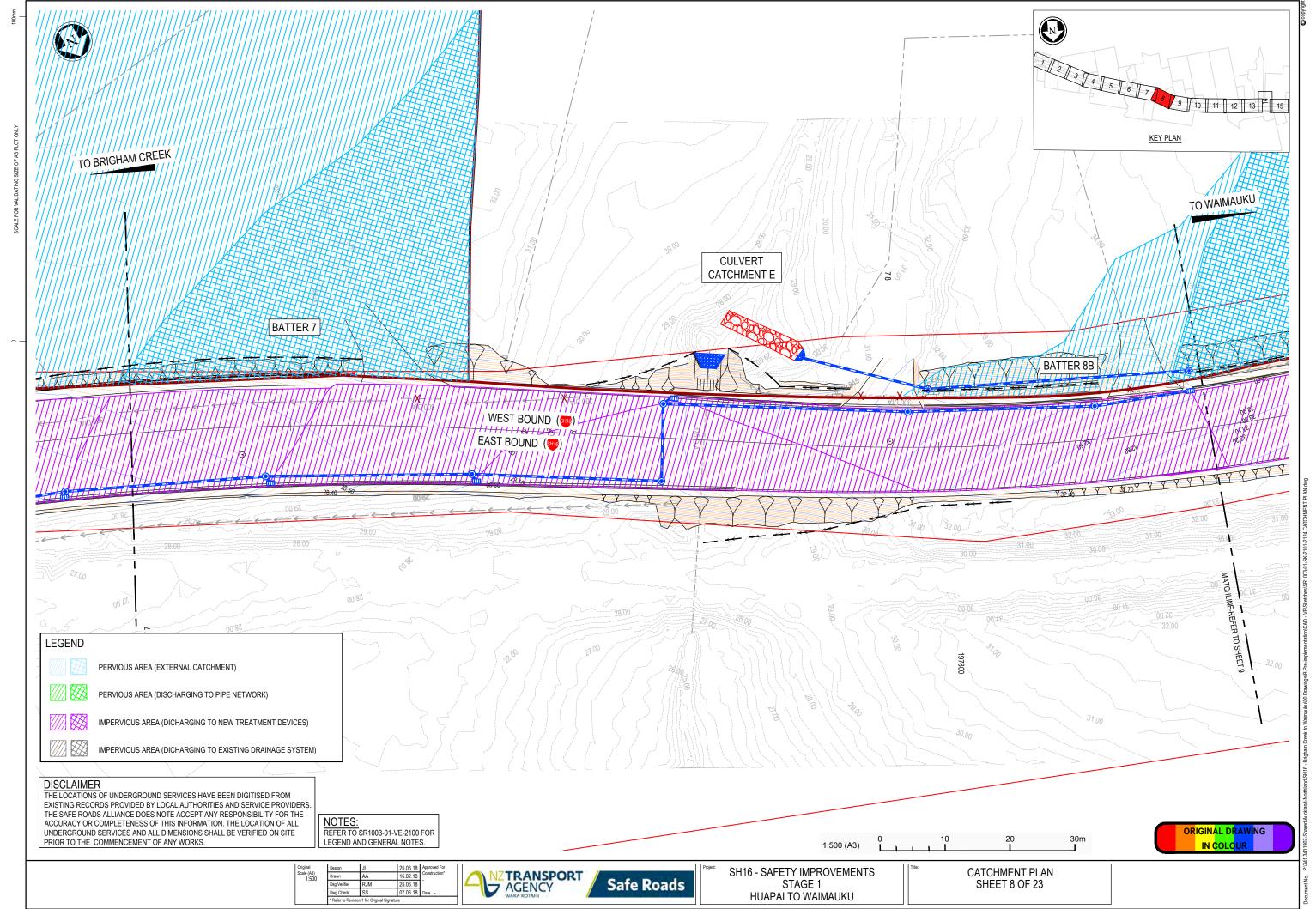


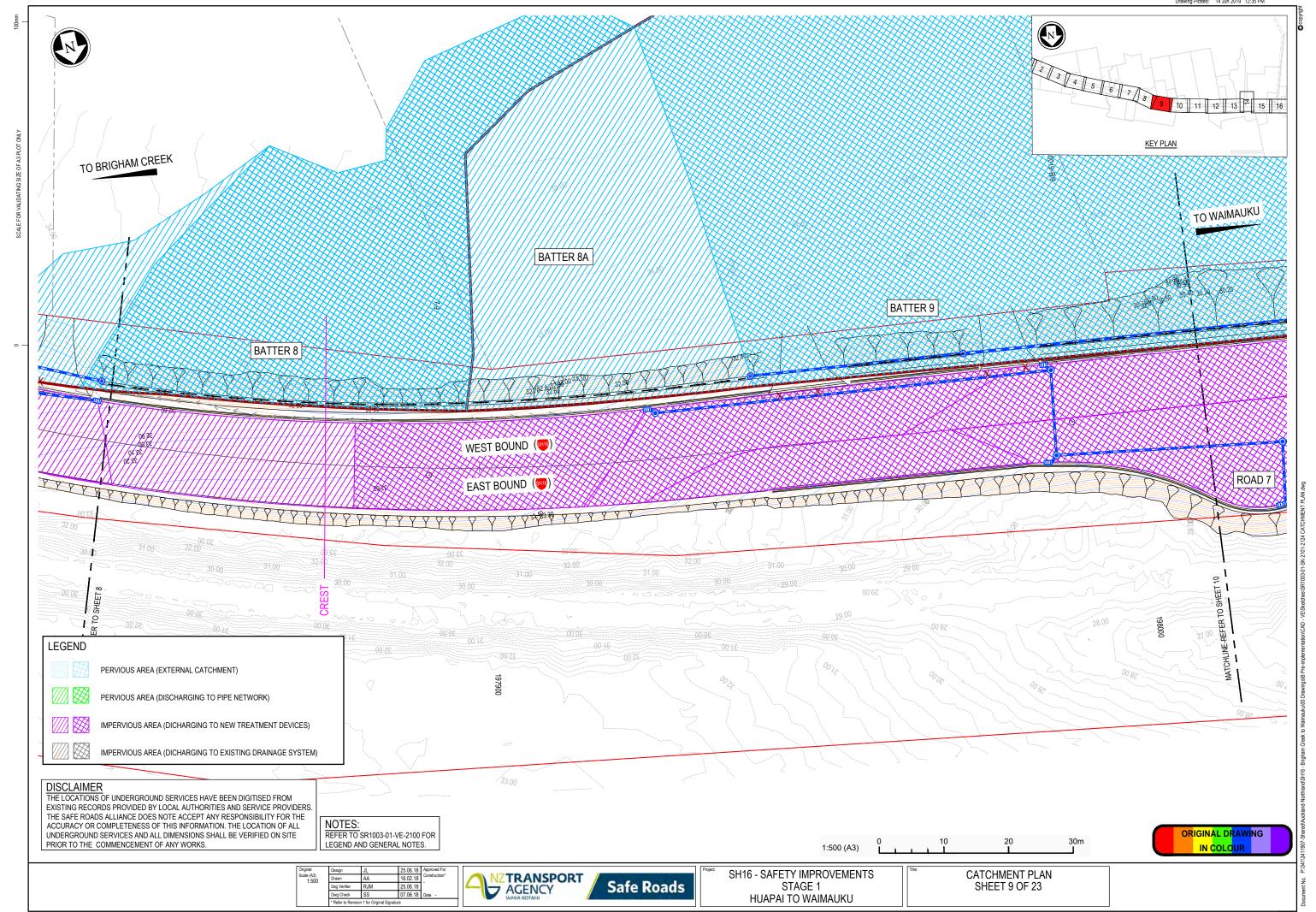


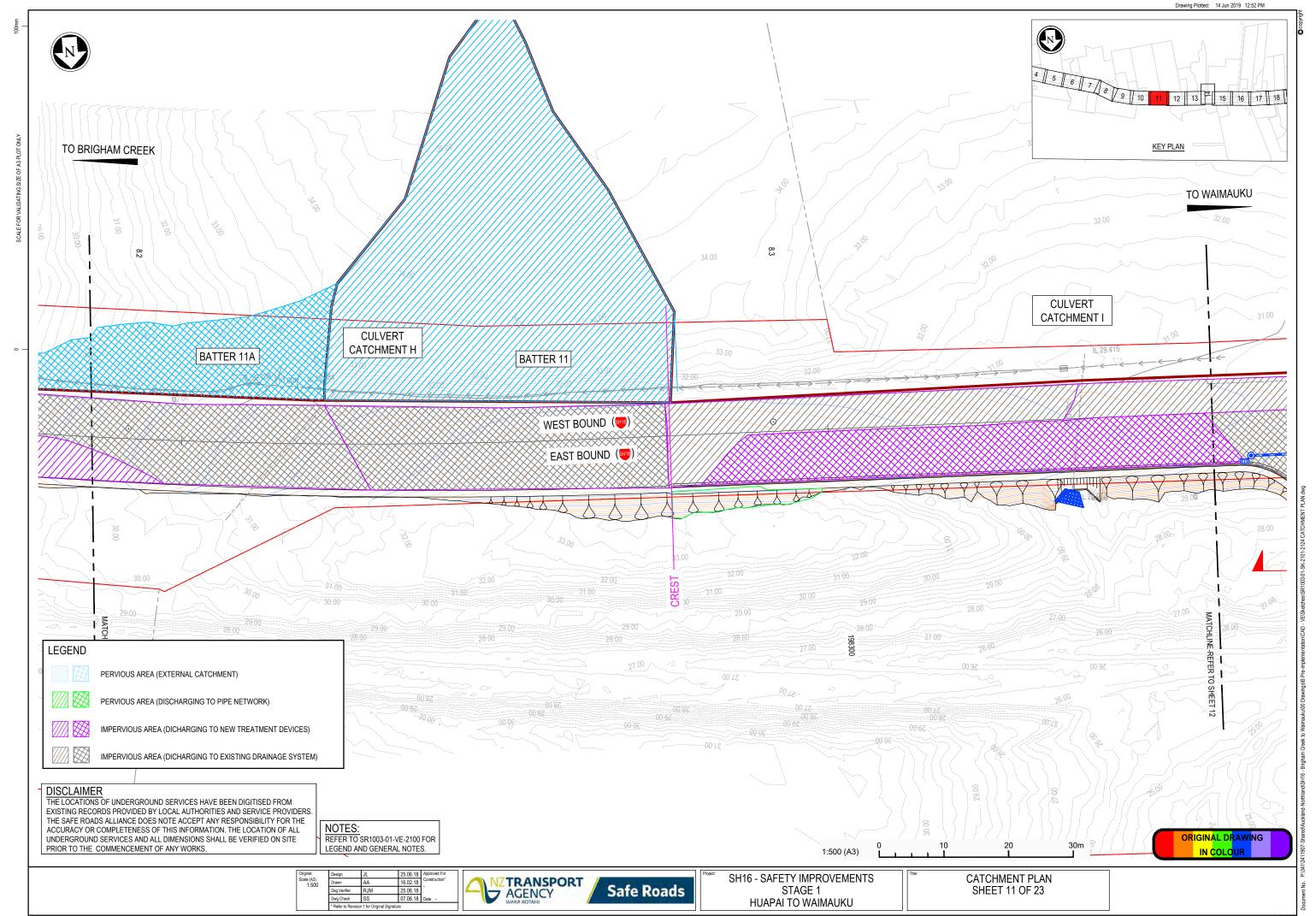


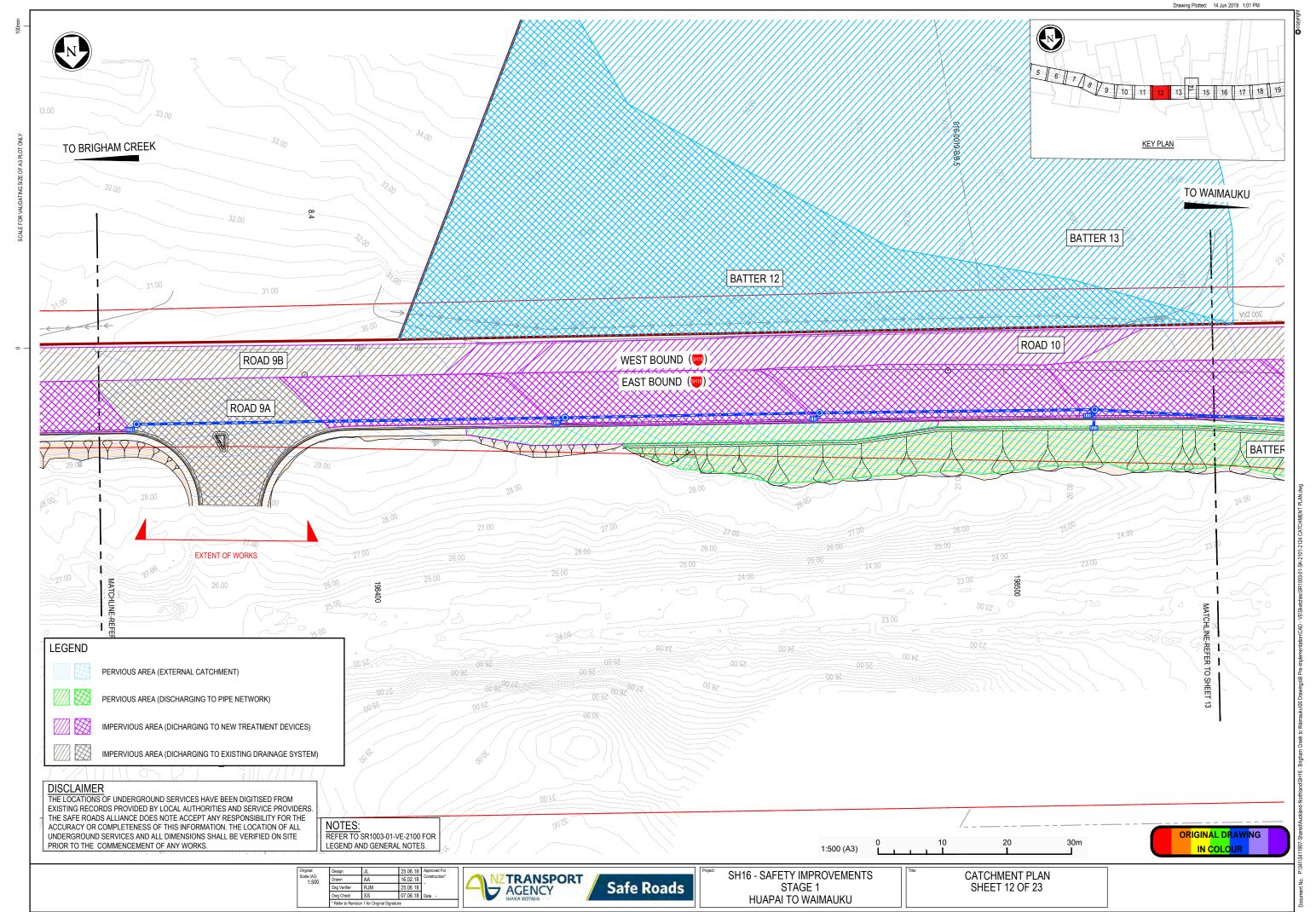
Safe Roads

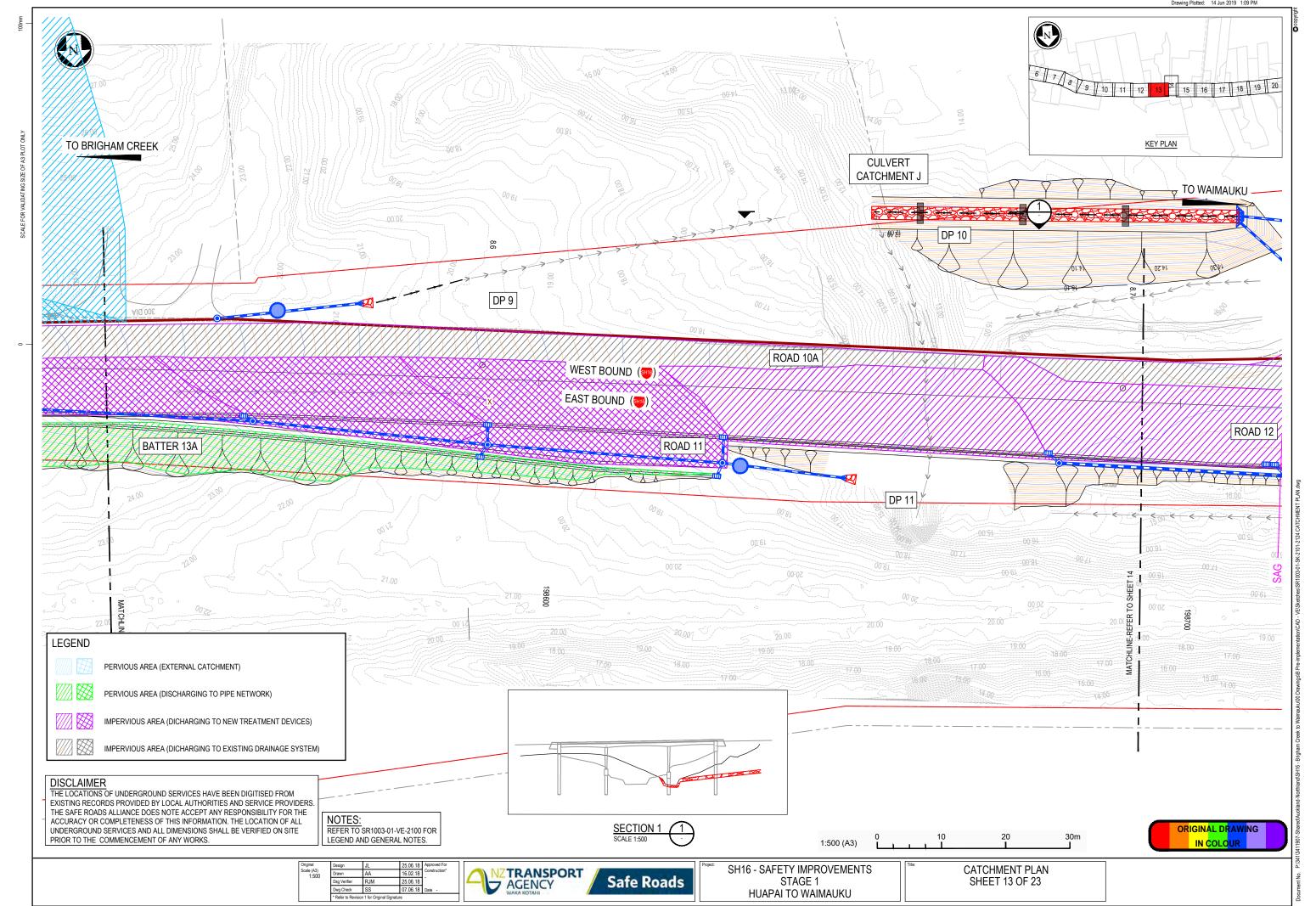
STAGE 1
HUAPAI TO WAIMAUKU

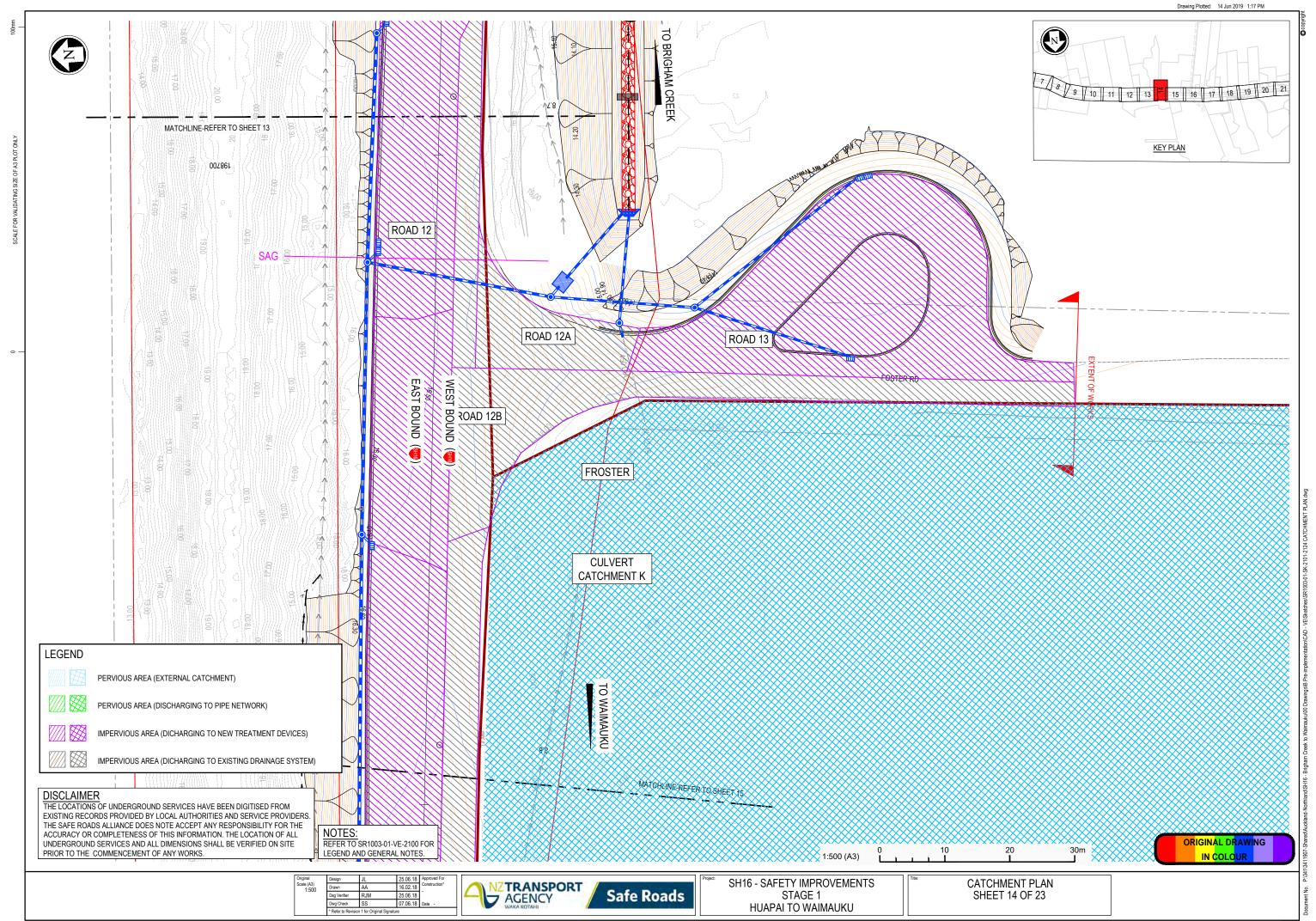












Safe Roads

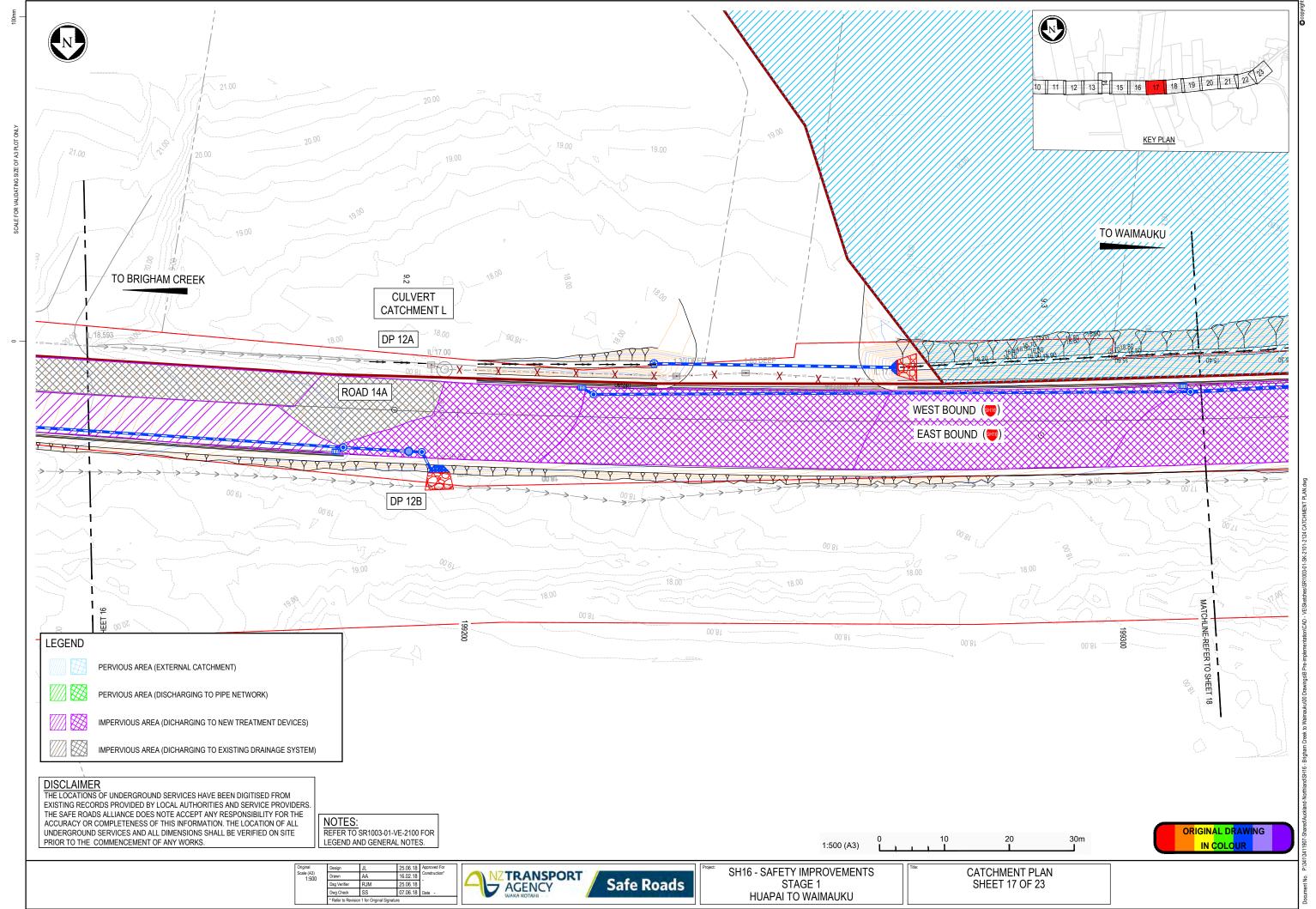
STAGE 1
HUAPAI TO WAIMAUKU

Safe Roads

STAGE 1

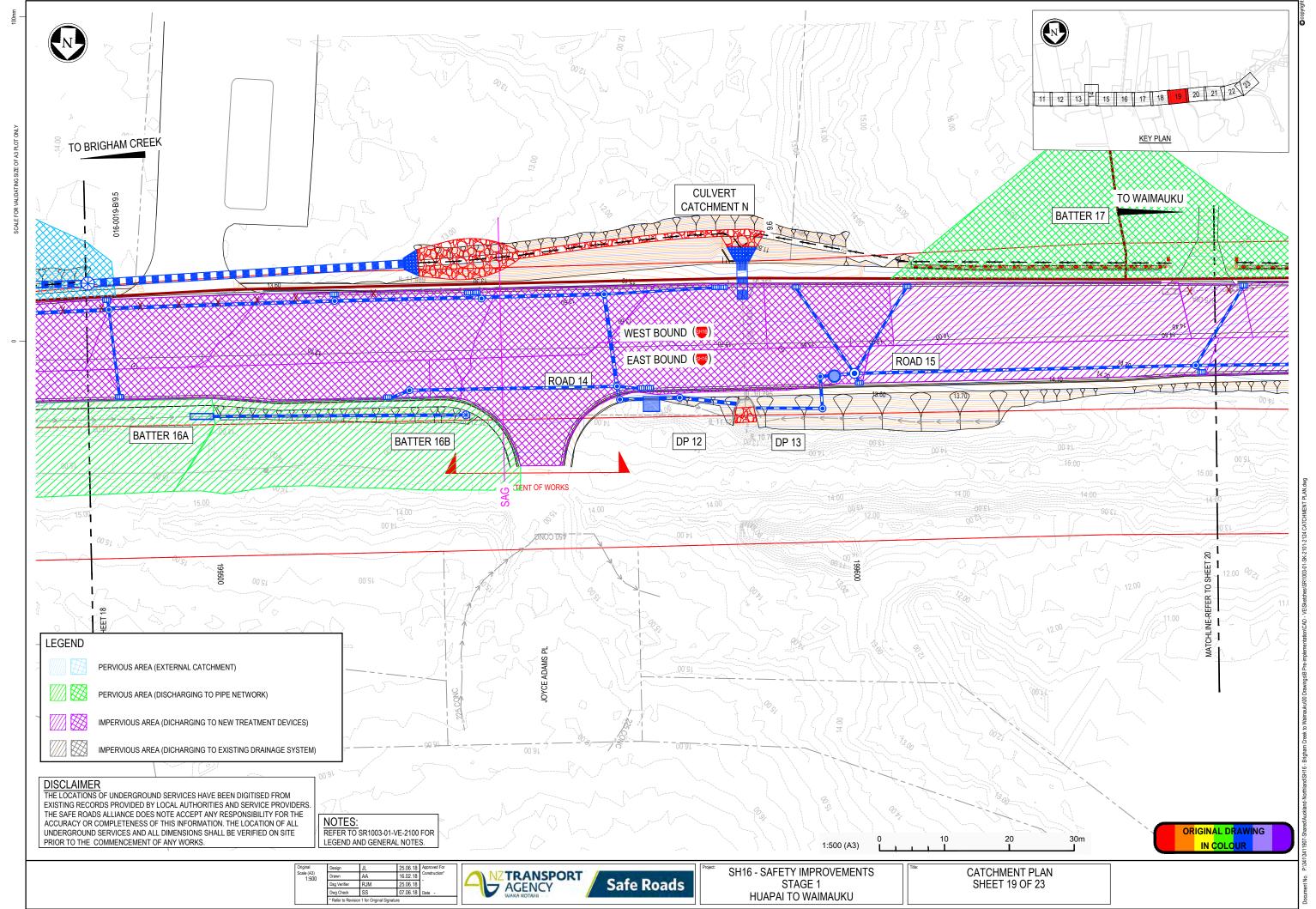
HUAPAI TO WAIMAUKU

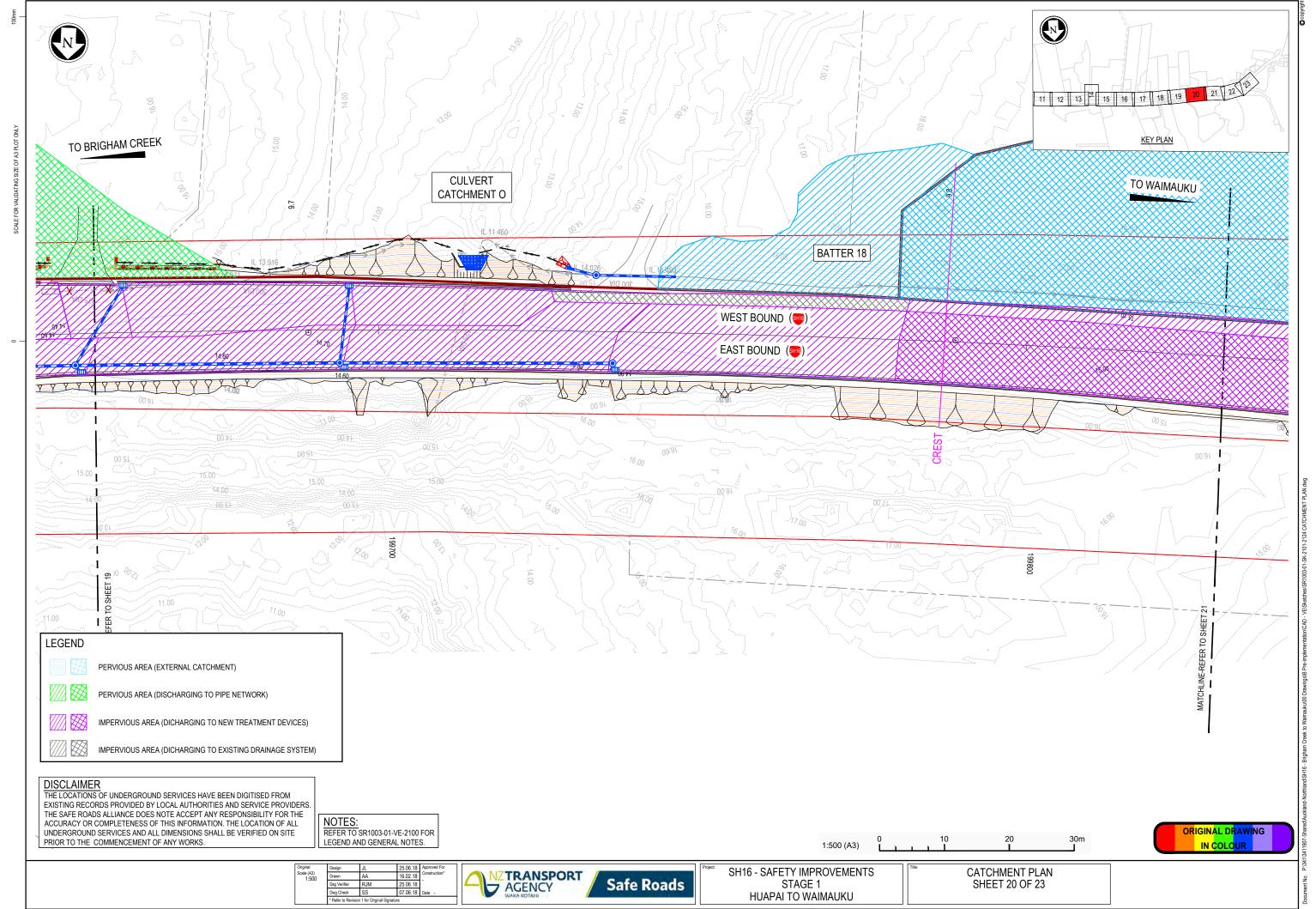
SHEET 16 OF 23

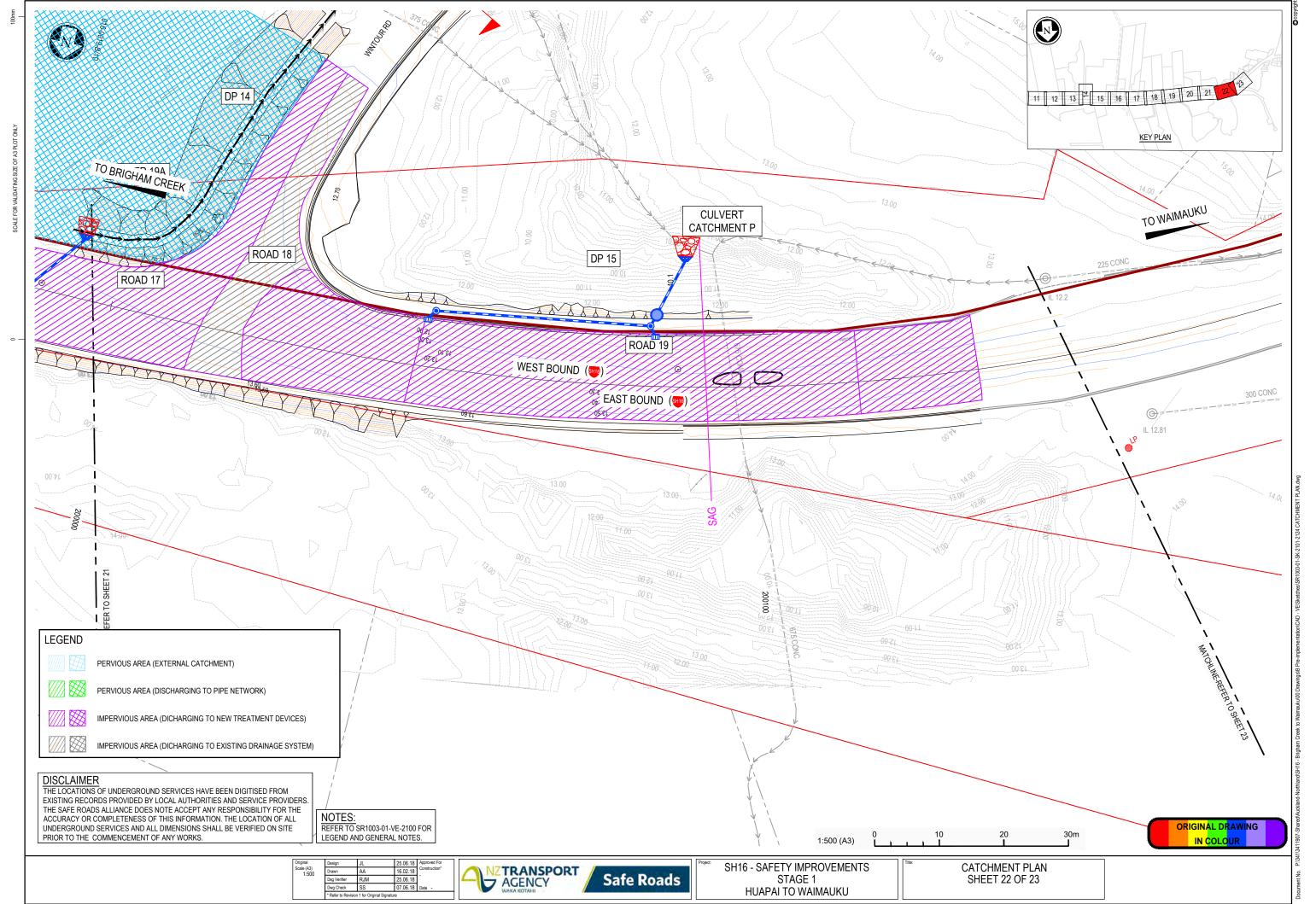


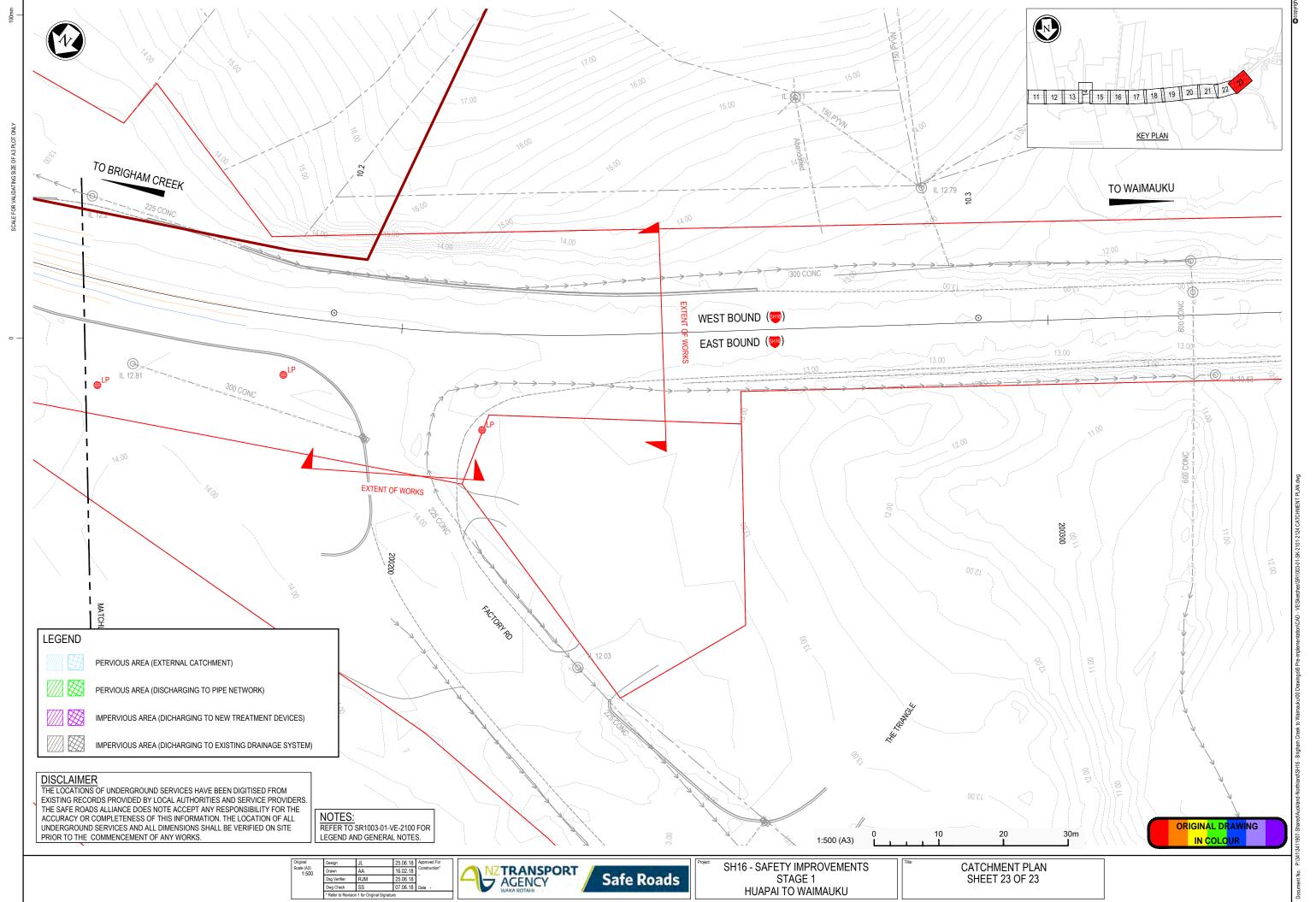
Safe Roads

STAGE 1
HUAPAI TO WAIMAUKU









APPENDIX E

EXISTING CROSS-CULVERT SUMMARY

APPENDIX F

CROSS-CULVERT FLOW SUMMARY

Table E: Existing cross culverts summary (Stage 1)

	EXISTING HYDROLOGY (4)									/ERT CONDITION (EXISTING CAPACITY	
	Catchment Name	Name (Ch)	Zoning (2)	Pervious area (ha) (3)	Impervious area (ha) (3)	Total (ha)	Q10 (m ³ /s)	Q100 (m ³ /s)	Age (years)	Diameter (m)	Length (m)	Qex (m ³ /s) (6)	Comment
	А	196750	Urban - Single	2.2	3.6	5.8	1.01	1.85	18	0.525	19.64	0.673	Less than Q10
	В	196910	Future Urban	1.7	0.3	2.1	0.36	0.73	7	0.375	22.1	0.253	Less than Q10
	С	197400 **	Future Urban	353.7	27.2	380.9	22.76	47.31	N/A	N/A	N/A	N/A	Kumeu No2 Bridge
	D	197550	Future Urban	5.5	0.7	6.2	1.07	2.19	N/A	0.700	33.9	1.620	Less than Q100; Culvert based on site observation
	Е	197780	Future Urban	7.4	0.7	8.2	1.08	2.23	28	0.375	42	0.537	Less than Q10
	F	198050	Future Urban	1.5	0.4	1.9	0.34	0.68	15	0.300	17.84	0.154	Less than Q10
	G	198140	Rural	12.6	1.1	13.7	1.76	3.65	28	0.300	39.56	0.112	Less than Q10
	Н	198220	Rural	0.2	0.1	0.3	0.05	0.10	4	0.300	25.5	0.100	Equal to Q100
Stage 1	1	198330	Rural	2.4	0.3	2.7	0.47	0.96	15	0.300	21.16	0.226	Less than Q10
	J	198660 **	Rural	1183.6	89.5	1273.1	54.62	113.69	N/A	N/A	N/A	N/A	Berry Creek Bridge
	K	198760	Rural	20.4	1.8	22.3	2.60	5.39	N/A	0.375	12.42	0.772	Less than Q10
	L	199220	Rural	2.4	0.4	2.8	0.50	1.01	18	0.375	16.01	0.304	Less than Q10
	M	199360 *	Rural	6.6	0.7	7.3	0.99	2.03	N/A	N/A	N/A	N/A	No existing culvert
	N	199580	Rural	35.6	2.9	38.5	4.03	8.36	24	1.6 W x 1.8 H	15.1	9.264	Box culvert. Greater than Q100.
	0	199710	Rural	8.1	0.8	8.8	1.08	2.24	24	0.450	24.07	0.767	Less than Q10
	Р	200080	1/6 Urban - Single 5/6 Rural	52.8	10.0	62.8	6.28	12.76	24	0.675	35.93	1.673	Less than Q10

Notes:

- 1. * Existing culvert not found at existing low point; ** Not analysed, included for completeness of catchments; N/A Information not available or applicable.
- 2. As defined in the Unitary Plan.
- 3. Pervious and impervious area are based on the existing condition as shown in aerial photograph. A minimal of 7% is assumed for impervious area for catchments lack of defined urban development/impervious area, or identified as green field and rural lots. Road impervious area is included in the impervious area, with area equating to 11m wide multiplied by the length of catchment interception.
- 4. Peak stormwater runoff is calculated using the SCS method as per the ARC TP108.
- 5. As per supplied NZTA RAMM data.
- 6. Existing capacity based on assessment of culverts for the follow scenarios and assumptions, the lesser of the two are recorded in the table:
 - a. Outlet controlled tailwater at outlet is assumed to be at the top of the pipe; headwater level is set at road level.
 - b. Inlet controlled modelled based on orifice follow where Q=0.65A(2gh)^{1/2}; h is limited to the road level

Table F: Culvert catchment flow summary (Stage 1)

S	Existing Areas (ha)			Existing Flow, Q (m³/s)		Proposed Areas (ha)			Proposed Flow, Q (m³/s)		Change in imperviousness	Note	
Catchment Name Culvert Location (CH)		Impervious	Pervious	Total	Q10	Q100	Impervious	Pervious	Total	Q10	Q100		
А	196750	3.56	2.20	5.76	1.01	1.85	3.68	2.09	5.76	1.01	1.86	2.01%	
В	196910	0.34	1.74	2.08	0.36	0.73	0.38	1.69	2.08	0.36	0.73	2.18%	
С	197400	27.18	353.75	380.92	22.76	47.31	27.30	353.63	380.92	22.76	47.32	0.03%	Kumeu No 2 Bridge
D	197550	0.67	5.52	6.19	1.07	2.19	0.76	5.44	6.19	1.08	2.20	1.43%	
E	197780	0.75	7.45	8.20	1.08	2.23	0.79	7.41	8.20	1.08	2.23	0.50%	
F	198050	0.35	1.52	1.88	0.34	0.68	0.43	1.45	1.88	0.34	0.68	4.23%	
G	198140	1.06	12.63	13.69	1.76	3.65	1.09	12.60	13.69	1.76	3.65	0.18%	
Н	198220	0.07	0.19	0.26	0.05	0.10	0.09	0.18	0.26	0.05	0.10	4.88%	
1	198330	0.33	2.36	2.69	0.47	0.96	0.36	2.33	2.69	0.47	0.96	1.21%	
J	198660	89.46	1183.60	1273.06	54.62	113.69	89.63	1183.43	1273.06	54.62	113.69	0.01%	Berrys Creek Bridge
К	198760	1.85	20.41	22.25	2.60	5.39	1.92	20.34	22.25	2.61	5.40	0.31%	Existing cross culvert under Froster Road
L	199220	0.41	2.42	2.84	0.50	1.01	0.46	2.37	2.84	0.50	1.02	1.79%	
M	199360*	0.67	6.62	7.30	0.99	2.03	0.71	6.59	7.30	0.99	2.04	0.53%	No cross culvert identified currently.
N	199580	2.91	35.63	38.54	4.03	8.36	2.96	35.58	38.54	4.04	8.36	0.13%	Box culvert, 1.6m W x 1.8m H
0	199710	0.76	8.08	8.84	1.08	2.24	0.79	8.05	8.84	1.08	2.24	0.38%	
Р	200080	10.00	52.80	62.80	6.28	12.76	10.09	52.71	62.80	6.28	12.77	0.13%	

APPENDIX G

EXISTING HIGHWAY WATERCOURSES

