Ref: 18122 11 February 2020



MacDonell Consulting Ltd

By email; bmacdonell@xtra.co.nz

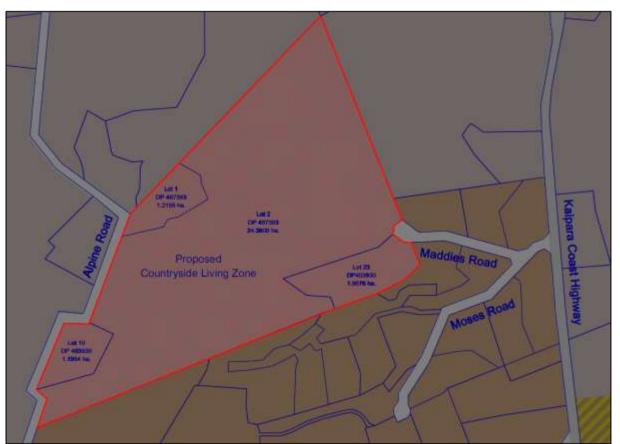
Attn: Barry MacDonell

Dear Barry

PRIVATE PLAN CHANGE – MADDIES ROAD, KAUKAPAKAPA ASSESSMENT OF TRAFFIC EFFECTS ON KAIPARA COAST HIGHWAY

As requested we have considered potential traffic generation effects associated with a proposed private plan change for land to the west of Maddies Road, north of Kaukapakapa. The plan change seeks to rezone the land from Rural Production to Rural Countryside Living. Rezoning of the land will facilitate subsequent subdivision of the land to establish up to 20 lots within the Countryside Living Zoning provision.

The proposed plan change covers four separate properties with a combined area of approximately 28.76 hectares. The area covered by the plan change is highlighted in Figure 1.



Source: C & R Surveyors Ltd – Proposed Plan Change Area May 2018

Figure 1: Proposed Plan Change Area

The plan change area has road frontage with Alpine Road to the west and connection with Maddies Road to the east. Alpine Road connects with McLachlan Road to the north of the site with sole access

to Kaipara Coast Highway/State Highway 16 (SH16) via an existing intersection further north. Alpine Road and a large proportion of McLachlan Road are currently unsealed with generally narrow carriageway width. The intersection of SH16 and McLachlan Road is located approximately 1.7km north of the intersection of SH16 and Moses Road.

Maddies Road is a short road with cul-de-sac turning head for access to six existing Countryside Living (CL) lots. Maddies Road connects with SH16 via Moses Road and a relatively new intersection constructed to provide access for CL zoned land on Moses Road and Maddies Road. We understand that the Moses Road intersection was designed and constructed in 2006 to 2007. We note that the design and construction of the intersection with extensive flush median, formal right turn bay and provision for safe deceleration and turning for left turning vehicles into Moses Road was undertaken to required standards for a 100km/h posted speed limit. Subsequent to the construction of the intersection the speed limit on SH16 past the intersection with Moses Road was reduced to 80km/h.

Consideration of intersection of SH16 with Moses Road

We consider that the most practical access option for future CL lots provided for by the proposed plan change is via Maddies Road, Moses Road and the intersection with SH16. As noted above the intersection was designed to comply with design standards for a 100km/h posted speed limit and the current speed limit at the intersection is now 80km/h. A review of recent crash data for SH16 between North Crescent in Kaukapakapa to the south of Moses Road and Kanohi Road to the north identified four crashes including one minor-injury crash over the five year period of 2015 to 2019. The minor-injury crash involved loss of control on the curve north of the Maddies Road intersection at night with a driver assessed to be above the legal alcohol limit. Overall the reported crashes do not highlight any concern regarding the operation of the Maddies road intersection. A copy of the crash listing and diagram is included as Attachment 1.

We have undertaken capacity analysis of the intersection of SH16 with Moses Road for both existing and future operation with additional CL lots provided for by the proposed plan change. The intersection capacity analysis used SIDRA intersection modelling software for the existing intersection layout and assessed vehicle operating speeds on approaches to the intersection. The following subsections outline the methodology followed to determine traffic volumes for assessment purposes.

Traffic movements to and from Moses Road

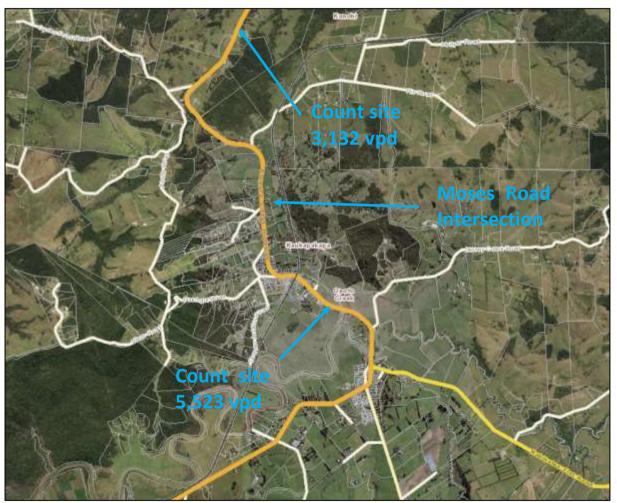
The following points are noted in relation to determining turning movements to and from Moses Road and Maddies Road at the intersection.

- There are currently around 35 lots that gain access onto Moses Road and Maddies Road with sole access via the intersection with SH16
- The proposed private plan change will provide for up to a further 20 lots to be accessed via the existing intersection
- An average trip generation rate of one (1) trip per lot during morning and evening peak hour periods has been adopted for the purpose of this assessment
- A directional split of around 80 percent outbound and 20 percent inbound during the morning peak and the reverse pattern in the evening peak has been adopted
- A directional split for turning movements at the intersection of between 80 and 90 percent to and from the south has been adopted

Assessed traffic volumes to and from Moses Road at the intersection with SH16 are 35 vehicle movements during each peak hour period for the current situation, increasing to 55 movements per hour with additional CL lots provided under the proposed plan change.

Traffic volumes on SH16

Current traffic volumes on SH16 have been sourced from the New Zealand Transport Agency (NZTA). Annual summary traffic volumes are available up to 2018 with a site immediately south of Kaukapakapa (1.5km south of Moses Road) recorded at an average daily volume of 5,523 for the two-way total. A separate count site located approximately 2.5km north of the Moses Road intersection was recorded at an average of 3,132 vehicles per day (vpd). Comparison of the two traffic count sites highlights a significant reduction in SH16 traffic volumes to the north of Kaukapakapa, including past the Moses Road intersection. Figure 2 identifies the two traffic count locations in relation to the Moses Road intersection.



Source: Auckland Council Geomaps – Traffic volumes NZTA website; recorded 2018 AADT (annual average daily traffic)

Figure 2: Location of NZTA Count Sites

We consider that an appropriate method to derive traffic volumes at the Moses Road intersection is to initially adopt the mean value between the two count sites. The Kaukapakapa township will account for the majority of the drop-off at the northern count site and adopting the mean value for the intersection located north of Kaukapakapa is considered to be a conservative approach. The mean daily traffic volume is 4,327 vpd which is approximately 78% of the recorded AADT at the count site south of Kaukapakapa.

We have sourced detail traffic count information from NZTA to determine hourly traffic volumes on SH16. The traffic count site south of Kaukapakapa is a continuous site for which daily traffic count data was available for 2019. In considering the traffic data available we have adopted daily and peak hour values that represent equivalent 85th percentile volumes for each direction as a base for assessment. Relevant values from the NZTA count data to the south of Kaukapakapa are outlined below.

- SH16 Northbound 3,222vpd (3 May 2019)
- SH16 Southbound 3,138vpd (21 November 2019)
- SH16 Combined 2-way (85th percentile equivalent) 6,360vpd
- Annual average daily traffic (NZTA reported value 2018) 5,523vpd

For assessment purposes peak hour volumes on SH16 at the Moses Road intersection were taken to be 80% of the relevant volumes recorded on the two 85th percentile dates noted above. A summary of NZTA traffic data collected and used to determine appropriate volumes at the Moses Road intersection is included as Attachment 2.

Figure 3 below presents adopted peak hour traffic volumes for the SH16 and Moses Road intersection for the existing, base operation.

Existing AM Peak Hour R2 T1 Tot 1 263 LV 100 % 95 % HV 0 % 5 % T1 SH16 (North) H۷ 100 % 3 100 % 0 % SH16 (South) T1 L2 Tot 6 145 LV 100 % 95 % HV 0 % 5 %

Existing PM Peak Hour

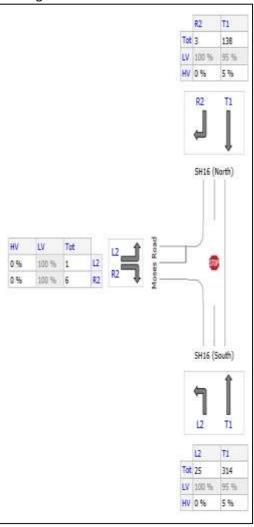


Figure 3: Adopted Base Traffic Volumes at Moses Road Intersection

Summary Findings from SIDRA Intersection Modelling

We undertook a series of SIDRA intersection model runs to reflect morning and evening peak hour periods for both the existing 35 lots on Moses Road and Maddies Road, and the future 55 lots under the proposed plan change. Summary model output is provided for all SIDRA model runs as Attachment 3 to this letter. The following points are noted for the predicted intersection operation under existing and proposed scenarios, with current background volumes on SH16.

- The right turn from Moses Road to SH16 experiences the highest delay in both peak hour periods
- The right turn from Moses Road has a level of service (LOS) B with all other movements at the intersection being LOS A
- Predicted delay values to and from Moses Road are almost entirely geometric and 'Stop' control delay rather than delay associated with priority traffic on SH16
- The predicted delay and queuing values at the intersection were essentially unchanged between the existing and proposed scenarios

In summary the intersection currently operates with no notable queuing or delay and the addition of 20 further CL lots accessed via the intersection generates no discernible impact on operation.

We undertook further intersection modelling to test future resilience of the intersection following growth to SH16 traffic volumes. The test scenarios applied percentage increases to the base SH16 peak hour traffic volumes at the intersection of 20% and 50%. The test scenario with a 20% increase in SH16 volumes results in no noticeable increase in delay or queuing at the intersection. The test scenario with a 50% increase to SH 16 volumes resulted in a minor increase in delay and queuing at the intersection and the right turn movement from Moses Road deteriorating from LOS B to LOS C during the evening peak hour. As noted above summary model output results are included as Attachment 3. Tables below are taken from Attachment 3 and present a comparison between the base scenario and a test scenario of 20 additional lots accessing via the intersection and SH16 volumes increasing by 50%.

Weekday morning peak hour – Base Scenario

Move	ement P	erformand	e - Vel	nicles								
Mov ID	Turn	Demand Total veb/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/r
South	: SH16 (A THURSDAY		032020			111240	****				***************************************
1	L2	6	0.0	0.003	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	153	5.0	0.080	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	80.0
Appro	ach	159	4.8	0.080	0.3	NA	0.0	0.0	0.00	0.03	0.00	79.3
North	: SH16 (I	North)										
8	T1	277	5.0	0.145	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	84.9
9	R2	1	0.0	0.001	7.6	LOSA	0.0	0.0	0.26	0.56	0.26	53.8
Appro	ach	278	5.0	0.145	0.0	NA	0.0	0.0	0.00	0.00	0.00	84.8
West:	Moses F	Road										
10	L2	3	0.0	0.003	3.7	LOSA	0.0	0.1	0.24	0.40	0.24	53.6
12	R2	26	0.0	0.050	10.8	LOS B	0.2	1.3	0.52	0.92	0.52	46.8
Appro	ach	29	0.0	0.050	10.1	LOS B	0.2	1.3	0.49	0.86	0.49	47.4
All Ve	hicles	466	4.6	0.145	0.8	NA	0.2	1.3	0.03	0.06	0.03	80.0

Weekday morning peak hour – Base Scenario plus Proposed 20 lots and 50% increase in SH16 volumes

Move	ement F	erforman	ce - Vel	nicles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: SH16 (South)	32340				1000000					
1	L2	9	0.0	0.005	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	229	5.0	0.120	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	80.0
Appro	ach	239	4.8	0.120	0.3	NA	0.0	0.0	0.00	0.02	0.00	79.3
North	: SH16 (North)										
8	T1	416	5.0	0.218	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	84.9
9	R2	2	0.0	0.002	8.0	LOSA	0.0	0.0	0.33	0.57	0.33	53.5
Appro	ach	418	5.0	0.218	0.1	NA	0.0	0.0	0.00	0.00	0.00	84.7
West:	Moses	Road										
10	L2	5	0.0	0.005	4.0	LOSA	0.0	0.1	0.31	0.43	0.31	53.3
12	R2	41	0.0	0.114	15.0	LOS B	0.4	2.8	0.67	1.00	0.67	43.7
Appro	ach	46	0.0	0.114	13.7	LOS B	0.4	2.8	0.63	0.94	0.63	44.6
All Ve	hicles	703	4.6	0.218	1.0	NA	0.4	2.8	0.04	0.07	0.04	79.4

Weekday evening peak hour – Base Scenario

Mov	Turn	Demand	Flows	Deg	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver, No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
200		veh/h	%	v/c	SEC	10000000	veh	m		500000	2300000	km/h
South	: SH16 (South)										
1	L2	26	0.0	0.014	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	331	5.0	0.173	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	79.9
Appro	ach	357	4.6	0.173	0.5	NA	0.0	0.0	0.00	0.05	0.00	78.6
North	SH16 (I	North)										
8	T1	145	5.0	0.076	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	85.0
9	R2	3	0.0	0.003	8.6	LOSA	0.0	0.1	0.41	0.60	0.41	53.1
Appro	ach	148	4.9	0.076	0.2	NA	0.0	0.1	0.01	0.01	0.01	84.2
West:	Moses F	Road										
10	L2	1	0.0	0.001	4.5	LOSA	0.0	0.0	0.38	0.44	0.38	53.0
12	R2	6	0.0	0.013	11.5	LOS B	0.0	0.3	0.54	0.88	0.54	46.3
Appro	ach	7	0.0	0.013	10.5	LOS B	0.0	0.3	0.52	0.81	0.52	47.1
All Ve	hicles	513	4.6	0.173	0.6	NA	0.0	0.3	0.01	0.05	0.01	79.6

Weekday evening peak hour – Base Scenario plus Proposed 20 lots and 50% increase in SH16 volumes

Mov	Tum	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Detay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
South	: SH16 (South)	%	v/c	sec		veh	m				km/
1	L2	41	0.0	0.022	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.
2	T1	496	5.0	0.260	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	79.
Appro	ach	537	4.6	0.260	0.6	NA	0.0	0.0	0.00	0.05	0.00	78.
North:	SH16 (I	North)										
8	T1	218	5.0	0.115	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	85.
9	R2	5	0.0	0.007	9.7	LOSA	0.0	0.2	0.51	0.66	0.51	52.
Appro	ach	223	4.9	0.115	0.2	NA	0.0	0.2	0.01	0.02	0.01	84.
West:	Moses F	Road										
10	L2	2	0.0	0.003	5.6	LOSA	0.0	0.1	0.47	0.51	0.47	52.
12	R2	9	0.0	0.031	16.6	LOS C	0.1	0.7	0.70	0.98	0.70	42.
Appro	ach	12	0.0	0.031	14.6	LOS B	0.1	0.7	0.66	0.89	0.66	44.
All Ve	hicles	772	4.6	0.260	0.7	NA	0.1	0.7	0.01	0.05	0.01	79.

For both peak hour periods the predicted delay for the right turn from Moses Road increases from around 11 seconds (almost exclusively geometric and Stop delay) to up to 16 seconds with the provision of the proposed plan change in place (20 additional lots) and a 50% increase in SH16 peak hour volumes.

Conclusions

We consider that the intersection of SH16 and Moses Road is a suitable point of access for additional traffic movements associated with the proposed plan change to Countryside Living zone. The intersection has been constructed to a high standard within the context of the adjacent road network and the recent crash history does not highlight any notable safety concern.

Traffic generation associated with the additional 20 CL lots anticipated by the plan change can comfortably be accommodated by the intersection with no notable effect on delay for other road users, including current residents on Moses Road and Maddies Road.

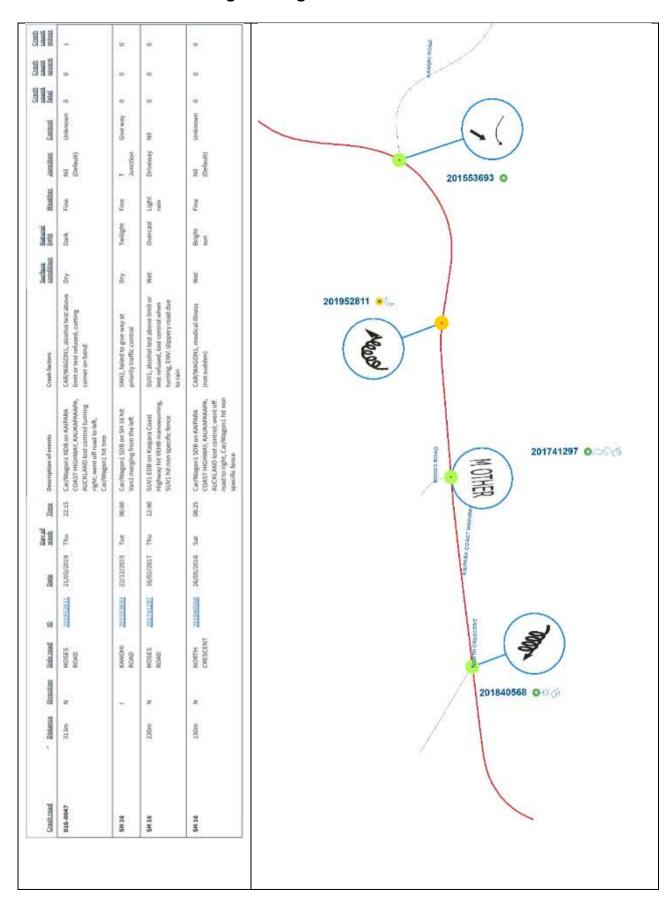
We trust the above provides sufficient detail for your immediate needs. Should you wish to discuss any matter in greater detail, do not hesitate contacting the undersigned.

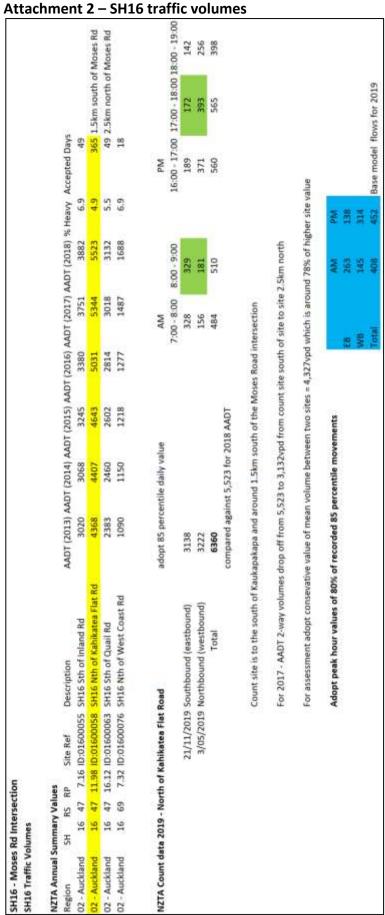
Yours faithfully

TRAFFIC ENGINEERING & MANAGEMENT LTD

David Philip

Attachment 1 - Crash listing and diagram





Attachment 3 – Summary SIDRA outputs

MOVEMENT SUMMARY

Site: 102 [SH16 Moses Road AM Existing]

SH16 Moses Road Existing AM Peak

Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand		Deg.	Average	Level of	95% Back		Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	
Courth	: SH16 (veh/h	%	v/c	sec		veh	m				km/h
1	L2	6	0.0	0.003	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	
2	T1	153	5.0	0.080	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	80.0
Appro	ach	159	4.8	0.080	0.3	NA	0.0	0.0	0.00	0.03	0.00	79.3
North	: SH16 (I	North)										
8	T1	277	5.0	0.145	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	84.9
9	R2	1	0.0	0.001	7.6	LOSA	0.0	0.0	0.26	0.56	0.26	53.8
Appro	ach	278	5.0	0.145	0.0	NA	0.0	0.0	0.00	0.00	0.00	84.8
West:	Moses F	Road										
10	L2	3	0.0	0.003	3.7	LOSA	0.0	0.1	0.24	0.40	0.24	53.6
12	R2	26	0.0	0.050	10.8	LOS B	0.2	1.3	0.52	0.92	0.52	46.8
Appro	ach	29	0.0	0.050	10.1	LOS B	0.2	1.3	0.49	0.86	0.49	47.4
All Ve	hicles	466	4.6	0.145	0.8	NA	0.2	1.3	0.03	0.06	0.03	80.0

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road AM Proposed]

SH16 Moses Road AM Peak with additional 20 lots Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
South	: SH16 (South)	0.0040					III OTT				NO. CONTRACTOR
1	L2	9	0.0	0.005	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	153	5.0	0.080	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	80.0
Appro	ach	162	4.7	0.080	0.4	NA	0.0	0.0	0.00	0.04	0.00	78.9
North	: SH16 (I	North)										
8	T1	277	5.0	0.145	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	84.9
9	R2	2	0.0	0.002	7.7	LOSA	0.0	0.0	0.26	0.57	0.26	53.8
Appro	ach	279	5.0	0.145	0.1	NA	0.0	0.0	0.00	0.00	0.00	84.7
West:	Moses I	Road										
10	L2	5	0.0	0.005	3.7	LOSA	0.0	0.1	0.24	0.41	0.24	53.6
12	R2	41	0.0	0.078	11.0	LOS B	0.3	2.0	0.53	0.94	0.53	46.7
Appro	ach	46	0.0	0.078	10.2	LOS B	0.3	2.0	0.50	0.88	0.50	47.4
All Ve	hicles	487	4.4	0.145	1.1	NA	0.3	2.0	0.05	0.10	0.05	78.5

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road AM Proposed 20% sensitivity]

SH16 Moses Road AM Peak with additional 20 lots and added 20% on SH16 Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
South	: SH16 (South)										
1	L2	9	0.0	0.005	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	183	5.0	0.096	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	80.0
Appro	ach	193	4.8	0.096	0.4	NA	0.0	0.0	0.00	0.03	0.00	79.1
North	: SH16 (North)										
8	T1	333	5.0	0.174	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	84.9
9	R2	2	0.0	0.002	7.8	LOSA	0.0	0.0	0.29	0.57	0.29	53.7
Appro	ach	335	5.0	0.174	0.1	NA	0.0	0.0	0.00	0.00	0.00	84.7
West	Moses I	Road										
10	L2	5	0.0	0.005	3.8	LOSA	0.0	0.1	0.27	0.42	0.27	53.5
12	R2	41	0.0	0.090	12.4	LOS B	0.3	2.3	0.58	0.97	0.58	45.6
Appro	ach	46	0.0	0.090	11.4	LOS B	0.3	2.3	0.54	0.91	0.54	46.4
All Ve	hicles	574	4.5	0.174	1.1	NA	0.3	2.3	0.04	0.09	0.04	79.0

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road AM Proposed 50% sensitivity]

SH16 Moses Road AM Peak with additional 20 lots and added 50% on SH16 Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand		Deg.	Average	Level of	95% Back		Prop.	Effective	Aver. No.	
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	
South	: SH16 (veh/h South)	%	v/c	sec		veh	m				km/h
1	L2	9	0.0	0.005	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	229	5.0	0.120	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	80.0
Appro	oach	239	4.8	0.120	0.3	NA	0.0	0.0	0.00	0.02	0.00	79.3
North	: SH16 (North)										
8	T1	416	5.0	0.218	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	84.9
9	R2	2	0.0	0.002	8.0	LOSA	0.0	0.0	0.33	0.57	0.33	53.5
Appro	oach	418	5.0	0.218	0.1	NA	0.0	0.0	0.00	0.00	0.00	84.7
West	Moses	Road										
10	L2	5	0.0	0.005	4.0	LOSA	0.0	0.1	0.31	0.43	0.31	53.3
12	R2	41	0.0	0.114	15.0	LOS B	0.4	2.8	0.67	1.00	0.67	43.7
Appro	oach	46	0.0	0.114	13.7	LOS B	0.4	2.8	0.63	0.94	0.63	44.6
All Ve	hicles	703	4.6	0.218	1.0	NA	0.4	2.8	0.04	0.07	0.04	79.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road PM Existing]

SH16 Moses Road Existing PM Peak Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	
0 - 11	01140	veh/h	%	v/c	sec		veh	m				km/h
South	: SH16 (LAURENCE CONTRACTOR										
1	L2	26	0.0	0.014	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	331	5.0	0.173	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	79.9
Appro	ach	357	4.6	0.173	0.5	NA	0.0	0.0	0.00	0.05	0.00	78.6
North	SH16 (North)										
8	T1	145	5.0	0.076	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	85.0
9	R2	3	0.0	0.003	8.6	LOSA	0.0	0.1	0.41	0.60	0.41	53.1
Appro	ach	148	4.9	0.076	0.2	NA	0.0	0.1	0.01	0.01	0.01	84.2
West:	Moses I	Road										
10	L2	1	0.0	0.001	4.5	LOSA	0.0	0.0	0.38	0.44	0.38	53.0
12	R2	6	0.0	0.013	11.5	LOS B	0.0	0.3	0.54	0.88	0.54	46.3
Appro	ach	7	0.0	0.013	10.5	LOS B	0.0	0.3	0.52	0.81	0.52	47.1
All Ve	hicles	513	4.6	0.173	0.6	NA	0.0	0.3	0.01	0.05	0.01	79.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road PM Proposed]

SH16 Moses Road PM Peak with additional 20 lots

Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand	Flows	Deq.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	100000	Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	
South	: SH16 (South)										
1	L2	41	0.0	0.022	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	331	5.0	0.173	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	79.9
Appro	ach	372	4.4	0.173	0.8	NA	0.0	0.0	0.00	0.07	0.00	78.0
North	: SH16 (North)										
8	T1	145	5.0	0.077	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	85.0
9	R2	5	0.0	0.005	8.7	LOSA	0.0	0.1	0.42	0.61	0.42	53.1
Appro	ach	151	4.8	0.077	0.3	NA	0.0	0.1	0.01	0.02	0.01	83.7
West	Moses I	Road										
10	L2	2	0.0	0.002	4.5	LOSA	0.0	0.1	0.38	0.45	0.38	53.0
12	R2	9	0.0	0.020	11.6	LOS B	0.1	0.5	0.55	0.89	0.55	46.1
Appro	ach	12	0.0	0.020	10.4	LOS B	0.1	0.5	0.52	0.81	0.52	47.3
All Ve	hicles	534	4.5	0.173	0.9	NA	0.1	0.5	0.02	0.07	0.02	78.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road PM Proposed 20% sensitivity]

SH16 Moses Road PM Peak with additional 20 lots and added 20% on SH16 Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	,	Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	
South	: SH16 (South)										
1	L2	41	0.0	0.022	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	397	5.0	0.208	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	79.9
Appro	ach	438	4.5	0.208	0.7	NA	0.0	0.0	0.00	0.06	0.00	78.2
North	SH16 (I	North)										
8	T1	175	5.0	0.092	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	85.0
9	R2	9	0.0	0.011	9.1	LOSA	0.0	0.3	0.46	0.65	0.46	52.7
Appro	ach	184	4.7	0.092	0.5	NA	0.0	0.3	0.02	0.03	0.02	83.1
West:	Moses F	Road										
10	L2	2	0.0	0.002	4.9	LOSA	0.0	0.1	0.42	0.47	0.42	52.8
12	R2	9	0.0	0.024	13.4	LOS B	0.1	0.6	0.61	0.92	0.61	44.8
Appro	ach	12	0.0	0.024	11.9	LOS B	0.1	0.6	0.57	0.84	0.57	46.1
All Ve	hicles	634	4.5	0.208	0.8	NA	0.1	0.6	0.02	0.07	0.02	78.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 102 [SH16 Moses Road PM Proposed 50% sensitivity]

SH16 Moses Road PM Peak with additional 20 lots and added 50% on SH16

Site Category: (None) Stop (Two-Way)

Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
South	: SH16 (LIDSON DO		NA CONTRACTOR OF THE PARTY OF T			1000-1000					
1	L2	41	0.0	0.022	6.9	LOSA	0.0	0.0	0.00	0.63	0.00	60.7
2	T1	496	5.0	0.260	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	79.9
Appro	ach	537	4.6	0.260	0.6	NA	0.0	0.0	0.00	0.05	0.00	78.5
North	SH16 (North)										
8	T1	218	5.0	0.115	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	85.0
9	R2	5	0.0	0.007	9.7	LOSA	0.0	0.2	0.51	0.66	0.51	52.0
Appro	ach	223	4.9	0.115	0.2	NA	0.0	0.2	0.01	0.02	0.01	84.1
West:	Moses I	Road										
10	L2	2	0.0	0.003	5.6	LOSA	0.0	0.1	0.47	0.51	0.47	52.3
12	R2	9	0.0	0.031	16.6	LOSC	0.1	0.7	0.70	0.98	0.70	42.6
Appro	ach	12	0.0	0.031	14.6	LOS B	0.1	0.7	0.66	0.89	0.66	44.1
All Ve	hicles	772	4.6	0.260	0.7	NA	0.1	0.7	0.01	0.05	0.01	79.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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