Development information form – Wastewater network planning summary assessment

Information to be completed by Developer/ Engineering Consultant

Development consideration	Description	Comments
Query status	Resource Consent Application	Pre-purchase enquiry / Enquiry to support Plan Change application / Pre-application enquiry / Resource Consent application / Engineering Plan approval.
Query submission date	16/09/2025	
Address	147-153 Edgewater Drive, Pakuranga, Auckland 2010	Include suburb
Attach layout plan	Please refer to DCC Civil Engineering Plan Set	Plan must clearly show proposed development site and include: • Aerial photograph with elevation contours (Note 1) • Road names • Boundary of development • Preferred point of connection to existing water supply and wastewater asset.
Current land use	Residential (single Family dwellings)	Residential (single family dwellings) / Residential (multi-unit dwellings) /
Proposed land use	Residential (multi- storey elderly retirement village apartment complex)	Residential (multi-storey apartment blocks) / Commercial / Industrial / Other (Please specify).
Unitary plan zoning	Mixed Housing Suburban Zone	Refer Auckland Unitary Plan
Total development site area (m²/hectares) (i.e. Land area for residential developments)	2990 m ²	
Total development floor area (m²) (i.e. Include all levels of multi-storey apartments and commercial developments)	Block A – 4715 m ² Basement: 659 m ² Ground Floor: 677 m ² Level 1: 676 m ² Level 3: 676 m ² Level 4: 676 m ² Level 5: 674 m ² Block B – 3625 m ² Ground Floor: 681 m ² Level 1: 626 m ² Level 2: 626 m ² Level 3: 626 m ² Level 4: 598 m ² Level 5: 469 m ²	

Number of proposed residential		Include type and number of bedrooms for		
dwellings		residential dwellings:		
(Typically consent or include	Two 6 storey elderly	<u>Type</u> :	<u>Quantity:</u>	
ultimate if development is to be	retirement village	1 bed	10	
staged and consented at a future	apartment complex	2 bed	12	
date)		2+ bed	23	
	Block A – 6 storey	3 bed	6	
	residential apartment	4bed	0	
	with 24 units.	5+bed	0	
	Block B – 6 Storey residential apartment with 27 units.			

Note: (1) Watercare's GIS Viewer for Asset Data Query and Land Development/ Subdivision can be used to display aerial photography and land contour information.

(This section should not be duplicated if both water and wastewater is applied. Refer to Chapter 6 of the CoP.)

Refer to the Auckland Code of Practice for Land Development and Subdivision chapter 5: Wastewater, when completing this form:

Design considerat	ion	Description	Comments
Existing site design flows -	Residential Design Flows (L/s)	Self-Cleansing Design Flow = 2.95 L/s	Show calculations based on Watercare CoP.
pre- development scenario		Peak Design Flow = 6.60 L/s	<u>Ultimate development:</u>
(If site is currently undeveloped, write 0.00 L/s in the design flows for this section)	Non-Residential Design Flows (L/s)	Self-Cleansing Design Flow = N/A Peak Design Flow = N/A	Ultimate development is where further development may / can / will occur upstream / or within the development site currently under consideration. If relevant Ultimate Peak Design Flow is
Proposed development	Residential Design Flows (L/s)	Self-Cleansing Design Flow = 3.92 L/s	to be calculated and will include number of potential units/lot.
site design flows - post- development scenario		Peak Design Flow = 8.75 L/s	For further guidance on whether this application needs to consider Ultimate development, refer CoP Sections:
Sections		And if relevant	 5.3.2 Structure Plan 5.3.3 Future development 5.3.4 System Design
		Ultimate Peak Design Flow =	
	Non-Residential Design Flows (L/s)	Self-Cleansing Design Flow =	
		Peak Design Flow	

Design considerat	ion	Description	Comments		
	Non-Residential Discharge profile / trend (i.e.	24 hr operation Residential	E.g. 24 hr operation / 10 hr (9am – 5pm, / Other (Please specify).		
Change in site flows	Operations) Net difference between post- development and pre-development site design flows	Net Change in Self-Cleansing Design Flow = 0.97 L/s Net Change in Peak Design Flow = 2.15 L/s			
New assets required for development		2 new 1050mm wastewater manholes. Two new 150mm u-PVC SN16 pipes.	If applicable please provide supporting calculations and indicative design parameters (i.e. pump station and rising main or storage.		
Existing network infrastructure capacity assessment A sewer capacity check is to be carried out if the 'Net Change in Peak Design Flow' calculated above shows a net increase of greater than 1.0 L/sec. Notes: 1. At Watercare's discretion, a Sewer Capacity Check may be required even if the net increase in site flow is < 1.0 L/sec. 2. The Level 1 Sewer Capacity Check as described in the CoP is to be undertaken in the first instance, unless specifically advised by Watercare. The Level 1 Capacity Check is intended to help identify applications that may require more accurate/detailed design calculations and/or identify whether data		Type of Sewer Capacity Check undertaken: = Level 3 Did the Existing WW Capacity Assessment Design Flow exceed the pipe-full capacity for any pipes within the Existing Network Assessment Extents? On pipes where asset data (i.e. gradient and diameter) is known: = No On pipes where asset data was assumed:	See Watercare's GIS Viewer for Asset Data Query and Land Development/Subdivision to assist with obtaining data required for the capacity assessment. In addition to the assessment finding summary requested here, other required existing network capacity assessment key steps/ deliverables include: 1. Network Assessment Extents to be identified as described in the CoP. A map is to be provided showing the network assessment extent. 2. Catchment Boundaries for the		

esign consideration	Description	Comments
held on the existing network is sufficient to enable an accurate assessment of capacity.		available, the developer and thei engineers will be required to produce catchment boundaries. a map is to be submitted depicting the catchment extents.
		3. Existing WW Capacity Assessment Design Flow is to be calculated at described in the CoP. The flows at to be tabulated for each pipe-receivithin the Network Assessment Extent. A pipe-reach will typically be regarded as the section of network between points where significant tributaries enter the network.
		4. Pipe Capacity Vs. Design Flow Check is to be carried out; a table detailing the calculated full pipe capacity compared to the 'Existir WW Capacity Assessment Design Flow' is to be provided. Pipes wit missing asset data are to have th missing data assumed as describ in the CoP.
		5. Pipe Full Capacity Exceedance - Pipes where the 'Existing WW Capacity Assessment Design Flow exceeds the pipe full capacity are be identified both in the tabular data, and on a map of the Netwo Assessment Extent. Pipes with assumed data are to be identified separately to those with known data.

Wastewater Calculations



			OUNSULTANTS
Job Name:	147-153 Edgewater Drive Pakuranga	Job No.:	496/03
Calculations By:	JF	Date:	2/05/2025
Checked By:	NP	Page No.:	1

Discharge Parameters	Pe	er Watercare	Services' standards	
Average Dry Weather Flow	Residential =	225	litres/person/day	
Peak Dry Weather Diurnal Flow	Residential =	675	litres/person/day	PF = 3
Peak Wet Weather Flow	Residential =	1500	litres/person/day	PF = 6.7
Occupancy Rates				
Typical Reside	ential Dwelling =	3	persons	
	Studio =	1	persons	
1 Bedroo	om Apartment =	2	persons	
	om Apartment =	2	persons	
	om Apartment =	3	persons	
3 Bedroo	om Apartment =	4	persons	

Existing Site

Type	No Units	No	ADWF	PDWDF (I/s)	PWWF
		Persons	(l/s)		(l/s)
Res Dwelling	4	12	0.03	0.09	0.21
Existing Totals	4	12	0.03	0.09	0.21

Proposed Site

Apartments	No Units	No Persons	ADWF (l/s)	PDWDF (I/s)	PWWF (l/s)	
1 bedroom	10	20	0.05	0.16	0.35	_
2 bedroom	12	24	0.06	0.19	0.42	
2+ bedroom	23	69	0.18	0.54	1.20	
3 bedroom	6	24	0.06	0.19	0.42	
Employees	10	10	0.03	0.08	0.17	
Proposed Total	61	137	0.38	1.15	2.56	_

Discharge Increase

		No	ADWF		PWWF
	No Units	Persons	(l/s)	PDWDF (I/s)	(l/s)
Increase	57	125	0.35	1.05	2.36

Wastewater Calculations





Job Name:	147-153 Edgewater Drive Pakuranga	Job No.:	496/03
Calculations By:	JF	Date:	2/05/2025
Checked By:		Page No.:	1

Discharge Parameters Per Watercare Services' standards

(refer also to the attached catchment plan)

Peak Wet Weather Flows

Residential = 1500 litres/person/day PWWF

= 0.017 litres/person/second PWWF

Hospital = 0.270 litres/second ADWF PF = 6.7

= 1.809 litres/second PWWF

Proposed development = 2.360 litres/second PWWF

Occupancy Rates

Average residential unit = 3 persons

Downstream Wastewater Capacity Check

Refer to attached Wastewater Catchment Plan, Rev 1
Min gradient = 0.55 % assumed for all pipes
Pipe ks factor = 1.5 mm (Coleboooke-White)

	Pipe ks factor =	1.5	mm (Coleb	oooke- white)				
Line	Ctmnt	Units	PWWF	Cum. PWWF	Pipe dia	Gradient	Capacity	Velocity
			I/s	l/s	m	%	I/s	m/s
А	А	39	2.03					
	Hospital		1.81	3.84	0.150	0.55	11.54	0.65
В	В	35	1.82	1.82	0.150	0.55	11.54	0.65
С	A+B			5.66	0.150	0.55	11.54	0.65
D	D	18	0.94	0.94	0.150	0.55	11.54	0.65
E	C+D			6.60	0.150	0.55	11.54	0.65

Adequate Capacity Confirmed

Wastewater Calculations





Job Name:	147-153 Edgewater Drive Pakuranga	Job No.:	496/03
Calculations By:	JF	Date:	2/05/2025
Checked By:	NP	Page No.:	2

Discharge Parameters Per Watercare Services' standards

(refer also to the attached catchment plan)

Peak Wet Weather Flows

Residential = 1500 litres/person/day PWWF

= 0.017 litres/person/second PWWF

Hospital = 0.270 litres/second ADWF PF = 6.7

= 1.809 litres/second PWWF

Proposed development = 2.360 litres/second PWWF

Occupancy Rates

Average residential unit = 3 persons

Downstream Wastewater Capacity Check

Refer to attached Wastewater Catchment Plan, Rev 1

Min gradient = 0.55 % assumed

Pipe ks factor = 1.5 mm (Coleboooke- White)

	Pipe ks factor =	1.5	mm (Coler	oooke- wnite)				
Line	Ctmnt	Units	PWWF	Cum. PWWF	Pipe dia	Gradient	Capacity	Velocity
			l/s	l/s	m	%	I/s	m/s
Α	A Prop site Hospital	35	1.82 2.36 1.81	5.99	0.150	0.55	11.54	0.65
В	В	35	1.82	1.82	0.150	0.55	11.54	0.65
С	A+B			7.81	0.150	2.10	22.55	1.28
D	D	18	0.94	0.94	0.150	0.55	11.54	0.65
Е	C+D			8.75	0.150	0.55	11.54	0.65
	1							

Adequate Capacity Confirmed



Development information form – Water network planning summary assessment

Development consideration	Description	Comments
Query status	Resource Consent Application	Pre-purchase enquiry / Enquiry to support Plan Change application / Pre-application enquiry / Resource Consent application / Engineering Plan approval.
Query submission date	16/09/2025	
Address	147-153 Edgewater Drive, Pakuranga, Auckland 2010	Include suburb
Attach layout plan	Please refer to DCC Civil Engineering Plan Set	Plan must clearly show proposed development site and include: • Aerial photograph with elevation contours (Note 1) • Road names • Boundary of development • Preferred point of connection to existing water supply and wastewater asset.
Current land use	Residential (single Family dwellings)	Residential (single family dwellings) / Residential (multi-unit dwellings) / Residential (multi-storey apartment blocks) / Commercial /
Proposed land use	Residential (multi-storey elderly retirement village apartment complex)	Industrial / Other (Please specify).
Unitary plan zoning	Mixed Housing Suburban Zone	Refer Auckland Unitary Plan
Total development site area (m²/hectares) (i.e. Land area for residential developments)	2990 m ²	
Total development floor area (m²)	Block A – 4715 m ² Basement: 659 m ²	



(i.e. Include all levels of multi-storey	Ground Floor: 677 m ²		
apartments and commercial developments)	Level 1: 676 m ²		
developments)	Level 2: 676 m ²		
	Level 3: 676 m ²		
	Level 4: 676 m ²		
	Level 5: 674 m ²		
	Block B – 3625 m ² Ground Floor: 681 m ²		
	Level 1: 626 m ²		
	Level 2: 626 m ²		
	Level 3: 626 m ²		
	Level 4: 598 m ²		
	Level 5: 469 m ²		
Number of proposed residential dwellings	Two 6 storey elderly retirement village	Include type and number of bedrooms residential dwellings:	for
(Typically consent or include ultimate if development is to be staged and		<u>Type:</u> <u>Quantity:</u>	
consented at a future date)	Block A – 6 storey residential apartment with 24 units.	1 bed 10	
		2 bed 12	
		2+ bed 23	
	Block B – 6 Storey	3 bed 6	
	residential apartment	4bed 0	
	with 27 units.	<i>5+bed</i> 0	

Note: (1) Watercare's GIS Viewer for Asset Data Query and Land Development/ Subdivision can be used to display aerial photography and land contour information.

Information to be completed by Developer/Engineering Consultant (This section should not be duplicated if both water and wastewater is applied. Refer to Chapter 5 of the CoP.)

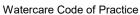
Refer to the Auckland Code of Practice for Land Development and Subdivision chapter 6: Water, when completing this form:



Design consideration	Description	Comments
		Show calculations based on Watercare
Average and Peak Residentia Demand (L/s)	Average Demand Design Flow = 0.24 L/s	Cop.
	Peak Demand Design Flow	
	= 0.91 L/s	
Average and Peak Non-Residentia	Average Demand Design Flow	Show calculations based on Watercare CoP.
Demand	=	
L/s)	Dook Domand Dooign Flour	
	Peak Demand Design Flow	
Non-Residential Demand typica	N/A	E.g. 24 hr operation / 10 hr (9am – 5pm)
daily consumption profile / trend	IV/A	/ Filling on-site storage at certain frequency.
Fire-fighting classification required by the proposed site	FW3	Refer to New Zealand Standard SNZ PAS 4509:2008.
Hydrant flow test results	⊠ Yes □ No	Attach hydrant flow test layout plan and results showing test date & time;
		location of hydrants tested and pressure logged; static pressure; flow; residual pressure.
Sprinkler system in building?	⊠ Yes □ No	Sprinkler design should consider
		Watercare Level of Service: minimum pressure at 200kPa and minimum flow at 25 l/min. The building owner shall
		conduct periodic review of sprinkler design.



WATER DEMAND CALCS



Job Name:	Ambridge Rose Apartment Complex	Job No.:	496/03
Calculations By:	JF	Date:	16/09/2025
Checked By:		Page No.:	1 of 1
File:	6-01 wat demand calcs - pre to post1.xlsx	Rev.:	1

POST-DEVELOPMENT

Peaking Factor

Population 10000 no. Peak day demand 1.50 PF Peak hourly demand = 2.5 PF

Hydraulic roughness values

Table 6.1 0.008

Fire-fighting classification = FW3

Design residential occupancy allowances

based on 220 L/p/day

0 no. Number of dwellings 1 bedroom = 0 no. Number of dwellings 2-4 bedroom = 0 no. Number of dwellings 5+ bedroom 0 no.

Number of dwellings unknown bedroom

residential buildings 4 storeys+ - based on 200 L/p/day

Number of dwellings 1 bedroom 10 no. Number of dwellings 2-4 bedroom 41 no. Number of dwellings 5+ bedroom 0 no. Number of dwellings unknown bedroom 0 no.

> Average residential demand 0.24 L/s Peak residential day demand 0.35 L/s 0.89 L/s Peak residential hourly demand =

Wet and dry commercial assumed design allowances

0 total m² Dry retail (where kitchen/toilets not normally available to public) 0 total m² Office buildings and dry retail - toilet facilities provided to customers Wet retail - food and or beverage retail/preparation 0 total m²

> 0.00 L/s Average commercial demand Peak commercial day demand 0.00 L/s Peak commercial hourly demand = 0.00 L/s

Wet and dry industrial assumed design allowances

 $0 m^2$ Light water users, or up to 2 storeys $0 m^2$ Medium water users, or 2 to 5 storeys $0 m^2$ Heavy water users, or 5 to 10 storeys $0 m^2$ Very heavy water users

0.00 L/s Average dry industry demand Peak dry industry day demand 0.00 L/s Peak dry industry hourly demand 0.00 L/s

Other facility design water flows and peaking factors			
Day facility <u></u>	=	0	beds
المعنى المعنى Night and day facility المعنى الم Staff بالمعنى المعنى المعن	=	0	beds
Staff [≗]	=	10	employees
Children 말 수 명 Staff 등 명 기계	=	0	children
Staff O Staff S Staff	=	0	employees
Primary school $_{\widehat{\mathfrak{D}}}$	=	0	students
Secondary school Secondary school Staff (sp) Staff (sp) Staff (sp)	=	0	students
Staff \vec{b} \vec{c}	=	0	employees
Secondary school (boarding)	=	0	students
Student accommodation	=	0	people
Guests 🧐 🥞 oc Staff 🗜 🕹 E	=	0	rooms
Staff 🗓 😞 🖺	=	0	employees
Community halls, churches, facilities with intermittent use	=	0	seats
Average other facility demand	=	0.01	L/s
Peak other facility day demand	=	0.01	L/s
Peak other facility hourly demand	=	0.02	L/s
Development Total Average Demand	=	0.24	L/s
Development Total Peak Day Demand	=	0.36	L/s
Development Total Peak Hourly Demand	=	0.91	L/s